Drainage Report

Regional Athletic Complex Parking Lot Design Lacey, Washington

Prepared For: City of Lacey

Prepared By: SCJ Alliance Whitney Holm, PE 8730 Tallon Lane NE, Suite 200 Lacey, WA 98516 360.352.1465





Drainage Report

Project Information

Project:	Regional Athletic Complex Parking Lot Design
Prepared for:	City of Lacey 420 College St SE Lacey, WA 98503 360.413.4340 Contact: Ashley Smith
Reviewing Agency	
Jurisdiction:	City of Lacey 420 College St SE Lacey, WA 98503
Project Representative	
Prepared by:	SCJ Alliance 8730 Tallon Lane NE, Suite 200 Lacey, WA 98516 360.352.1465 scjalliance.com
Contact:	Whitney Holm, PE
Project Reference:	SCJ #22-000313
	Path: N:\Projects\0620 City of Lacey\22- 000313 RAC Parking Lot Design\Design\Storm\Drainage Report.docx

Signature

I hereby state that this Drainage Control Plan Report for the Regional Athletic Complex Parking Lot Design has been prepared by me or under my supervision and meets the standard of care and expertise which is usual and customary in this community for professional engineers. I understand that the City of Lacey does not and will not assume liability for the sufficiency, suitability, or performance of drainage BMPs prepared by me.



Prepared by Kelcie Hopkins, EIT

Approved by Whitney Holm, PE

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Attachment 4: Maintenance and Source Control Manual

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1 Proposed Project Description

The following report was prepared for the Regional Athletic Complex (RAC) project in Lacey, WA. This report was prepared to comply with the minimum technical standards and requirements that are set forth in the City of Lacey 2022 *Stormwater Design Manual (SDM)*.

Project Proponent:	City of Lacey
Parcel Numbers:	A: 11814410200, B: 11814410300
Total Parcel Area:	A: 4.3 Acres, B: 67.06 Acres
Current Zoning:	LD 3-6, Low-Density Residential
Required Permits:	Grading, Utility, Paving, etc.
Site Address:	8323 Steilacoom Rd SE
Section, Township, Range:	Section 14, Township 18 N, Range 1 W

The proposed RAC parking lot site is located on primarily on one parcel, 11814410200 that contains a total of 4.3 acres, the project site includes some frontage and a portion of parcel number 11814410300, which totals the project acreage to 5.84 acres. The site is bounded by Steilacoom Rd SE to the north, RAC Entrances to the east and south, and Marvin Rd SE to the west. The site contains an existing gravel parking area, concrete sidewalk along the frontage on Marvin Rd SE, approximately 90' of concrete sidewalk along the frontage on Steilacoom Rd SE, and asphalt sidewalk along the frontages on the RAC Entrances. Specifically, the proposed site improvements/construction activities for this project include the following:

- Demolition of existing gravel areas
- Site preparation, grading, and erosion control activities
- Construction of parking lot, sidewalks, and plaza area
- Construction/installation of on-site water quality and flow control facilities

A site vicinity map of the proposed project location, an Existing and Proposed Basin Map, and a worksheet for determining the number of Core Requirements for this project has been prepared and is enclosed herein as **Appendix 3**. Core requirements 1-9 are required for this project. Table 1 below describes the land use of the parcel.

Land Type Designations	Area (Acres)	% of Total Area
Total On-Site Area	5.58	100
Existing Pervious Surface	5.32	95.3
Existing Impervious Surface	0.26	4.7
Proposed Pervious Surface	1.66	29.7
Proposed Impervious Surface	3.92	70.3

Table 1. Land Type Designations Existing vs. Proposed

1.1 Summary of Compliance On-Site

The stormwater design complies with the 9 core requirements as follows:

1.1.1 Core Requirement #1 – Preparation of Stormwater Site Plans

This Drainage Report will meet all of the requirements of the SDM.

1.1.2 Core Requirement #2 – Construction Stormwater Pollution Prevention

A pollution prevention plan has been included within the stormwater site plan, enclosed herein as **Attachment 2**, which describes the 13 required elements. Further, an erosion control plan has been prepared and is part of the engineering plan set, enclosed herein as **Attachment 1**. The contractor may need to amend and update these plans as part of development and/or management of the SWPPP. The contractor will be responsible for preparing the full SWPPP which shall comply with all of the required elements and the Washington Department of Ecology requirements for coverage under the NPDES Construction Stormwater General Permit.

1.1.3 Core Requirement #3 – Source Control of Pollution

All source control BMPs have been evaluated for feasibility and are identified in the Maintenance and Source Control Manual, see **Attachment 4**.

1.1.4 Core Requirement #4 – Preservation of Natural Drainage Systems and Outfalls

Currently, the majority of stormwater runoff throughout the parcel infiltrates on site. Stormwater runoff that doesn't infiltrate sheet flows from a high point in the east portion of the site to a low point located in the west portion of the site. After construction, the majority of the stormwater runoff will be collected, treated, and infiltrated on-site via bioretention ponds located in the southwest portion of the site. See section 4 of this manual for drainage system design descriptions.

1.1.5 Core Requirement #5 On-Site Stormwater Management

In accordance with Core Requirement #7, this project is not flow control exempt. Using Figure 2.1: Flow Chart for Determining Requirements for New Development, the proposed project is a new development

triggering core requirement #1-9, therefore the project shall employ the On-Site Stormwater Management BMPs in accordance with the Low Impact Performance Standard or List #2. The project will demonstrate compliance with List #2, see below.

1.1.5.1 Lawn and Landscaped Areas

Per Chapter 7 Section 7.4.1, the 2016 SDM, Post Construction Soil Quality and Depth will be utilized to the maximum extent practicable. See landscape plans for details.

1.1.5.2 Roofs

- Full Dispersion (Chapter 7, Section 7.4.2) or Downspout Infiltration (Chapter 7, Section 7.4.10)
- Bioretention (Chapter 7, Section 7.4.4)
- Downspout Dispersion Systems (Chapter 7, Section 7.4.10)
- Perforated Stub-out Connections (Chapter 7, Section 7.4.10)
- All roof related BMPs are not applicable because the site development does not include the construction of buildings or structures with overhangs.

1.1.5.3 Other Hard Surfaces:

- Full Dispersion (Chapter 7, Section 7.4.2): Full dispersion is not feasible for this project site. Full dispersion requires that the site protects at least 65% of the site in a forest or native condition. For this reason alone, this BMP is not feasible. In addition, the existing topography and the surrounding development does not allow for the required native vegetation paths.
- Permeable Pavement (Chapter 7, Section 7.4.6): Permeable pavement is not feasible due to the site grading and the requirement for treatment. The on-site soils also do not allow for treatment through infiltration.
- Bioretention (Chapter 7, Section 7.4.4): Bioretention is feasible for a portion of the proposed project improvements and will be used to the maximum extent practicable.
- Sheet Flow Dispersion or Concentrated Flow Dispersion (Chapter 7, Section 7.4.2): For the reasons mentioned above, all dispersion BMPs are infeasible for the proposed project.

1.1.6 Core Requirement #6 – Runoff Treatment

The proposed project is located within the 5-year time of travel zone for a wellhead protection area according to Figure 8B.1 of the SDM and therefore requires enhanced treatment. Enhanced treatment will be provided for the pollution generating impervious surfaces through the use of a bioretention facility. A bioretention facility was chosen for this project based on the step-by-step selection process in Section 8.2.1 of the SDM. See Section 4 of this report for an in-depth description of the selection process.

1.1.7 Core Requirement #7 – Flow Control

This requirement will be met through infiltration in multiple proposed stormwater facilities. The stormwater runoff from the proposed improvements will infiltrate within a bioretention facility that connects to an overflow retention facility. See Section 4 of this report for flow control design information.

1.1.8 Core Requirement #8 – Wetlands Protection

There are no wetlands on the project site nor does the project site does currently discharge into a wetland.

1.1.9 Core Requirement #9 – Operation and Maintenance

A Maintenance and Source Control Manual has been included as **Attachment 4**.

2 Existing Conditions Description

2.1 Existing On-Site Conditions

The subject site is +/- 5.58 acres in size, this site includes parcel 11814410200, a portion of parcel 11814410300, and frontage. Topography within the property is generally flat and level. The parcel currently has a gravel parking lot with frontage improvements along three sides of the parcel. This lot currently contains a retention pond for the RAC. See the figures below.



Figure 1 – Existing Conditions (1990)



Figure 2 – Existing Conditions (2020)

2.2 Critical Areas

Flood Zones: The project parcel is located with Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 53067C0192E. According to the FIRM Map the project parcel is located within Zone X. Zone X is determined to be an area of minimal flood hazard. See **Appendix 3** for the FIRM Map.

Wellhead Protection Areas: According to Figure 8B.1 of the SDM, the proposed project is located within the 5-year time of travel zone for a wellhead protection area. See **Appendix 3** for the Wellhead Protection Area Map.

2.3 On-site Soils Information

A geotechnical investigation was conducted by Landau Associates, Inc. (Landau) on August 17, 2022. A technical memorandum was provided on September 1, 2022 and summarized Landau's findings as follows. The site is primarily surfaced with gravel and asphalt, and surficial deposits of the site are mapped as Vashon recessional outwash (Qgo) and Latest Vashon recessional [outwash] sand and minor silt (Qgos). Soils on-site were observed in two general categories, fill and recessional outwash. Fill extended to 0.25 to 1.0 ft bgs in a dense and damp to moist condition. Fill consisted of asphalt and crushed gravel, sand and gravel with variable asphalt and plastic content. Recessional outwash was observed beneath the fill up to 10.5 ft bgs and consisted of light brown or gray to dark gray sand and gravel with variable silt and cobble content in a medium to dense and damp to moist condition. On-site soils are conducive to infiltration and bioretention facilities. Field infiltration rates measured to be 20 - 25 inches per hour. Factoring in a safety factor of about 4, a design infiltration rate of 5.5 inches per hour was utilized when sizing infiltration facilities. See **Attachment 3** for the Soils Report.

3 Vicinity Analysis and Subbasin Description

There are no known flooding or bank overtopping problems, and no steep slopes are located near the project site.

The project site is located within the Category I Critical Aquifer Recharge Area. The project site is also located in the 5-year time of travel zone for wellhead protection therefore requiring enhanced treatment. There are no known fuel tanks on-site.

3.1 Qualitative Upstream Analysis

The surrounding roadways to the parcel, with the exception of Steilacoom Rd SE, have curbing along both sides. Stormwater systems are located through all of the roadways as well. Therefore, stormwater runoff from the roadways adjacent to the parcel is collected by catch basins and conveyed through the City of Lacey stormwater system. No stormwater runoff is conveyed onto the parcel from the adjacent areas.

3.2 Qualitative/Quantitative Downstream Analysis

The majority of the stormwater runoff from the proposed improvements will be collected, treated, and infiltrated on-site. The project site is divided between two subbasins. Basin 1 collects stormwater runoff from 0.54 acres of frontage improvements along Steilacoom Rd SE and conveys the water via a new installment of catch basins and pipe into the city's stormwater collection system via an existing catch basin on the southeast corner of Steilacoom Rd SE and Marvin Rd SE. Basin 1 added less than 5,000 square feet of pervious area, therefore the frontage improvements are flow control exempt. Basin 2 collects stormwater runoff from 5.04 acres of asphalt and sidewalk improvements and conveys the water to a treatment and flow control system located in the southwest portion of the project site. The stormwater runoff will first be treated on-site within a bioretention facility sized to infiltrate 91% of the runoff then conveyed to a larger flow control pond to release the stormwater at rates less than predeveloped rates. See **Appendix 3** for Basin Maps. See **Appendix 1** for facility sizing calculations. See Section 4 of this report for the facility sizing descriptions.

4 Flow Control and Water Facility Sizing

4.1 Summary Section

Following Figure 2.1 and Figure 2.2 (See **Appendix 3**), Basin 1 of this project classifies as a redevelopment that triggers core requirements #1 - 5. Following Figure 2.1 (See **Appendix 3**), Basin 2 of this project classifies as a new development that triggers all of the core requirements. For Basin 2, bioretention facilities were sized dependent on the project site pervious area, see the following sections of this report for more information. See **Appendix 3** for the existing and proposed basin maps.

4.2 Performance Standards and Goals

Following Figure 2.1 – Flow Chart for Determining Requirements for New Development, the project site is considered a new development. Following Figure 2.2 – Flow Chart for Determining Requirements for Re-Development, the project site triggers all of the requirements for the new impervious surfaces. The majority of the stormwater runoff from the disturbed area of the project parcels will be collected, treated, and infiltrated on-site. Enhanced treatment will be provided for all of the pollution-generating impervious surfaces through the use of a bioretention facility.

4.3 Conveyance System Analysis and Design

The piped conveyance system is sized to convey the developed conditions 25-year return period peak runoff. All main stormwater conveyance pipes are a minimum of 12-inch in diameter and designed at a minimum slope of 0.5%. A backflow analysis was modeled for this system using CAD Storm and Sanitary Analysis (SSA) and passed. See **Appendix 1** for conveyance calculations.

4.4 Water Quality System

Figure 8.1 in the SDM determines that enhanced treatment is required for this project. The water will be treated via a bioretention facility which leads to a retention facility designed as described below:

- Treatment Basin
 - 5.58 acres of impervious and pervious area conveyed to the Bioretention and retention facilities.
 - Bioretention Facility Dimensions:
 - Bottom surface area: 1,538 SF
 - Bottom elevation: 222.50
 - Outlet elevation: 226.00
 - Lined with 1.5' amended soils in depth
 - Depth: 4.5' including at least 1' of freeboard
 - 3:1 Side Slopes
 - Design infiltration rate: 3 in/hr
 - Overflow Retention Facility Dimensions:
 - Bottom surface area: 5,330 SF
 - Bottom elevation: 223.00
 - Inlet elevation: 225.80

- Depth: 4' including at least 1' of freeboard
- 3:1 Side Slopes
- Design infiltration rate: 5.5 in/hr
- 100-year stage depth: 3.2 feet

See **Appendix 1** for WWHM Calculations. See **Attachment 1** for the site development drawings which include the drainage plans.

4.5 Flow Control System

Basin 1 of the proposed development does not add 5,000 square feet or more of new hard surfaces, therefore flow control is not required.

Flow control is required for Basin 2 of the proposed development and will be provided through infiltration within a bioretention facility and a retention facility. Stormwater runoff from the proposed parking and sidewalk areas is collected by catch basins throughout the site area and conveyed via 12" pipe to the southwest portion of the project site into a bioretention pond designed to infiltrate 91% of the stormwater runoff at 3 in/hr per design standards in Section 8.3.6 of the SDM. The remaining 9% of the stormwater runoff will be conveyed to a retention pond designed to infiltrate 100% at 5.5 in/hr, per design standards in Section 7.2.3 of the SDM.

The Western Washington Hydrology Model (WWHM 2012) was used to size the infiltration facilities. The drainage plan with the conveyance, treatment, and infiltration layouts has been included as Drainage Control Plan **Attachment 1**. See **Appendix 1** for the WWHM reports.

Land Type Designations	Area (Acres)
Basin 2	5.04
Asphalt	3.42
Sidewalk	0.07
Landscape	1.55

Table 2. Flow Control Basin Area Analysis

5 Aesthetic Considerations for Facilities

All of the stormwater facilities will be designed in such a way that they will provide necessary treatment and flow control while also be aesthetically pleasing to the proposed use of the site. The bioretention facility and retention facility will be planted in a way that will match the surrounding landscaping areas.

6 Covenants, Dedications, Easements

It is the City of Lacey's policy that the property owner(s) shall maintain their stormwater drainage facilities. Thus, the City of Lacey will be responsible for maintaining and insuring that all installed

drainage facilities are functioning in accordance with their design purposes. The City of Lacey will keep a copy of the maintenance plan at the project site. The Maintenance and Source Control Manual is a standalone document submitted separately from the Stormwater Site Plan and the Establishment of Maintenance Covenant will be included as **Attachment 5** at the time of the civil permit submittal.

It is important to note that only slow release fertilizers shall be applied for the life of the development at a maximum amount of 4 lbs of nitrate as Nitrogen annually and no more than 1 lb. per application for every 1,000 square feet of turf grass. Only fertilizer formulas with a minimum of 50% water insoluble form of nitrogen are permitted for use. Approved water insoluble forms of nitrogen include sulfur and/or polymer coated fertilizers, Isobutylidene Diurea (IBDU), Methylene Urea and Ureaform, and organic fertilizers registered with Washington Department of Agriculture.

7 Agreements and Guarantees

Maintenance and/or operational bonding or other appropriate financial guarantees are required for all projects to ensure construction and functionality of drainage facilities in compliance with applicable standards. These guarantees are to be consistent with the most recent edition of the City of Lacey Development Guidelines and Public Works Standards.

8 Other Permits or Conditions Placed on the Project

There are no other known required permits at this time.

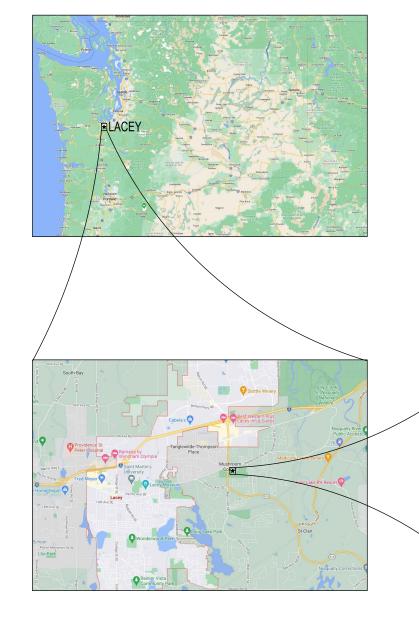
END OF STORMWATER SITE PLAN

Drainage Control Plan Attachment 1

Site Development Drawings

SEC. 14, T 18N., R 1W., W.M.

RAC PARKING LOT EXPANSION 90% CONSTRUCTION PLANS LACEY, WA



OWNER / APPLICANT

CITY OF LACEY 420 COLLEGE ST SE LACEY, WA 98503 PHONE: 360.413.4340 CONTACT: ASHLEY SMITH

CONSULTANTS

SCJ ALLIANCE 8730 TALLON LANE NE, SUITE 200 LACEY, WA 98516 PHONE: 360.352.1465 CONTACT: WHITNEY HOLM, PE

SITE INFORMATION

PARCEL NUMBER: 11814410200; 11814410300 ACRES: ±6.28

SITE ADDRESS: 8345 STEILACOOM RD SE LACEY, WA 98513

BASIS OF BEARINGS:

MERIDIAN IS WASHINGTON COORDINATE SYSTEM OF 1983/91 - SOUTH ZONE DERIVED FROM TIES TO HPON STATIONS SANDERSON, MCKENNA AND CBL1110 AND TO WSDOT GPS STATIONS G259R, GP34005-2, GP34005-4, GP34101-32, GP34101-39, HC34-2, LUHR RW2, TS34-33, TS34-59 AND TO THURSTON COUNTY GPS STATIONS U-531, AT-194, AT-352, AT-355, AT-447, AT449 AND AT-478. AI-332, AI-333, AI-447, AI-447, AI-449 AND AI-470. DISTANCES SHOWN ARE GROUND SCALE U.S. SURVEY FEET. COMBINED SCALE FACTOR (GROUND TO GRID) IS 0.999935701. SURVEY AF# 3111152 DATED 09-24-1997.

DATUM:

THE CITY OF LACEY BENCHMARKS ARE BASED ON NGVD 29 DATUM FROM PRIMARY CONTROL ESTABLISHED BY FEDERAL AND STATE ACENCIES TO FIRST OR SECOND ORDER ACCURACY. THE CITY OF LACEY'S BENCHMARKS ARE GENERALLY TO THIRD ORDER ACCURACY.

LEGAL DESCRIPTION: SECTION 13 / 14 TOWNSHIP 18 RANGE 1W QUARTER NW SW / NE SE BLA13101203 TR A DOCUMENT 4335765

SECTION 13 / 14 TOWNSHIP 18 RANGE 1W QUARTER NW SW / NE SE BLA13101203LA TR B DOCUMENT 4335765

UTILITIES

STORMWATER/SEWER: CITY OF LACEY UTILITY SERVICES PO BOX 3400 420 COLLEGE STREET SE LACEY, WA 98509-3400 PH: 360.491.5616

WATER/SOLID WASTE: CITY OF LACEY UTILITY SERVICES PO BOX 3400 420 COLLECE STREET SE LACEY, WA 98509-3400 PH: 360.491.5616

ELECTRIC/NATURAL GAS: PUGET SOUND ENERGY – PSE 10885 NE 4TH STREET BELLEVUE, WA 98009 PH: 425.452.1234 EMERGENCY SERVICE: 888.225.5773 CONTACT: DAVE LESINSKI ENGINEERING SPECIALIST, CONSTRUCTION SERVICES PH: 253.476.6425

CUSTOMER CONSTRUCTION 3130 S 38TH ST TACOMA, WA 98409 PH: 888.321.7779

TELEPHONE: QMEST COMMUNICATIONS 711 CAPITOL WAY S, STE 307 OLYMPIA, WA 98501 PH: 360.754.5912 CONTACT: WAYDE HOLMQUIST SR DESIGN ENGINEER

CABLE:

ZABLE: COMCAST 410 VALLEY AVE NW, STE 307 PUYALLUP, WA 98371 PH: 253.261.0945 FX: 253.864.4215 CONTACT: ERIC PARTON COMCAST BUSINESS SERVICES GROUP



A PORTION OF SEC 14, T 18N., R 1W., W.M. LACEY, WA

CALL BEFORE YOU DIG THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR THE LOCATION AND PROTECTION OF ALL EXISTING UTILITIES. THE CONTRACTOR SHALL VERIFY ALL UTILITY LOCATIONS PRIOR TO CONSTRUCTION BY CALLING THE UNDERVIDE LOCATE LINE AT BIT OR 1.200.424.5555 A MINIMUM OF 48 HOURS PRIOR TO ANY EXCAVATION.

	SHE	EET INDEX
SHEET NUMBER	SHEET TITLE	SHEET DESCRIPTION
01	CV-01	COVER SHEET
02	GN-01	GENERAL NOTES
03	EX-01	EXISTING CONDITIONS MAP COVER SHEET
04	EX-02	EXISTING CONDITIONS MAP
05	EX-03	EXISTING CONDITIONS MAP
06	EC-01	DEMOLITION AND TESC PLAN
07	EC-02	EROSION CONTROL NOTES & DETAILS
08	SP-01	SITE & PAVING PLAN
09	SP-02	SITE PLAN NOTES & DETAILS
10	SP-03	SITE PLAN DETAILS
11	HC-01	HORIZONTAL CONTROL PLAN
12	CG-01	GRADING PLAN
13	CG-02	ENLARGED GRADING PLAN
14	CG-03	ENLARGED GRADING PLAN
15	SD-01	DRAINAGE PLAN
16	SD-02	STORM DRAINAGE NOTES & DETAILS
17	SD-03	STORMWATER POND PROFILES
18	SD-04	STORM PIPE PROFILES
19	SD-05	STORM PIPE PROFILES
FRONTAGE IMPRO	OVEMENTS	
20	FR-01	STEILACOOM TESC & DEMO PLAN
21	FR-02	STEILACOOM ROADWAY PLAN
22	FR-03	STEILACOOM STRIPING PLAN
23	FR-04	STEILACOOM RD DETAILS
LANDSCAPE		
24	LS-00	OVERALL LANDSCAPE PLAN
25	LS-01	NORTHWEST LANDSCAPE PLAN
26	LS-02	NORTHEAST LANDSCAPE PLAN
27	LS-03	SOUTHWEST LANDSCAPE PLAN
28	LS-04	SOUTHEAST LANDSCAPE PLAN
29	LS-05	LANDSCAPE DETAILS
30	LS-10	OVERALL IRRIGATION PLAN
31	LS-11	NORTHEAST IRRIGATION PLAN
32	LS-12	NORTHEAST IRRIGATION PLAN
33	LS-13	SOUTHWEST IRRIGATION PLAN
34	LS-14	SOUTHEAST IRRIGATION PLAN
35	LS-15	IRRIGATION NOTES AND DETAILS

BY	
DATE	
REVISIONS	
$\overline{\triangleleft}$	
***	SCJ ALLIANCE CONSULTING SERVICES 8730 TALLON LANE NE, SUITE 200, LACEY, WA 98516 P: 360.352.1465 F: 360.352.1509 SCIALLIANCE.COM
SHEET TILE: COVER SHEET	PROJECT NAME: RAC PARKING LOT EXPANSION 8345 STELLACOOM RD SE LACEY, WA
SEAL:	TNEY G. A
PROFES	or Washing and a second
DESIGNER W. H DRAWN B	IOLM
K. G. APPROVE	ANS D BY:
W.H DATE: DEC	IOLM EMBER 2022
JOB NO: 22-0	00313
	00313 CV-01 NO:
	NO: CV-01

SCJ ALLIANCE GENERAL CONSTRUCTION NOTES:

- ALL WORK, WORKMANSHIP AND MATERIALS FOR THIS PROJECT SHALL MEET OR EXCEED THE PROJECT SPECIFICATIONS, CITY OF LACEY ENGINEERING DESIGN AND DEVELOPMENT STANDARDS AND MAY INCLUDE REFERENCES TO THE MOST CURRENT VERSION OF THE \$\$\$-WSDOT STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION—\$\$\$. IF THERE IS A CONFLICT BETWEEN THE SPECIFICATION TYPES, THE MOST RESTRICTIVE REFERENCE SHALL BE ADHERED TO.
- 2. ALL GOVERNMENTAL SAFETY REGULATIONS SHALL BE STRICTLY ADHERED TO INCLUDING OSHA, WISHA AND THE WASHINGTON DEPARTMENT OF LABOR AND INDUSTRY.
- 3. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DULY NOTIFY THE CITY IN ADVANCE OF THE COMMENCEMENT OF ANY AUTHORIZED WORK AND TO SCHEDULE REQUIRED INSPECTIONS. ALL REQUIRED INSPECTION TESTS WILL BE PERFORMED AT THE CONTRACTOR'S EXPENSE.
- 4. THE CONTRACTOR SHALL PROVIDE A TWO-YEAR WARRANTY ON ALL WORKMANSHIP AND MATERIAL FOLLOWING ACCEPTANCE OF THE PROJECT BY THE OWNER.
- 5. THE APPROVAL OF THESE PLANS BY THE CITY OF LACEY DOES NOT RELIEVE THE CONTRACTOR OR DEVELOPER OF THE RESPONSIBILITY TO COMPLY WITH THE REQUIREMENTS OF OTHER GOVERNING AGENCIES.
- 6. <u>CAUTION NOTICE TO CONTRACTOR</u> THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON THE PROJECT SURVEY AND OTHER RECORDS OF UTILITIES. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR SHALL CALL 1–800–424–5555 48 HOURS PRIOR TO PLANNED EXCAVATIONS.
- 7. THE DESIGN SHOWN IS BASED UPON THE ENGINEER'S UNDERSTANDING OF THE EXISTING CONDITIONS. THE EXISTING CONDITIONS SHOWN ON THIS PLAN SHEET ARE BASED UPON COMPILED SURVEY DATA. THE CONTRACTOR IS RESPONSIBLE FOR VERFINING FIELD CONDITIONS PRIOR TO BIDDING THE PROPOSED WORK IMPROVEMENTS. IF CONFLICTS ARE DISCOVERED, THE CONTRACTOR SHALL NOTIFY THE OWNER OR OWNERS REPRESENTATIVE.
- 8. EXISTING UTILITIES ARE SHOWN FOR REFERENCE ONLY. PRIOR TO CONNECTION TO EXISTING UTILITIES THE CONTRACTOR SHALL VERIFY EXACT LOCATION, DIA, LENGTH, CONDITION, PIPE TYPE, SLOPE AND VERTICAL AND HORIZONTAL ALIGNMENT OF THE EXISTING UTILITY AND PROPOSED POINT OF CONNECTION. THE CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO ENGINEER PRIOR TO CONSTRUCTION.
- 9. PRIOR TO COMMENCING WORK, THE CONTRACTOR SHALL OBTAIN ALL NECESSARY LOCAL, STATE, AND FEDERAL APPROVALS AND PERMITS.
- 10. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO HAVE A COPY OF THE APPROVED PLANS, SPECIFICATIONS, AND CONTRACT DOCUMENTS AT THE CONSTRUCTION SITE AT ALL TIMES.
- 11. THE CONTRACTOR SHALL PROVIDE SLOPE PROTECTION FOR SLOPES OF 5:1 OR GREATER ACCORDING TO ASSOCIATED GENERAL CONTRACTORS (AGC) STANDARD GUIDELINES AND BEST MANAGEMENT PRACTICES (BMP'S).
- 12. THE CONTRACTOR SHALL MAINTAIN EROSION CONTROL FACILITIES IN ACCORDANCE WITH THE CITY OF LACEY, DEPARTMENT OF ECOLOGY STORNWATER MANUAL, THE STORNWATER POLLUTION PREVENTION PLAN (SWPPP), AND PER THE EROSION CONTROL PLAN.
- 13. THE CONTRACTOR SHALL SAFELY MAINTAIN TRAFFIC AND CONTINUOUS ACCESS TO PRIVATE AND/OR PUBLIC PROPERTY.
- 14. CONSTRUCTION SIGNING AND TRAFFIC CONTROL SHALL BE PER THE CURRENT COPY OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).
- ALL VEHICLES AND EQUIPMENT SHALL BE KEPT WITHIN THE WORK AREAS ESTABLISHED FOR THAT WORK SHIFT UNLESS TRAVELING TO OR FROM THE SITE. UNDER NO CIRCUMSTANCES SHALL VEHICLES OR EQUIPMENT BE PARKED OR STORED OUTSIDE OF THESE AREAS.
- 16. OTHER CONSTRUCTION PROJECTS MAY OCCUR NEAR THE PROJECT SITE AND MAY BE IN PROGRESS CONCURRENT WITH THE PROJECT. THE CONTRACTOR SHALL COOPERATE AS NECESSARY AND NOT INTERFERE OR HINDER THE PROGRESS OR COMPLETION OF WORK BEING PERFORMED BY OTHER CONTRACTORS.
- 17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING AND INSTALLING ALL MATERIALS, LABOR, AND EQUIPMENT NECESSARY TO COMPLETE THE WORK SHOWN ON THESE DRAWINGS AND TO OBTAIN ACCEPTANCE BY THE CITY OF LACEY AND THE PROJECT OWNER.
- 18. ALL AREAS DISTURBED DURING CONSTRUCTION SHALL BE RESTORED TO THEIR ORIGINAL "PRE CONSTRUCTION" STATE OR BETTER.
- 19. DRIVEWAY ACCESS AND UTILITY SERVICE TO EXISTING HOMES AND BUSINESSES SHALL BE MAINTAINED AT ALL TIMES.
- 20. SEE LANDAU DRAFT TECHNICAL MEMORANDUM DATED SEPTEMBER 1, 2022 FOR ADDITIONAL INFORMATION INCLUDING GROUNDWATER

SEC. 14, T 18N., R 1W., W.M.

CITY OF LACEY GENERAL CONSTRUCTION NOTES

- 1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH CITY OF LACEY STANDARDS AND THE MOST CURRENT EDITION OF THE STATE OF WASHINGTON STANDARD SPECIFICATIONS FOR ROAD, BRIDGE AND MUNICIPAL CONSTRUCTION (WSDOT/APWA). IN CASES OF CONFLICT, THE MOST STRINGENT STANDARD SHALL APPLY.
- 2. THE CONTRACTOR SHALL BE IN COMPLIANCE WITH ALL SAFETY STANDARDS AND REQUIREMENTS AS SET FORTH BY OSHA, WISHA AND THE STATE OF WASHINGTON, DEPARTMENT OF LABOR AND INDUSTRIES.
- 3. ALL APPROVALS AND PERMITS REQUIRED BY THE CITY OF LACEY SHALL BE OBTAINED BY THE CONTRACTOR PRIOR TO THE START OF CONSTRUCTION.
- 4. IF CONSTRUCTION IS TO TAKE PLACE IN THE COUNTY RIGHT-OF-WAY, THE CONTRACTOR SHALL NOTIFY THE COUNTY AND OBTAIN ALL THE REQUIRED APPROVALS AND PERMITS.
- 5. A PRE-CONSTRUCTION MEETING SHALL BE HELD WITH THE CITY OF LACEY CONSTRUCTION INSPECTOR A MINIMUM 72 HOURS PRIOR TO THE START OF CONSTRUCTION.
- 6. THE CITY OF LACEY CONSTRUCTION INSPECTOR SHALL BE NOTIFIED A MINIMUM OF 48 HOURS IN ADVANCE OF A TAP CONNECTION TO AN EXISTING MAIN. THE INSPECTOR SHALL BE PRESENT AT THE TIME OF THE TAP.
- 7. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR THE LOCATION AND PROTECTION OF ALL EXISTING UTILITIES. THE CONTRACTOR SHALL VERIFY ALL UTILITY LOCATIONS PRIOR TO CONSTRUCTION BY CALLING THE UNDERGROUND LOCATE LINE AT 811 NOT LESS THAN TWO BUSINESS DAYS AND NOT MORE THAN 10 BUSINESS DAYS PRIOR TO ANY EXCAVATION. THE CONTRACTOR WILL ALSO BE RESPONSIBLE FOR MAINTAINING ALL LOCATE MARKS ONCE THE UTILITIES HAVE BEEN LOCATED.
- 8. TEMPORARY STREET PATCHING SHALL BE ALLOWED FOR AS APPROVED BY THE CITY ENGINEER. TEMPORARY STREET PATCHING SHALL BE PROVIDED BY PLACEMENT AND COMPACTION OF 1-INCH MAXIMUM ASPHALT CONCRETE COLD MIX. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTENANCE AS REQUIRED.
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TRAFFIC CONTROL IN ACCORDANCE WITH THE WSDOT/APWA STANDARD PLANS FOR ROAD, BRIDGE AND MUNICIPAL CONSTRUCTION (ALL APPLICABLE K: PLANS) AND/OR THE MANUAL ON UNFORM TRAFFIC CONTROL DEVICES (MUTCD). PRIOR TO DISRUPTION OF ANY TRAFFIC, A TRAFFIC CONTROL PLAN SHALL BE PREPARED AND SUBMITTED TO THE CITY FOR APPROVAL. NO WORK SHALL COMMENCE UNTIL ALL APPROVED TRAFFIC CONTROL IS IN PLACE.
- 10. EROSION CONTROL/WATER POLLUTION MEASURES SHALL BE REQUIRED IN ACCORDANCE WITH SECTION 1-07.15 OF THE WSDDT/APWA STANDARD SPECIFICATIONS FOR ROAD, BRIDGE AND MUNICIPAL CONSTRUCTION AND THE CITY OF LACEY STORWMATER DESIGN MANUAL. AT NO TIME WILL SILTS AND DEBRIS BE ALLOWED TO DRAIN INTO AN EXISTING OR NEWLY INSTALLED FACILITY UNLESS SPECIAL PROVISIONS HAVE BEEN DESIGNED.
- 11. ALL SURVEYING AND STAKING SHALL BE PERFORMED PER THE CORRESPONDING CHAPTER OF THE CITY OF LACEY DEVELOPMENT GUIDELINES AND PUBLIC WORKS STANDARDS.
- 12. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO HAVE A COPY OF AN APPROVED SET OF PLANS ON THE CONSTRUCTION SITE AT ALL TIMES.
- 13. ANY CHANGES TO THE DESIGN SHALL FIRST BE REVIEWED AND APPROVED BY THE PROJECT ENGINEER AND THE CITY OF LACEY.
- 14. IF CONSTRUCTION IS TO TAKE PLACE IN OTHER JURISDICTION'S RIGHT-OF-WAY(I.E., THE COUNTY, THE STATE, THE CITY OF OLYMPIA, OR OTHER ADJACENT MUNICIPALITIES), THE CONTRACTOR SHALL NOTIFY THE JURISDICTION AND OBTAIN ALL THE REQUIRED APPROVALS AND PERMITS.
- 15. PRIOR TO BACKFILL ALL MAINS AND APPURTENANCES SHALL BE INSPECTED AND APPROVED BY THE CITY OF LACEY CONSTRUCTION INSPECTOR. APPROVAL SHALL NOT RELIEVE THE CONTRACTOR FOR CORRECTION OF ANY DEFICIENCIES AND/OR FAULTESA SO DETERMINED BY SUBSEQUENT TESTING AND INSPECTIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE CITY OF LACEY FOR THE REQUIRED INSPECTIONS.
- 16. THE CITY WILL BE GIVEN 72-HOURS' NOTICE PRIOR TO SCHEDULING A SHUTDOWN. WHERE CONNECTIONS REQUIRE "FIELD VERIFICATION", CONNECTION POINTS SHALL BE EXPOSED BY THE CONTRACTOR AND FITTINGS VERIFIED 72 HOURS PRIOR TO DISTRIBUTING SHUT-DOWN NOTICES.

ABBRE	VIATIONS
&б©€. =. >. #%	AND ANGLE APPROXIMATELY AT CENTERLINE DEGREE EQUALS FOOT GREATER THAN INCH NUMBER PERCENT
AC	ASPHALTIC CONCRETE
ADD'L	ADDITIONAL
ADJT	ADJACENT
AFF	ABOVE FINISH FLOOR
AP	ANGLE POINT
APPROX	APPROXIMATE
ARCH	ARCHITECT
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
ATB	ASPHALT TREATED BASE COURSE
AVE	AVENUE
BCR	BEGIN CURB RETURN
BFV	BUTTERFLY VALVE
BGS	BELOW GROUDS SURFACE
BLK	BLOCK(S)
BLDG	BUILDING
BM	BENCHMARK
BVC	BEGIN VERTICAL CURB
C CF CF CIP CIP MON CJ € CL CL CCR CO COMPT CONC CONT CONT CONT CONT CONT CONT CON	CONDUIT CATCH BASIN CUBIC FEET CUBIC TEAT CAST-IN-PLACE CAST-IN-PLACE CAST-IN-PLACE CONTRUCT CENTER JOINT CENTER JOINT CENTER JOINT CENTER JOINT CONTRUCT COMUNICATION COMUNICATION COMUNICATION COMUNICATION COMUNICATION COMUNICATION CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CONSTRUCT CUBIC YARD
D/W DEF DEG DEMO DIA DIM DIP DR DWG(S)	DRIVEWAY DEFLECTION DEGREE DEMOLISH/DEMOLITION DIAMETER DIMENSION(S) DUCTILE IRON PIPE DRIVE DRIVE DRAWING(S)
e	EAST OR ELECTRICAL
EA	EACH
ECR	END CURR RETURN
EHH	ELECTRICAL HANDHOLE
EL, ELEV	ELEVATION
ELCC	ELECTRIC(AL)
ENGR	ENGINEER
EOP	EDGE OF PAVEMENT
EQ	EQUAL(LY)
EQUIP	EQUIPMENT
ESMT	EASEMENT
EVC	END VERTICAL CURVE
EX, EXIST	EXISTING EXP EXPANSION
EXP	EXPANSION
FDC	FIRE DEPARTMENT CONNECTION
FDN	FOUNDATION
FF	FINISH FLOOR
FG	FINISH GRADE ELEVATION
FH	FIRE HVDRANT
FIN	FINISH(ED)
FL	FIRE LINE/FLANGE
FT	FOOT/FEET
G	GAS
GALV	GALVANIZED
GRND	GROUND
GV	GATE VALVE
hh	Handhole
Horiz	Horizontal
Ht	Height
ie	INVERT ELEVATION
In	INCH
JB, J-BOX JT	JOINT TRENCH
KV	KILOVOLTS
KW	KILOWATT
KWH	KILOWATT HOURS

CALL BEFORE YOU DIG

THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR THE LOCATION AND PROTECTION OF ALL EXISTING UTILITIES. THE CONTRACTOR SHALL VERITY ALL UTILITY LOCATIONS PRIOR TO CONSTRUCTION BY CALLING THE UNDERGROUND LOCATE LINE AT 811 OR 1.800.424.5555 A MINIMUM OF 48 HOURS PRIOR TO ANY EXCAVATION.

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L	LENGTH	DATE	
lb(S) lf lp lt	POUND(S) LINEAR FEET LOW POINT ELEVATION LEFT		
МАХ	MAXIMUM		
mfr Mh Min	MANUFACTURER MANHOLE MINIMUM, MINUTE		
MISC MON	MISCELLANEOUS MONUMENT IN CASE		
N N/A	NORTH, NORTHING NOT APPLICABLE		
NE NEMA NIC	NORTHEAST NATIONAL ELECTRICAL MANUFACTURER ASSOCIATION NOT IN CONTRACT		
NO, NO NTS NW	NUMBER NOT TO SCALE NORTHWEST	ş	
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OD OSHA	OUTSIDE DIAMETER OCCUPATIONAL SAFETY & HEALTH ADMINISTRATION	\triangleleft	
P PC PCC	POWER, POWER VAULT POINT OF CURVATURE POINT OF COMPOUND CURVE		
PED	OR PORTLAND CEMENT CONCRETE PEDESTAL		98516
PI P POC	POINT OF INTERSECTION PROPERTY LINE POINT OF CONNECTION		
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PSI PT	POUNDS PER SQUARE INCH POINT OF TANGENCY		ALLIA BULTING SERV LIE NE, SUITE 200, 1 52.1465 F: 360.33 SCIALLIANCE.COM
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PVMT PWR	POWER	▼	CJ ALLIANC CONSULTING SERVICES LON LANE NE, SUITE 200, LACEY W P: 360.352.1465 F. 360.352.1509 SCIALUANCE.COM
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r RD REF	RADUS ROAD, ROADWAY REFERENCE		8730 1
reinf Req'd	REINFORC(E, ED, ING, MENT) REQUIRED		
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R/W, ROW	RIGHT OF WAY		
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T TBD	TELEPHONE OR TELEPHONE VAULT TO BE DETERMINED		RAC
tbm TC Tele	TEMPORARY BENCH MARK TOP OF CURB ELEVATION TELEPHONE	ШЕ	T NAME:
TEMP TOW	TEMPORARY TOP OF WALL ELEVATION	SHEET T	PROJECT NAME
TP, T/P TYP	TOP OF PIPE TYPICAL	SEAL:	and a second
UDG VAP	UNDERGROUND VERTICAL ANGLE POINT	A A A A A A A A A A A A A A A A A A A	OF WASHING
VC VERT	VERTICAL CURVE VERTICAL	PRO	
VOL	VOLUME	Tel	REGISTERED GIN
W W/ W/O	WEST, WDTH, WDE OR WATER WITH WITHOUT	DESIGNER:	12/09/2022
W/O WM WV	WIIGUUI WATER MAIN OR WILLAMETTE MERIDIAN WATER VALVE	W. HOI DRAWN BY:	M
XFMR	TRANSFORMER	K. GAN APPROVED I	
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GN-01

02 OF 35

SHEET N

TOPOGRAPHIC SURVEY OF REGIONAL ATHLETIC COMPLEX EAST $\frac{1}{2}$, SEC. 14, T18N, R1W, W.M.

BASIS OF BEARING MERIDIAN IS WASHIMIOTON COORDINATE SYSTEM OF 1983/91 - SOUTH ZONE DERIVED FROM TIES TO HPGN STATIONS SANDERSON, MCKENNA AND CBL1110 AND TO WSDOT GPS STATIONS G2698, CP34005-2, CP34005-4, CP34101-32, CP34101-39, HC34-2, LUHR RM2, TS34-33, TS34-9 AND TO THURSTON COUNTY OPS STATIONS U-531, AT-194, AT-352, AT-355, AT-447, AT449 AND AT-478. DISTANCES SHOWN ARE GROUND SCALE U-S. SURVEY FEET. COMBINED SCALE FACTOR (GROUND TO GRID) IS 0.999935701. SURVEY AF# 3111152 DATED 09-24-1997.

METHOD OF SURVEY SURVEY PERFORMED BY CONVENTIONAL FIELD TRAVERSE USING A LEICA TS-16 (THREE SECOND TOTAL STATION), LINEAR AND ANGULAR CLOSURE OF THE TRAVERSE MEET THE STANDARDS OF WAG 328-130-090.

BARB WIRE FENCE CHAIN LINK FENCE CONCRETE FENCE ROCK FENCE - X00- X00- X00- X00- X00- WIRE FENCE WOOD FENCE GRAVEL TOE OF SLOPE _____ · · · TOP · · · ____ TOP OF SLOPE DITCH SD _____ STORM (PAINTED) SEWER (PAINTED) _____ SS _____ RIGHT OF WAY OVERHEAD CABLE _____ OTV ____ UNDERGROUND CABLE (PAINTED) _____ G _____ GAS (PAINTED) OVERHEAD PHONE _____ OF0 _____ OVERHEAD FIBER OPTIC UNDER GROUND PHONE (PAINTED) UNDERGROUND FIBER OPTIC (PAINTED) OVERHEAD ELECTRIC

UNDERGROUND ELECTRIC (PAINTED)

RECLAIMED WATER (PAINTED)

WATER (PAINTED)

----- RW -----

BOUNDARY THE BOUNDARY WAS COMPILED USING PHYSICAL FEATURES TIED IN THE FIELD AND DOCUMENTS OF RECORD AS SHOWN BELOW.

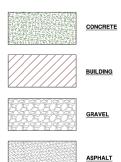
BLA REFERENCED

VERTICAL THE CITY OF LACEY BENCHMARKS ARE BASED ON NOVD 29 DATUM FROM PRIMARY CONTROL ESTABLISHED BY FEDERAL AND STATE AGENCIES TO FIRST OR SECOND ORDER ACCURACY. THE CITY OF LACEY'S BENCHMARKS ARE GENERALLY TO THIRD ORDER ACCURACY ACCURACY

> BENCHMARK CITY OF LACEY BM#993 2" SURFACE MONUMENT AT THE C/L OF VINE MAPLE DR SE, 80" 4-' SOUTH OF STEILACOOM RD SE ELEV.=218.711'

SOURCE OF CONTOURS COMPILE FIELD MEASUREMENTS USING AUTOCAD CIVIL 3D-2020 SOURCE OF UTILITY MARKINGS SURFACE MARKINGS PROVIDED BY UTILITY NOTIFICATION CENTER (callbeforeyoudig.org) TICKET # 21189894 "COMMUNICATION UTILITY WAS NOT PROPERLY MARKED IN FIELD

FIELD SURVEYOR & FIELD BOOK MIKE BREDESON & 256



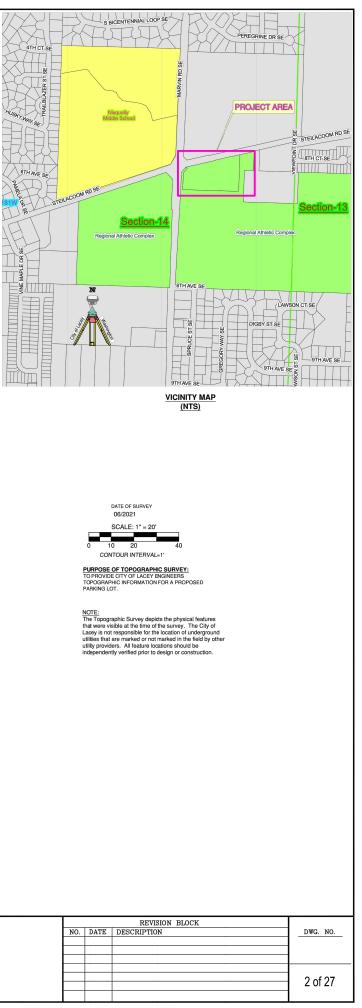
Axle	Bike Symbol	Sewer Meter	Reduce Pressure Backflow Assem.	*GATE Gate	O PVC Pipe	Alder Tree
Brass Disk	E Handicap	B Sewer Pump	+ws Water Service	-℃ Guy Pole	D Rock	Birch Tree
Boatspike	Left turn	Sewer Valve	+ www. Water Service-Reclaimed	A Headstone	Service Disconnect	Cedar Tree
Cased Monument	Only Text	OBO Blowoff	R Water Tank	Junction Box type 1	-Sv Service Pole	Cherry Tree
Concrete Monument	Parallel Park End	o RW Blowoff-Reclaimed	Water Valve	Junction Box type 2	Sign	Reconifer Tree
Hub & Tack	Parallel Park Tic	💢 Hydrant	Water Valve-Reclaimed	Junction Box type 3	프프 Sign (Large)	 Cottonwood Tree
Iron Pipe	Railroad Crossing	💥 Hydrant-Reclaimed	Water Vault	X-→ Light Pole	Signal Control Cabinet	Deciduous Tree
Magnail w/ flasher	Right turn	R Irrigation Valve	Water Vault-Reclaimed	Light Standard	Signal Pole	🔆 Fir Tree
Magnail	Straight Thru	Irrigation Valve-Reclaimed	well Well	💥 Luminaire	Street Light	Fruit Tree
PKnail w/flasher	Straight Thru and Left Turn	Post Indicator Valve	J Anchor	P Mailbox	Telehone Booth	Hemlock tree
Knail	Straight thru and Right Turn	Sampling Station	Basketball Hoop	Des Pedestrain Head	O _T Telephone Manhole	Holly Tree
Rebar no/cap	Catch Basin	RW Sampling Station-Reclaimed	BBQ BBQ	Pile	T Telephone Pedestal	Madrona Tree
1/2" or #4 Rebar w/cap	Catch Basin w/EJIW inlet	Sprinkler head	Bollard	E Pinic Table	Telephone Vault	O Maple Tree
5/8" or #5 Rebar w/cap	Culvert	sprinkler head-Redaimed	Dumpster	O Pole	Trash Can	Oak Tree
Railroad Spike	°C0 Storm C/O	S Water Air Release	Filler Cap	D Post	TV Pedestal	Pine Tree
X Scribe	Storm Manhole	Water Manhole	Flag Pole	Power Meter	-OU Utility Pole	Poplar Tree
Traverse Point	Storm Strip Drain	Water Manhole-Reclaimed	O _G Gas Manhole	Power Pole	X Yard Light	Spruce Tree
SR Non Standard Curb Return	Lift Station	W Water Meter	SG Gas Meter	Power Pole w/drop	റ്റ് w Water Air Release-Reclaimed	Willow Tree
Truncated Domes	OCO Sewer C/O	Rw Water Meter-Reclaimed	Gaspump	Power Pole w/Luminaire	Tree Grate	🗢 Bush
	Sewer Manhole	Water Pump	Gas Vavle	D. Power Vault		凡 Stump

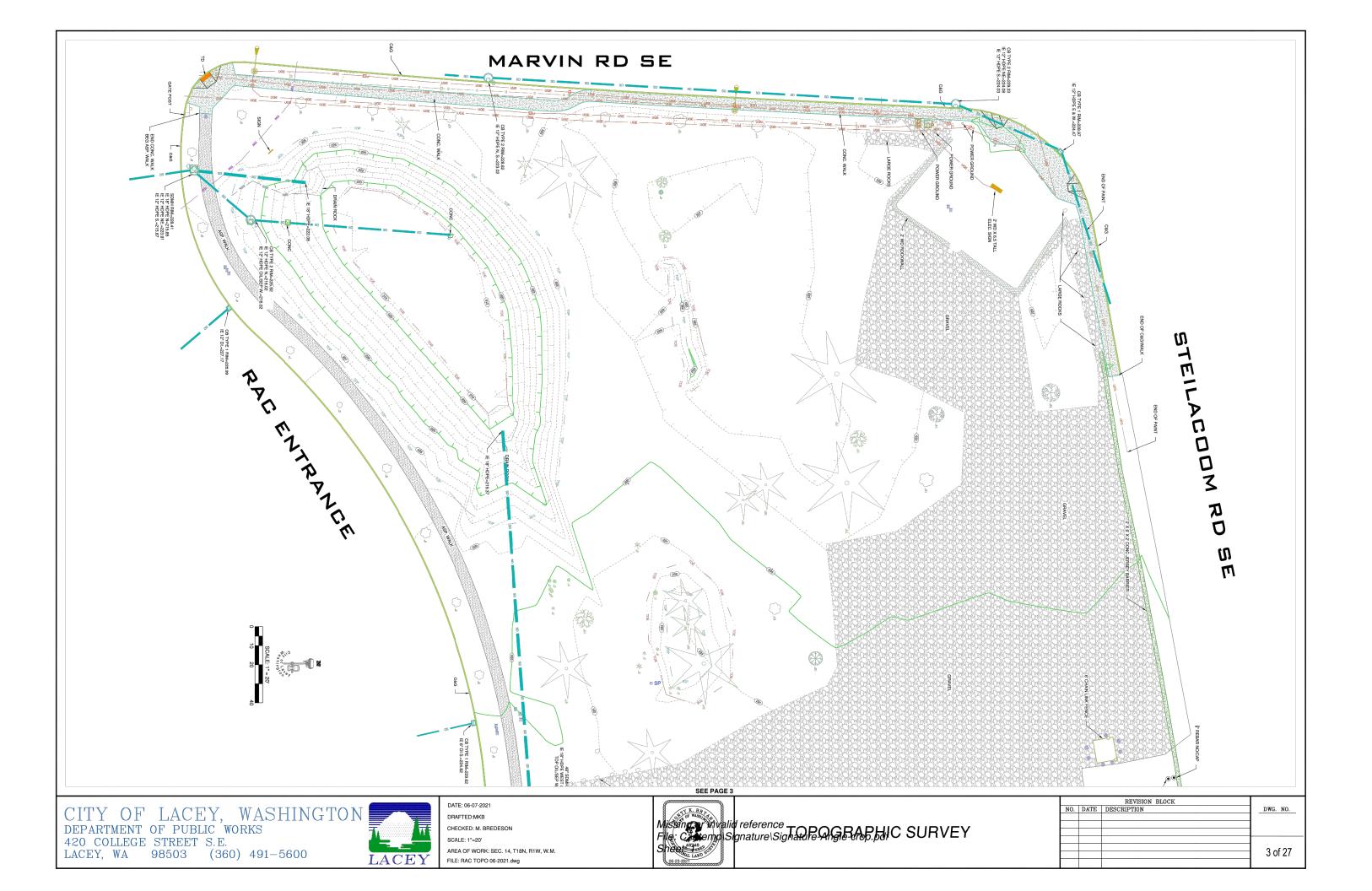
CITY OF LACEY, WASHINGTON DEPARTMENT OF PUBLIC WORKS 420 COLLEGE STREET S.E. LACEY, WA 98503 (360) 491-5600

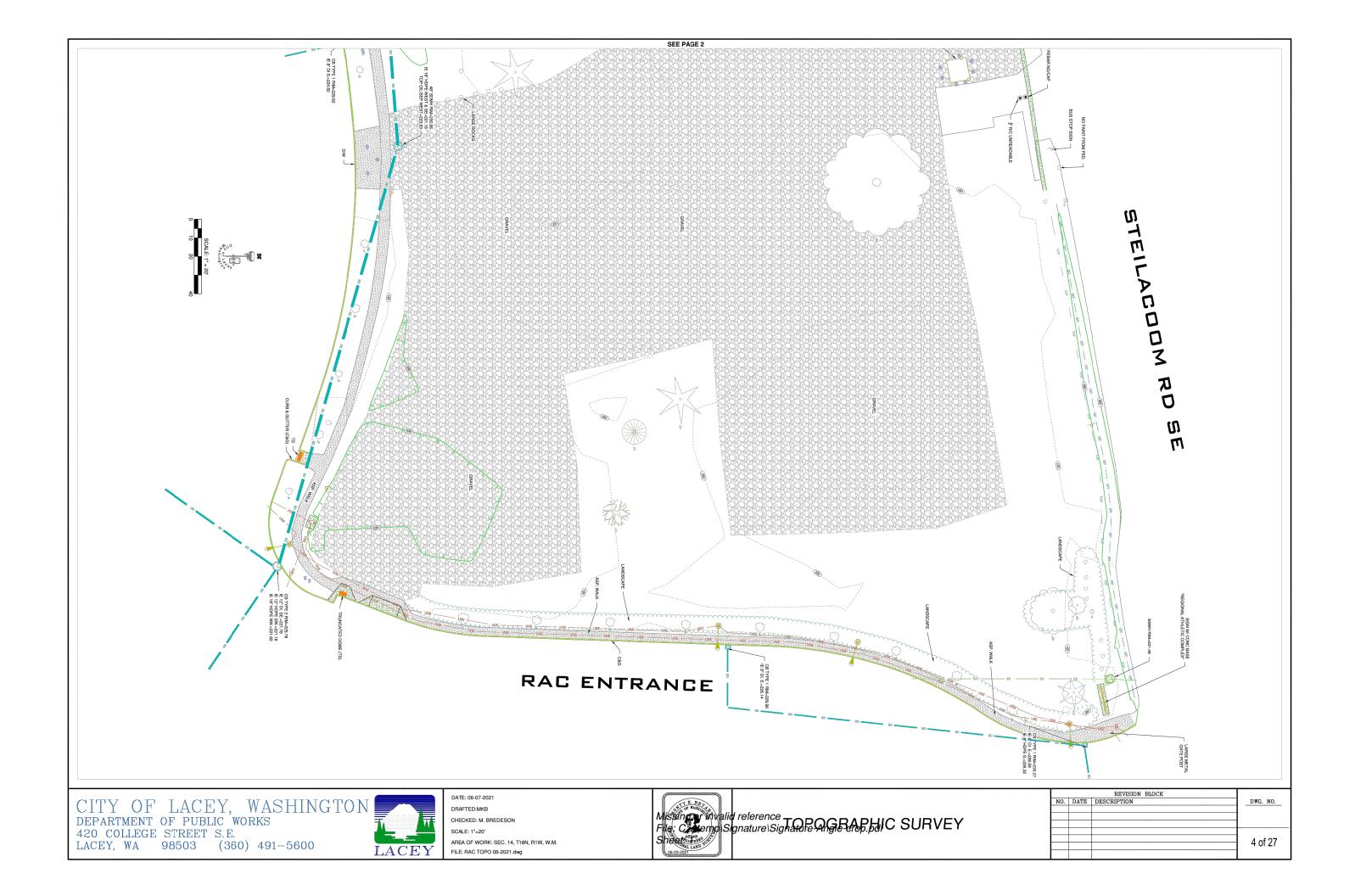


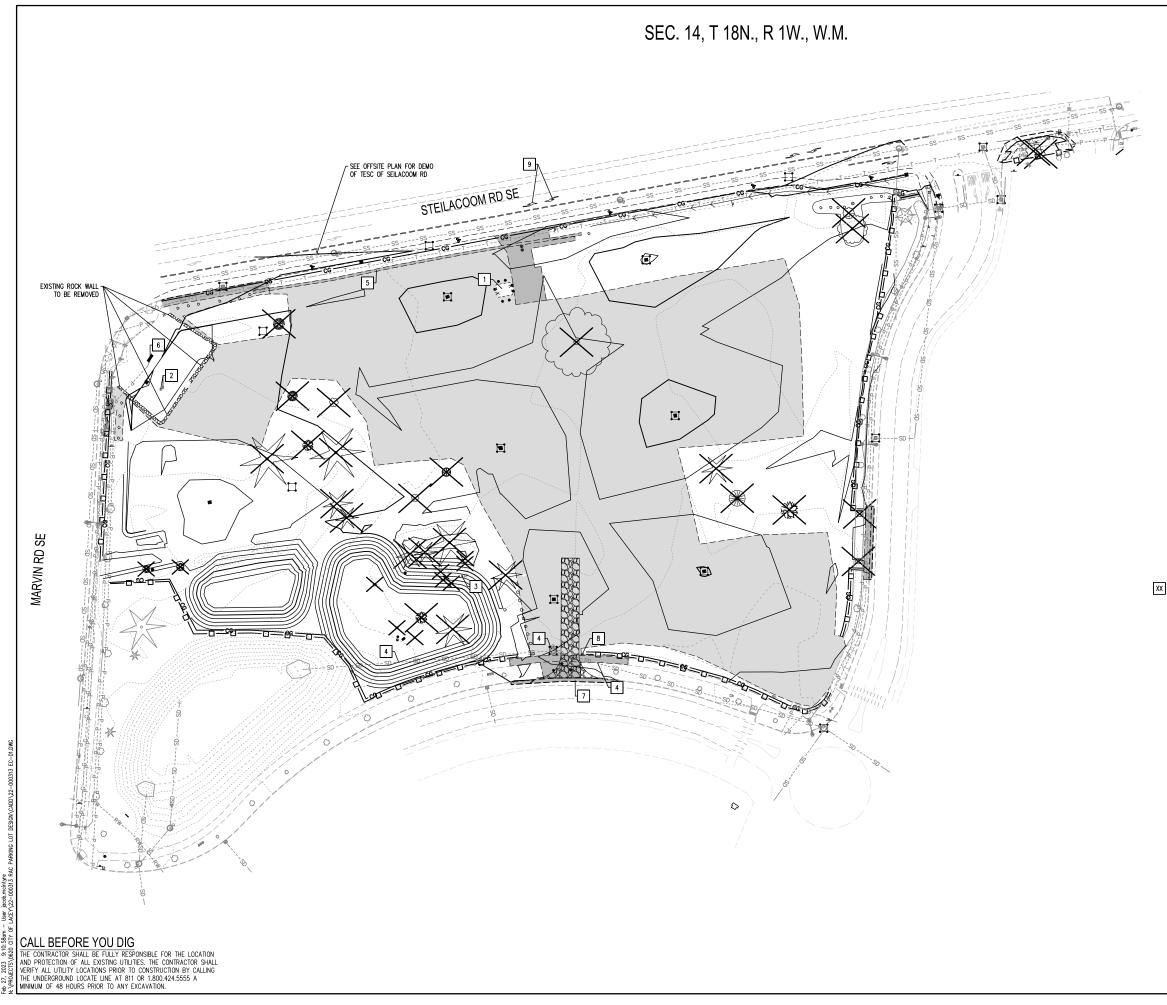
CHECKED: M. BREDESON SCALE: 1"=20' AREA OF WORK: SEC. 14, T18N, R1W, W.M. FILE: RAC TOPO 06-2021.dwg



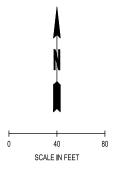




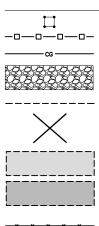




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0343 3 I EILAUOUMI RU SE I ACFY WA	8/30 IALLON LANE NE, SUITE 200, LACEY, WA 36318 P: 360.352.1465 F: 360.352.1509				
	SCJALLIANCE.COM				



LEGEND



INLET SEDIMENT PROTECTION: SEE EC-	-02
SILT FENCE: SEE EC-02	

CLEARING AND GRUBBING LIMITS

STABILIZED CONSTRUCTION ENTRANCE: SEE EC-02

SAWCUT

EXISTING TREE AND ROOT BALL TO BE REMOVED AND BACK FILLED

EXISTING GRAVEL TO BE REMOVED

EXISTING ASPHALT/CONCRETE TO BE REMOVED

TREE PROTECTION FENCING SEE EC-02

- 1. PSE GAS AND ELECTRICAL EQUIPMENT AND ASSOCIATED FENCE AND BOLLARDS SHALL BE REMOVED: CONTRACTOR SHALL COORDINATE WITH PSE
- 2. IRRIGATION VALVES TO BE REMOVED AND RELOCATED: SEE LANDSCAPE PLANS
- 3. SPRINKLER HEAD TO BE REMOVED AND RELOCATED: SEE LANDSCAPE PLANS
- 4. PROTECT-IN-PLACE EXISTING STORM LINE AND STRUCTURES
- 5. REMOVE EXISTING ECOLOGY BLOCKS AND RETURN TO CITY OF LACEY. CONTRACTOR SHALL COORDINATE WITH CITY OF LACEY ON DELIVERY LOCATION
- 6. PROTECT-IN-PLACE DIGITAL MESSAGE BOARD AND ASSOCIATED WIRING
- 7. SAWCUT
- 8. SIGN REMOVAL

DESIGNER: W. HOLM DRAWN BY: K. GANS APPROVED BY: W. HOLM DATE: DECEMBER 2022 JOB NO: 22-000313 DRAWING FILE NO: 22-000314 DRAWING FILE NO: 22-000315 DRAWING FILE NO: 20-01 DRAWIN

CITY OF LACEY STANDARD EROSION CONTROL NOTES

A CERTIFIED EROSION AND SEDIMENT CONTROL LEAD (CESCL) IS REQUIRED FOR ALL CONSTRUCTION PROJECTS. THE NAMED PERSON OR FIRM SHALL BE ON-SITE OR ON-CALL AT ALL TIMES. FOR THIS SITE, THE PERSON/FIRM IS _______ AND THEIR TELEPHONE NUMBERS ARE:

(OFFICF:) (CELL:)____-

APPROVAL OF THIS EROSION & SEDIMENT CONTROL (ESC) PLAN DOES NOT CONSTITUTE AN APPROVAL OF PERMANENT ROAD OR DRAINAGE DESIGN (E.G. SIZE AND LOCATION OF ROADS, PIPES, RESTRICTORS, CHANNELS, RETENTION FACILITIES, UTILITIES, ETC.).

THE IMPLEMENTATION OF THESE ESC PLANS AND THE CONSTRUCTION, MAINTENANCE, REPLACEMENT, AND UPGRADING OF THESE ESC FACILITIES IS THE RESPONSIBILITY OF THE APPLICANT/CONTRACTOR UNTIL ALL CONSTRUCTION IS COMPLETED AND APPROVED AND VEGETATION/LANDSCAPING IS ESTABLISHED.

THE CLEARING LIMIT BOUNDARIES SHOWN ON THIS PLAN SHALL BE CLEARLY FLAGGED IN THE FIELD PRIOR TO CONSTRUCTION. DURING THE CONSTRUCTION OF TIME FLOW STURBANCE BEYOND THE FLOWED IN THE FLOW FINITES SHALL BE PERMITTED. THE FLAGGING SHALL BE MAINTAINED BY THE APPLICANT/CONTRACTOR FOR THE DURATION OF CONSTRUCTION

THE ESC FACILITIES SHOWN ON THIS PLAN MUST BE CONSTRUCTED IN CONJUNCTION WITH ALL CLEARING AND GRADING ACTIVITIES. AND IN SUCH A MANNER AS TO ENSURE THAT SEDIMENT AND SEDIMENT-LADEN WATER DO NOT ENTER THE DRAINAGE SYSTEM, ROADWAYS, OR VIOLATE APPLICABLE SURFACE WATER, GROUND WATER, OR DISCHARGE STANDARDS.

THE ESC FACILITIES SHOWN ON THIS PLAN ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS EVENTS AND TO ENSURE THAT SEDIMENT AND SEDIMENT-LADEN WATER DO NOT LEAVE THE SITE.

THE ESC FACILITIES ON ACTIVE SITES SHALL BE INSPECTED DAILY BY THE APPLICANT/CONTRACTOR--AND MAINTAINED, REPAIRED, OR AUGMENTED AS NECESSARY--TO ENSURE THEIR CONTINUED FUNCTIONING.

THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED MONTHLY AND WITHIN 48 HOURS FOLLOWING A MAJOR STORM EVENT (21"RAINFALL IN 24 HOURS) BY THE APPLICANT/CONTRACTOR — AND MAINTAINED, REPAIRED, OR AUGMENTED AS NECESSARY — TO ENSURE THEIR CONTINUED FUNCTIONING, STORM DRAIN INLETS OPERABLE DURING CONSTRUCTION SHALL BE PROTECTED SO THAT STORMWATER RUNOFF DOES NOT ENTER THE CONVEYANCE SYSTEM WITHOUT FIRST BEING FILTERED OR TREATED TO REMOVE SEDIMENT. AT NO TIME SHALL MORE THAN 1 FOOT OR 1/3 OF THE SUMP VOLUME (WHICHEVER IS LESS) OF SEDIMENT BE ALLOWED TO ACCUMULATE WITHIN A CATCH BASIN. ALL CATCH BASINS AND CONVEYANCE LINES SHALL BE CLEANED PRIOR TO PROJECT COMPLETION AND ACCEPTANCE. THE CLEANING OPERATION SHALL NOT FLUSH SEDIMENT-LADEN WATER OFFSITE WITHOUT TREATMENT.

STABILIZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION AND MAINTAINED FOR THE DURATION OF THE PROJECT. ADDITIONAL MEASURES MAY BE REQUIRED TO ENSURE THAT ALL PAVED AREAS ARE KEPT CLEAN FOR THE DURATION OF THE PROJECT.

 ROADS SHALL BE CLEANED THOROUGHLY AS NEEDED TO PROTECT DOWNSTREAM WATER RESOURCES OR STORNWATER
INFRASTRUCTURE. SEDIMENT SHALL BE REMOVED FROM ROADS BY SHOVELING OR PICKUP SWEEPING AND SHALL BE TRANSPORTED TO A CONTROLLED SEDIMENT DISPOSAL AREA.

FROM OCTOBER 1 THROUGH APRIL 30, NO SOILS SHALL REMAIN EXPOSED AND UNWORKED FOR MORE THAN 2 DAYS. FROM WAY TO SEPTEMBER 30, NO SOLS SHALL REMAIN EXPOSED AND UNWORKED FOR MORE THAN 2 DATS. FROM MAY 1 TO SEPTEMBER 30, NO SOLS SHALL REMAIN EXPOSED AND UNWORKED FOR MORE THAN 7 DAYS. SOLS SHALL BE STABILIZED AT THE END OF THE SHIFT BEFORE A HOLDAY OR WEEKEND IF NEEDED BASED ON THE WEATHER FOREOAST. LINEAR CONSTRUCTION ACTIVITES, SUCH AS RIGHT-OF-WAY AND EASEMENT CLEARING, ROADWAY DEVELOPMENT, IPPELINES, AND TEMOCING FOR UTILITES, SHALL COMPLY WITH THESE REQUIREMENTS. THESE STABILIZATION REQUIREMENTS APPLY TO ALL SOLS ON SITE, WHETHER AT FINAL GRADE OR NOT. THE LOCAL PERMITTING AUTHORITY MAY ADJUST THESE TIME LIMITS IF IT CAN BE SHOWN THAT A DEVELOPMENT SITE'S EROSION OR RUNOFF POTENTIAL JUSTIFIES A DIFFERENT STANDARD

FROM OCTOBER 1 THROUGH APRIL 30, CLEARING, GRADING, AND OTHER SOIL-DISTURBING ACTIVITIES SHALL ONLY BE FRMITED IF SHOWN TO THE SATISFACTOR OF THE LOCAL PERMITTING AUTHORITY THAT THE TRANSPORT OF SEDIMEN FROM THE CONSTRUCTION SITE TO RECEIVING WATERS WILL BE PREVENTED.

SOIL STOCKPILES MUST BE STABILIZED AND PROTECTED WITH SEDIMENT-TRAPPING MEASURES.

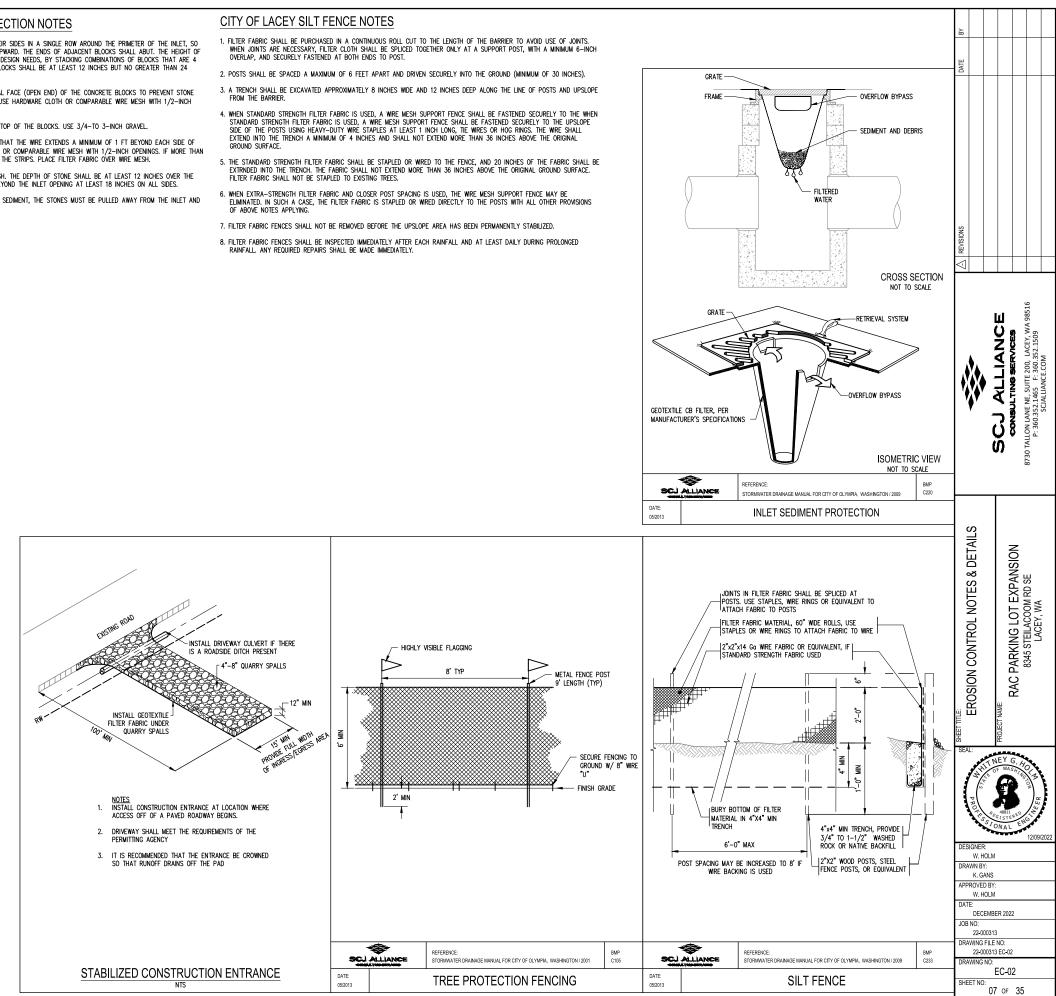
 ALL POLLUTANTS, INCLUDING WASTE MATERIALS AND DEMOLITION DEBRIS, THAT OCCUR ON SITE DURING
CONSTRUCTION SHALL BE HANDLED AND DISPOSED OF IN A MANNER THAT DOES NOT CAUSE CONTAMINATION OF STORMWATER. WOODY DEBRIS MAY BE CHOPPED AND SPREAD ON SITE.

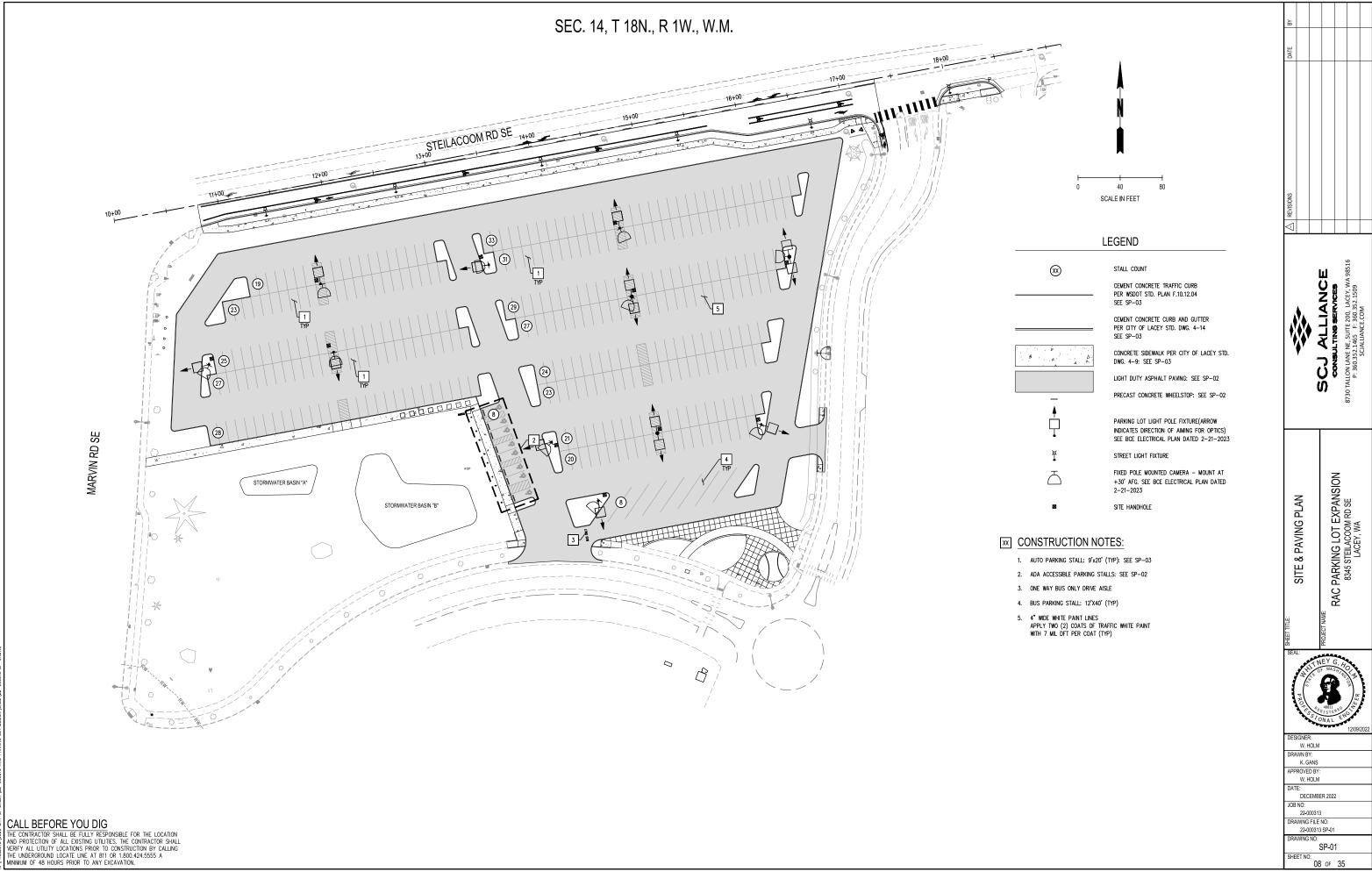
 MAINTENANCE AND REPAIR OF HEAVY EQUIPMENT AND VEHICLES AND OTHER ACTIVITIES WHICH MAY RESULT IN DISCHARGE OR SPILLAGE OF POLLUTANTS TO THE GROUND OR INTO STORMWATER RUNOFF MUST BE CONDUCTED USING SPILL PREVENTION MEASURES, SUCH AS DRIP PANS. REPORT ALL SPILLS TO 911

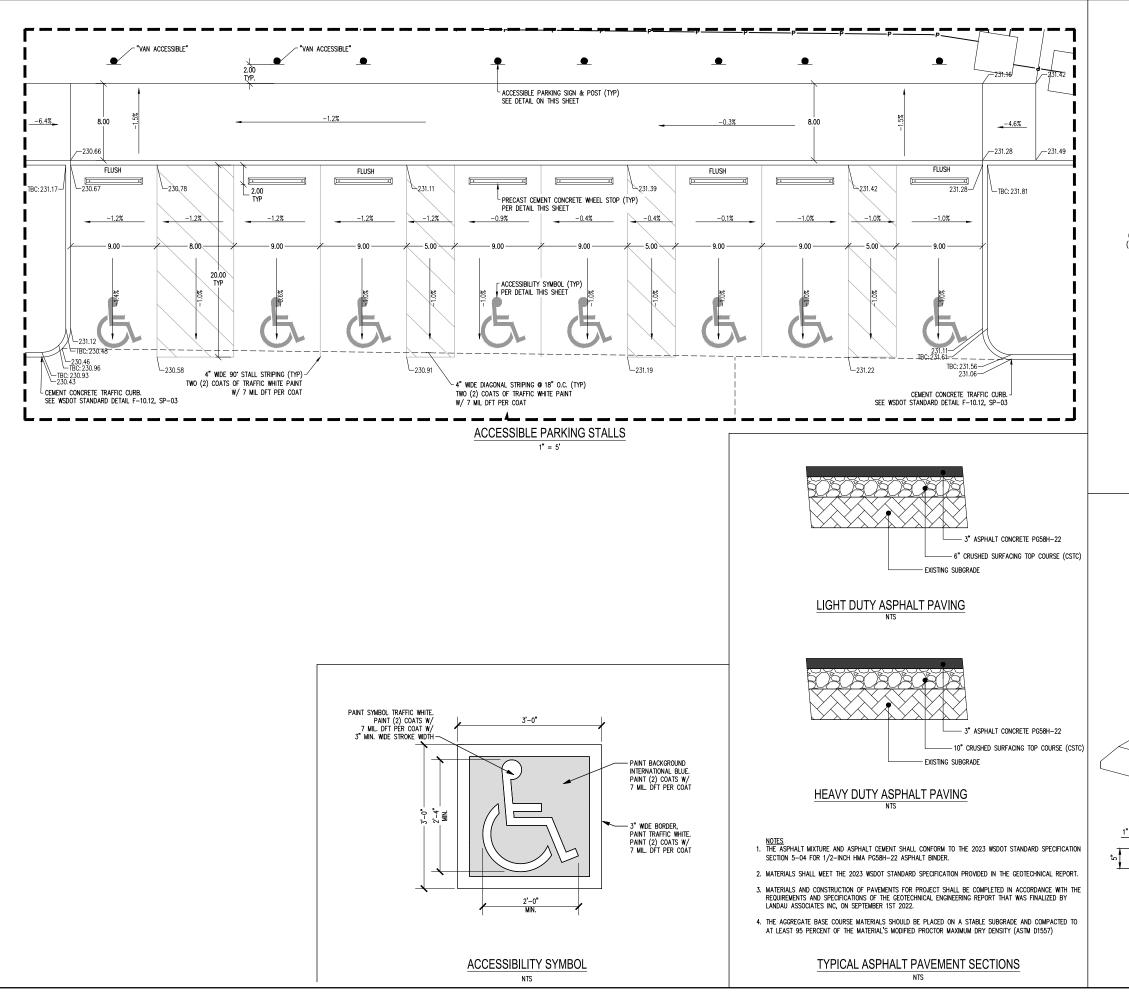
WATER FROM MOST DEWATERING OPERATIONS SHALL BE DISCHARGED INTO A SEDIMENT TRAP OR POND. CLEAN. NON-TURBID WATER MAY BE DISCHARGED TO STATE SURFACE WATERS, RROWDED THE DISCHARGE DOES NOT CAUSE EROSION OR FLOCOING, HIGHLY TURBID OR CONTAMINATED DEWATERN, WATER FROM CONSTRUCTION EQUIPMENT OPERATION, CLAMSHELL DIGGING, CONCRETE TREMIE POUR, OR WORK INSIDE A COFFERDAM SHALL BE HANDLED SEPARATELY FROM STORWWATER AND PROPERLY DISPOSED.

- PLACE CONCRETE BLOCKS LENGTHWISE ON THEIR SIDES IN A SINGLE ROW AROUND THE PRIMETER OF THE INLET, SO THAT THE OPEN ENDS FACE OUTWARD, NOT UPWARD. THE ENDS OF ADJACENT BLOCKS SHALL ABUT. THE HEIGHT OF THE BARRIER CAN BE VARIED, DEPENDINGE ON DESIGN INEEDS, BY STACKING COMBINATIONS OF BLOCKS THAT ARE 4 INCHES, AND 12 INCHES WIDE, THE ROW OF BLOCKS SHALL BE AT LEAST 12 INCHES BUT NO GREATER THAN 24 INCHES HIGH
- 2. PLACE WIRE MESH OVER THE OUTSIDE VERTICAL FACE (OPEN END) OF THE CONCRETE BLOCKS TO PREVENT STONE FROM BEING WASHED THROUGH THE BLOCKS. USE HARDWARE CLOTH OR COMPARABLE WIRE MESH WITH 1/2-INCH
- 3. PILE STONE AGAINST THE WIRE MESH TO THE TOP OF THE BLOCKS. USE 3/4-TO 3-INCH GRAVEL.
- 4. PLACE WIRE MESH OVER THE DROP INLET SO THAT THE WIRE EXTENDS A MINIMUM OF 1 FT BEYOND EACH SIDE OF THE INLET STRUCTURE, USE HARDWARE CLOTH OR COMPARABLE WIRE MESH WITH 1/2-INCH OPENINGS. IF MORE THAN ONE STRIP OF MESH IS NECESSARY, OVERLAP THE STRIPS. PLACE FILTER FABRIC OVER WIRE MESH.
- 5. PLACE 3/4-INCH GRAVEL OVER THE WIRE MESH. THE DEPTH OF STONE SHALL BE AT LEAST 12 INCHES OVER THE ENTIRE INLET OPENING. EXTEND THE STONE BEYOND THE INLET OPENING AT LEAST 18 INCHES ON ALL SIDES.
- 6. IF THE STONE FILTER BECOMES CLOGGED WITH SEDIMENT, THE STONES MUST BE PULLED AWAY FROM THE INLET AND CLEANED OR REPLACED

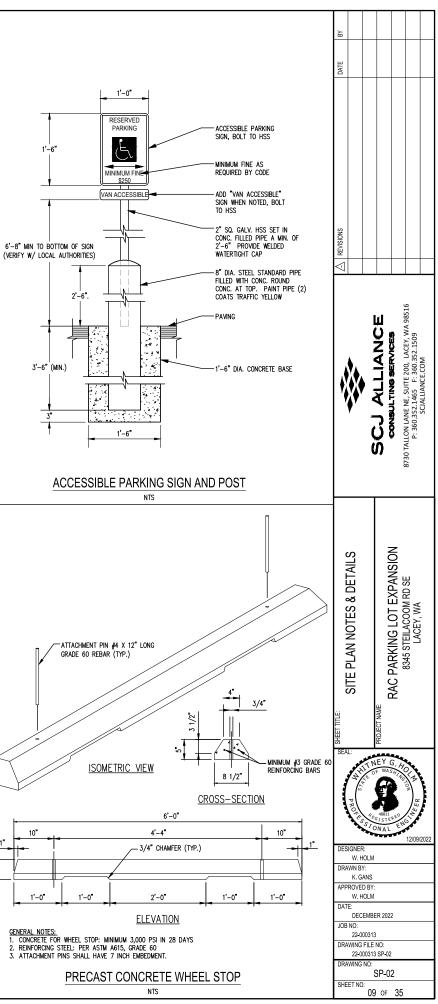
- WHEN JOINTS ARE NECESSARY, FILTER CLOTH SHALL BE SPLICED TOGETHER ONLY AT A SUPPORT POST, WITH A MINIMUM 6-INCH OVERLAP, AND SECURELY FASTENED AT BOTH ENDS TO POST.
- FROM THE BARRIE
- STANDARD STRENGTH FILTER FABRIC IS USED, A WIRE MESH SUPPORT FENCE SHALL BE FASTENED SECURELY TO THE UPSLOPE SIDE OF THE POSTS USING HEAVY-DUTY WIRE STAPLES AT LEAST 1 INCH LONG, TIE WIRES OR HOG RINGS. THE WIRE SHALL EXTEND INTO THE TRENCH A MINIMUM OF 4 INCHES AND SHALL NOT EXTEND MORE THAN 36 INCHES ABOVE THE ORIGINAL GROUND SURFACE.
- OF ABOVE NOTES APPLYING
- RAINFALL. ANY REQUIRED REPAIRS SHALL BE MADE IMMEDIATELY

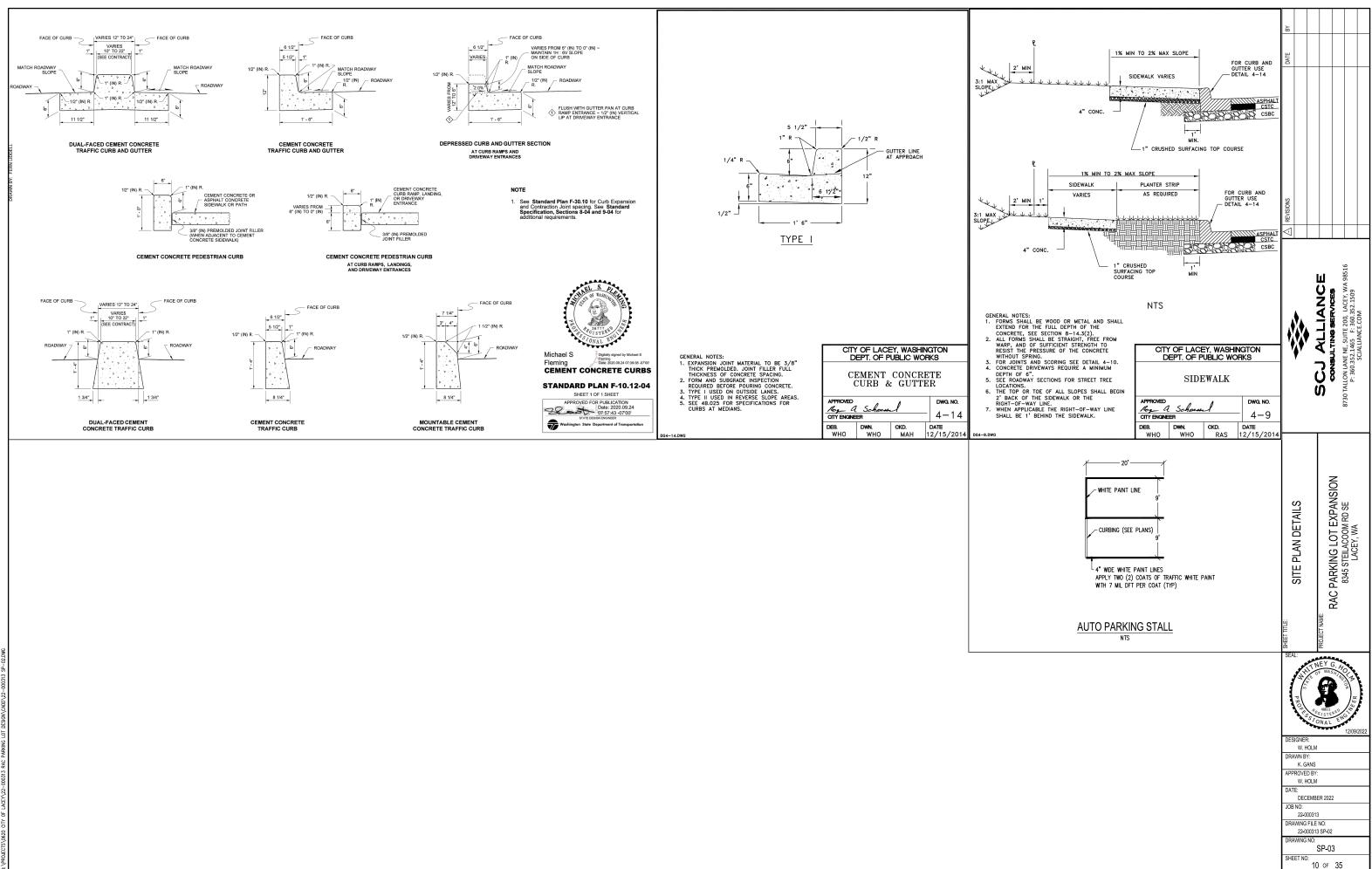


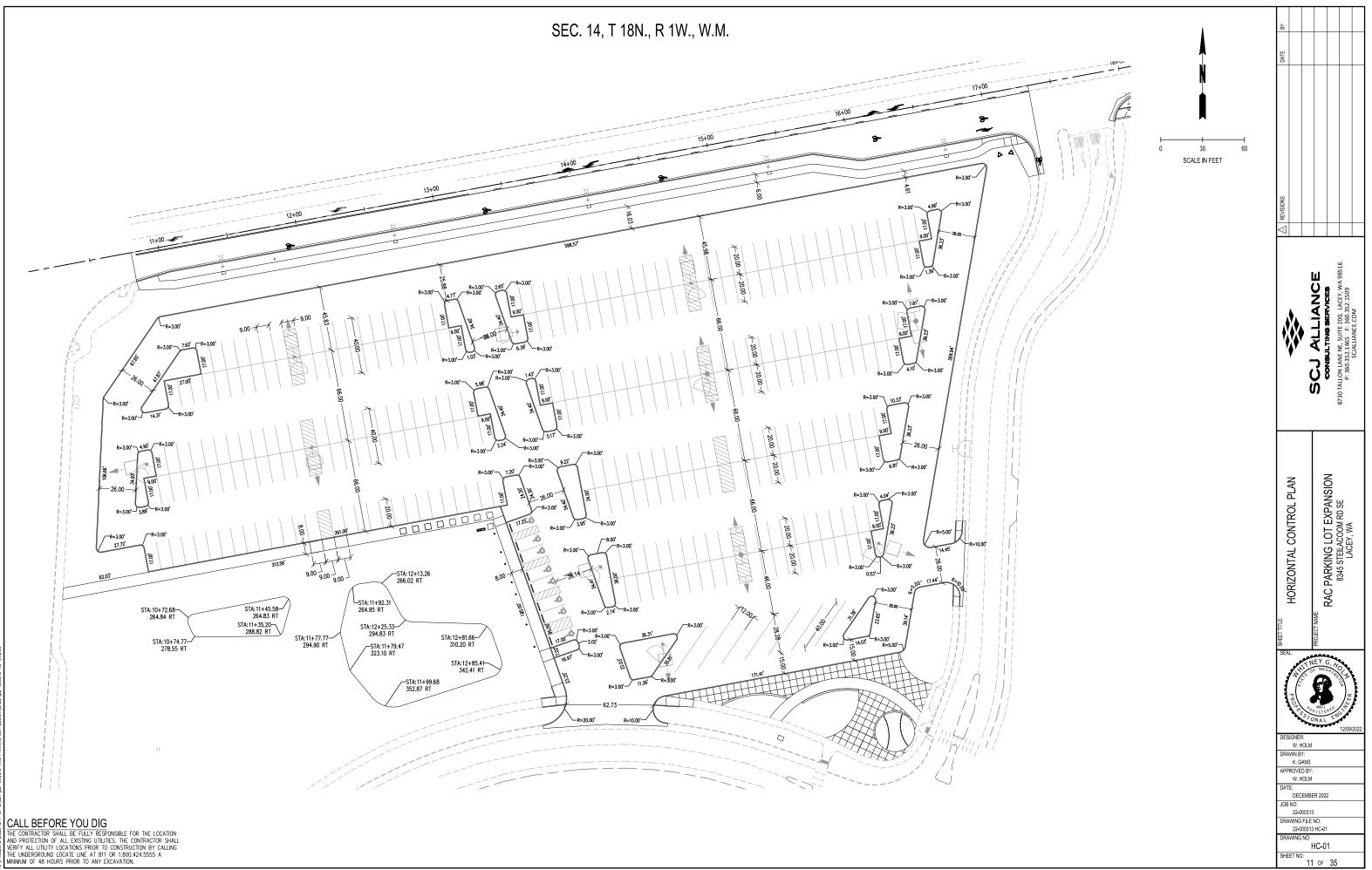




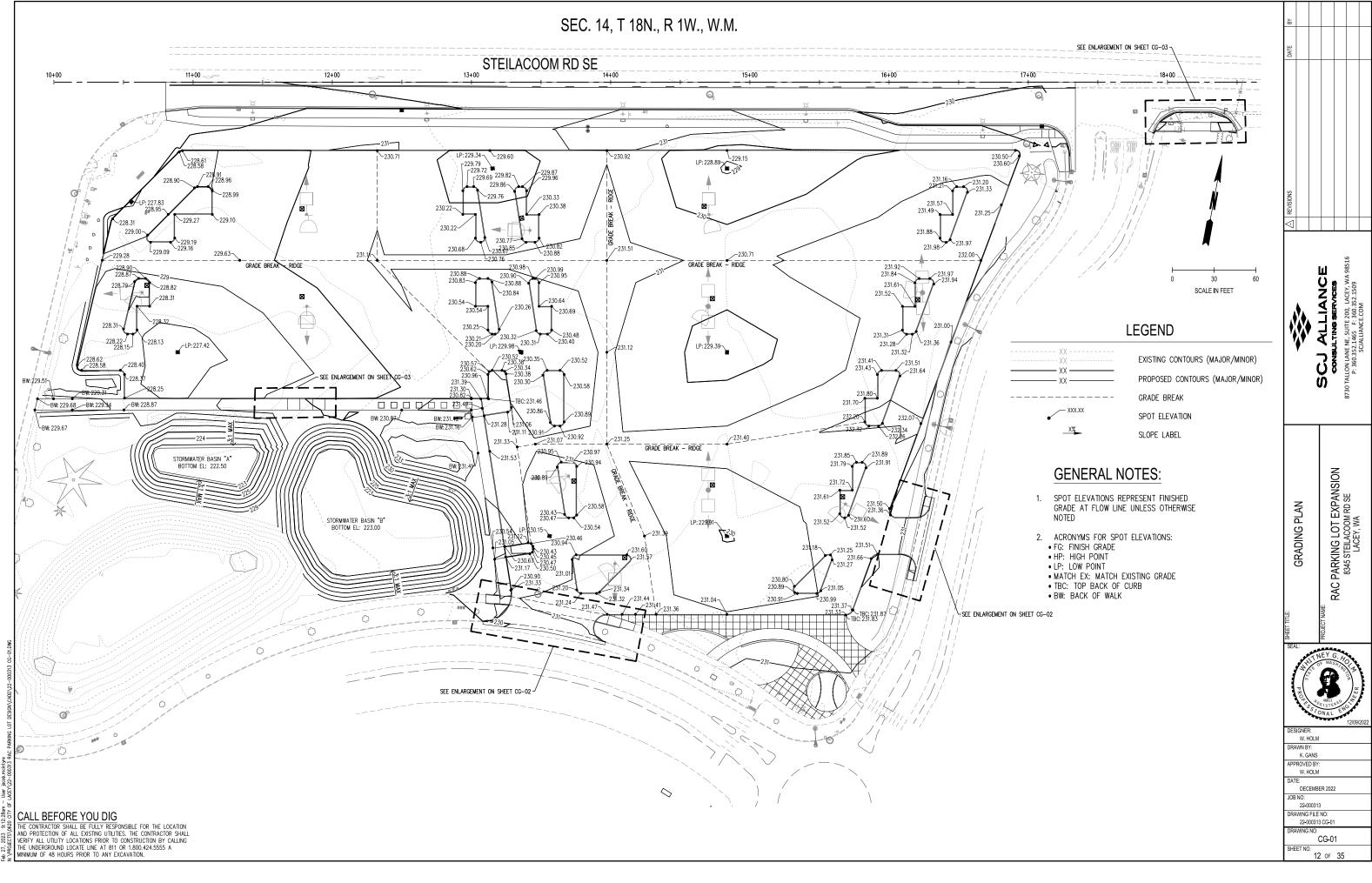
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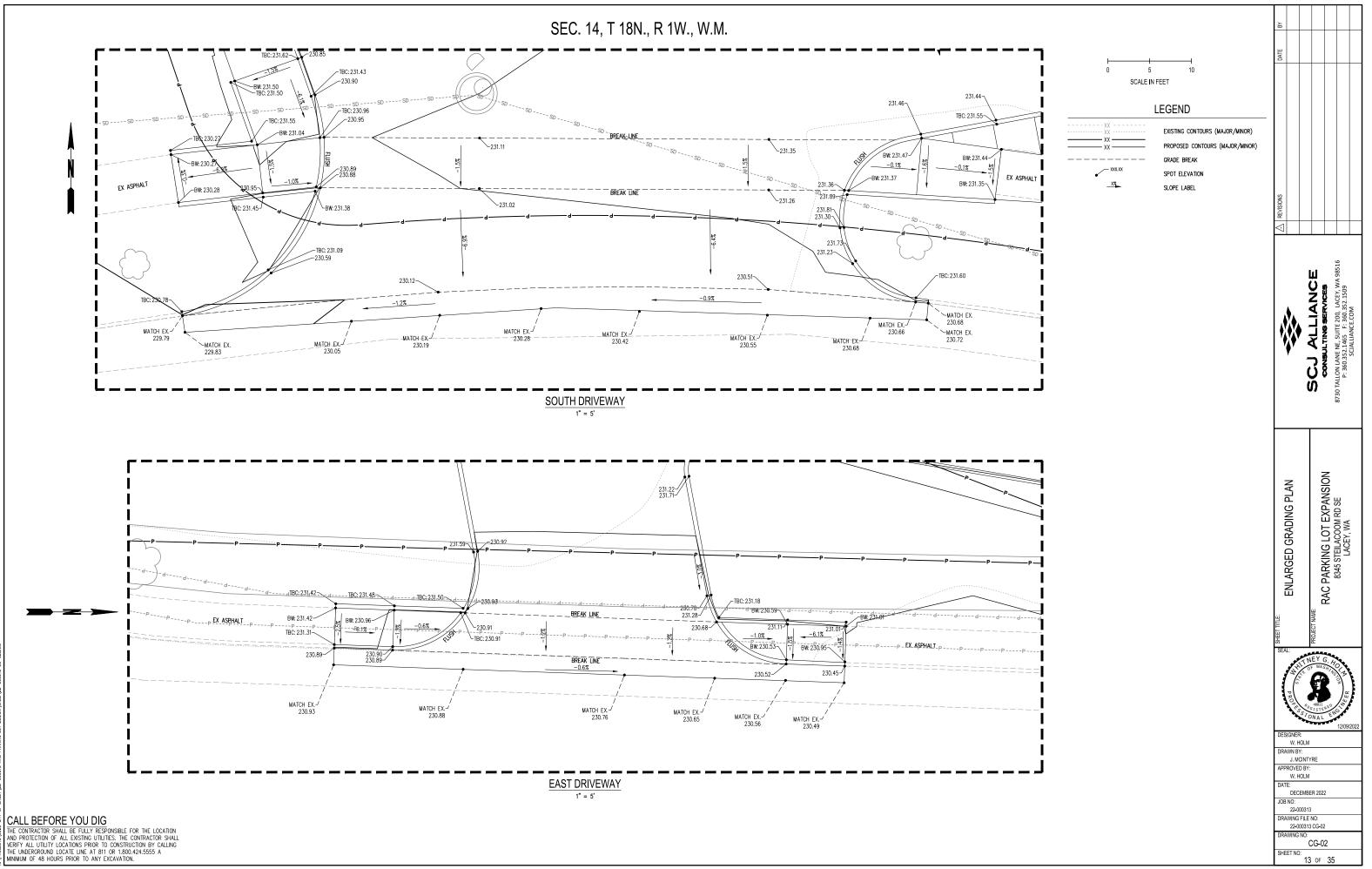




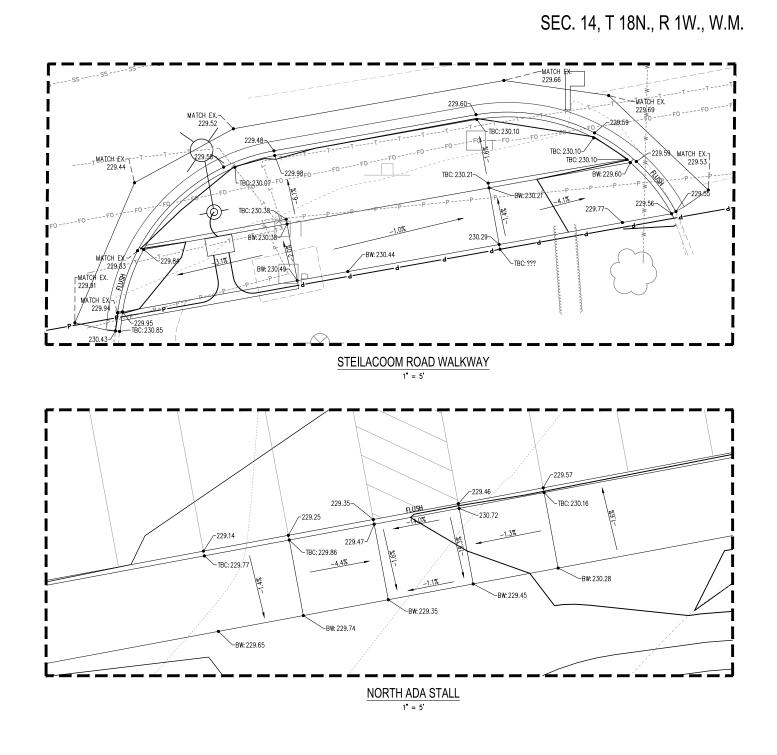


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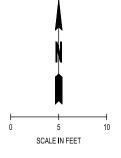




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CALL BEFORE YOU DIG THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR THE LOCATION AND PROTECTION OF ALL EXISTING UTILITIES. THE CONTRACTOR SHALL VERIFY ALL UTILITY LOCATIONS PROR TO CONSTRUCTION BY CALLING THE UNDERGROUND LOCATE LINE AT 811 OR 1.800.424.5555 A MINIMUM OF 48 HOURS PRIOR TO ANY EXCAVATION.



LEGEND

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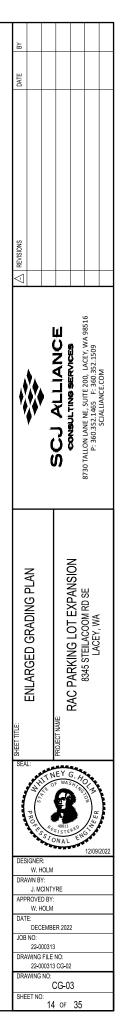
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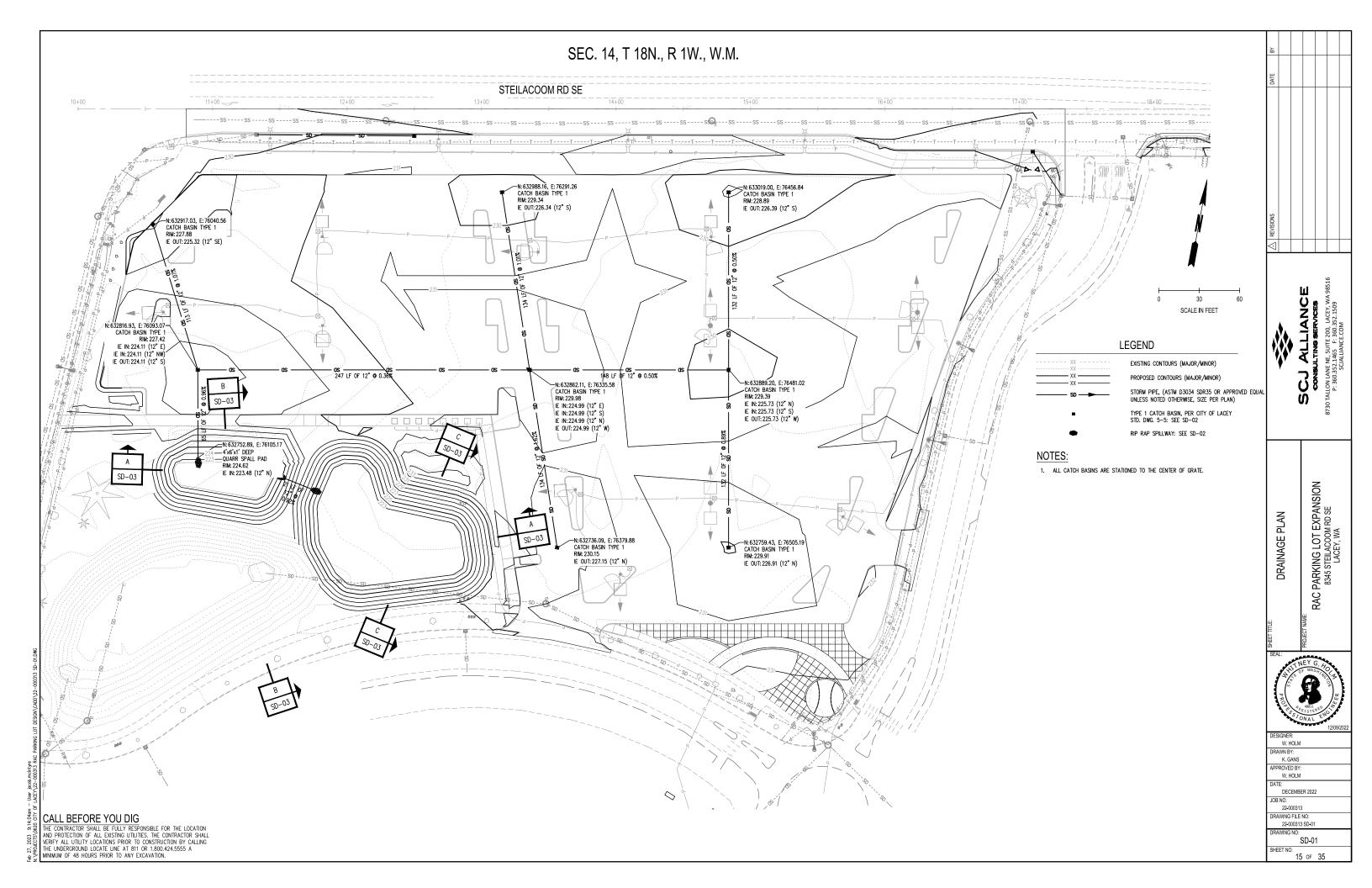
EXISTING CONTOURS (MAJOR/MINOR) PROPOSED CONTOURS (MAJOR/MINOR) GRADE BREAK SPOT ELEVATION SLOPE LABEL

GENERAL NOTES:

1. SPOT ELEVATIONS REPRESENT FINISHED GRADE AT FLOW LINE UNLESS OTHERWISE NOTED

ACRONYMS FOR SPOT ELEVATIONS:
 FG: FINISH GRADE
 HP: HIGH POINT
 LP: LOW POINT
 MATCH EX: MATCH EXISTING GRADE
 TBC: TOP BACK OF CURB
 BW: BACK OF WALK



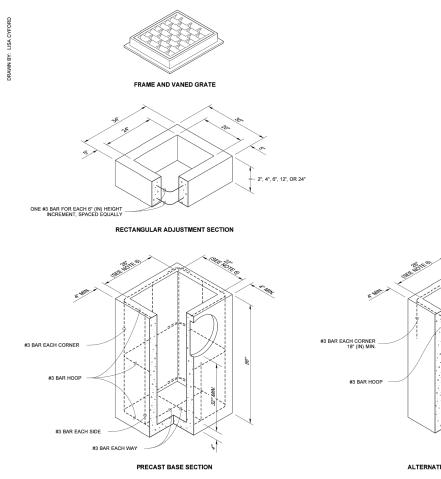


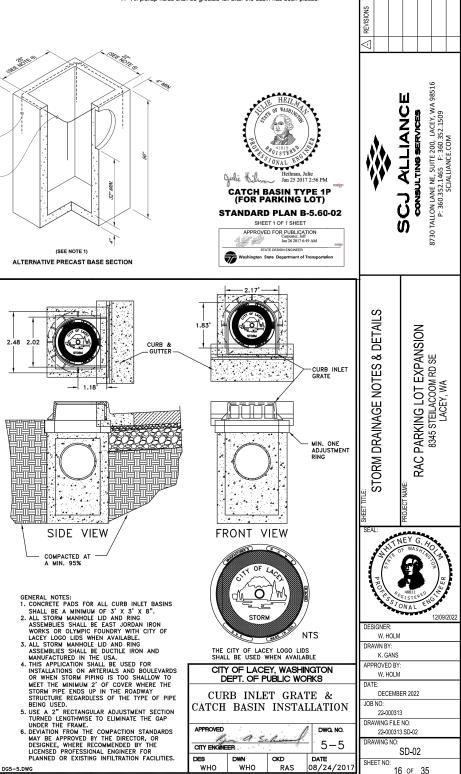
CITY OF LACEY GENERAL STORM DRAIN CONSTRUCTION NOTES

- ALL APPROVALS AND PERMITS REQUIRED BY THE CITY OF LACEY SHALL BE OBTAINED BY THE CONTRACTOR PRIOR TO THE START OF CONSTRUCTION. A GRADING PERMIT FOR STORM POND CONSTRUCTION MAY BE REQUIRED.
 STORM DRAIN PIPE MATERIAL SHALL BE ON THE WISDOT QUALIFIED PRODUCTS LIST FOR THE SPECIFICATION LISTED BELOW AND APPROVED BY THE CITY PRIOR TO INSTALLATION:
- A. PLAIN CONCRETE STORM SEWER PIPE OR REINFORCED CONCRETE STORM SEWER PIPE PER WSDOT STANDARD SPECIFICATION 9-05.7.
- B. SOLID WALL PVC STORM SEWER PIPE PER WSDOT STANDARD SPECIFICATION 9-05.12(1).
- C. DUCTILE IRON SEWER PIPE PER WSDOT STANDARD SPECIFICATION 9-05.13.
- D. HANCOR BLUE SEAL™ AND ADVANCED DRAINAGE SYSTEMS (ADS/HANCOR) N-12 HDPE AND (ADS/HANCOR) SANITITE UP TO 36 INCH IN DIAMFTER PER WEDDT STANDARD SPECIFICATIONS 9-05/20 AND 9-05/24
- E. ADVANCE DRAIMAGE SYSTEMS (ADS) CORRUGATED POLYPROPYLENE PIPE (CPEP) FROM 42" TO 60" IN DIAMETER PER WSDOT 9-05.24 (1) FOR USE NOT IN RIGHT OF WAY.
- F. F. CONTECH DUROMAXX STEEL RIB REINFORCED POLYETHYLENE PIPE, IN DIAMETERS FROM 24 INCH TO 60 INCH PER WSDOT STANDARD SPECIFICATION 9-05.22.
- 3. ALL STORM DRAINAGE SYSTEMS SHALL BE TESTED PER WSDOT STANDARD SPECIFICATION SECTION 7-04.3. TESTING SHALL BE DONE BY THE CONTRACTOR.
- 4. TESTING OF THE STORM SEWER SHALL INCLUDE VIDEO RECORDING OF THE MAIN BY THE CONTRACTOR. IMMEDIATELY PRIOR TO VIDEO RECORDING, ENOUGH WATER SHALL BE RUN DOWN THE LINE SO IT COMES OUT THE LOWER CATCH BASIN. A COPY OF THE VIDEO RECORDING SHALL BE SUBMITTED TO THE CITY OF LACEY. ACCEPTANCE OF THE LINE WILL NOT BE MADE UNTIL AFTER THE RECORDING HAS BEEN REVIEWED AND APPROVED BY THE CITY. TESTING SHALT AKE PLACE AFTER ALL UNDERGROUND UTILITIES ARE INSTALLED AND COMPACTION OF THE ROADWAY SUBGRADE IS COMPLETE.
- SPECIAL STRUCTURES, OIL/WATER SEPARATORS, AND OUTLET CONTROLS SHALL BE INSTALLED PER PLANS AND MANUFACTURERS' RECOMMENDATIONS. WHERE OIL/WATER SEPARATORS ARE CONNECTED TO A SEWER SYSTEM, THEY SHALL BE INSTALLED WITH A P-TRAP OR CHECK VALVE TO PREVENT ODORS.
- 6. ALL DISTURBED AREAS SHALL BE STABILIZED IN ACCORDANCE WITH CORE REQUIREMENT 2 OF THE CURRENT CITY OF LACEY STORWWATER DESIGN MANUAL. FOR SITES WHERE VEGETATION HAS BEEN PLANTED THROUGH HYDROSEEDING, THE FINANCIAL GUARANTEE WILL NOT BE RELEASED UNTIL THE VEGETATION HAS BEEN THOROUGHLY ESTABLISHED.
- 7. WHERE CONNECTIONS REQUIRE "FIELD VERIFICATIONS", CONNECTION POINTS WILL BE EXPOSED BY CONTRACTOR AND FITTINGS VERIFIED 48 HOURS PRIOR TO DISTRIBUTING SHUT-DOWN NOTICES.
- 8. ALL CATCH BASINS/MANHOLES SHALL HAVE PADS PER LACEY STANDARD DETAIL.
- 9. ANY CHANGES TO THE DESIGN SHALL FIRST BE REVIEWED AND APPROVED IN WRITING BY THE PROJECT ENGINEER AND THE CITY OF LACEY.
- 10. ALL STORM PIPE SHALL BE A MINIMUM OF 12-INCH DIAMETER FOR MAINS AND 8- INCH DIAMETER FOR LATERALS CROSSINGS. WHEN PRIVATE STORMWATER (LE. ROOF, LOT, OR FOOTING DRAINS) CANNOT BE INFILTRATED ON INDIVIDUAL LOTS, THE MINIMUM STANDARD PIPING CONNECTION TO THE PUBLIC SYSTEM SHALL BE 8- INCH PVC. THE 8-INCH MAIN USED FOR CONNECTION SHALL BEGIN AT THE RIGHT OF-WAY. THE CONNECTION TO THE CATCH BASIN OR MANHOLE SHALL BE CORED.
- 11. ALL STORM MAINS AND STORWWATER TREATMENT AND/OR FLOW CONTROL BMPS/FACILITIES AREAS SHALL BE STAKED FOR GRADE AND ALIGNMENT BY AN ENGINEERING OR SURVEY FIRM LICENSED TO PERFORM SUCH WORK.
- 12. THE MINIMUM STAKING OF STORM SEWER SYSTEMS SHALL BE AS FOLLOWS:
- A. STAKE LOCATION OF ALL CATCH BASINS, MANHOLES, AND OTHER FIXTURES FOR GRADE AND ALIGNMENT.
- B. STAKE LOCATION, SIZE, AND DEPTH OF STORMWATER TREATMENT AND/OR FLOW CONTROL BMPS/FACILITIES.
- C. STAKE FINISHED GRADE OF ALL STORMWATER FEATURES, INCLUDING BUT NOT LIMITED TO CATCH BASIN/MANHOLE RIM ELEVATIONS, OVERFLOW STRUCTURES, WEIRS, AND INVERT ELEVATIONS OF ALL PIPES IN CATCH BASINS, MANHOLES, AND PIPES THAT DAYLIGHT.
- 13. PIPE SIZE, SLOPE, COVER, ETC., SHALL BE AS SPECIFIED IN THE CITY OF LACEY DEVELOPMENT GUIDELINES AND PUBLIC WORKS STANDARDS.
- 14. ALL DRIVEWAY CULVERTS SHALL BE OF SUFFICIENT LENGTH TO PROVIDE A MINIMUM 3:1 SLOPE FROM THE EDGE OF THE DRIVEWAY TO THE BOTTOM OF THE DITCH AT THE CULVERT END. CULVERTS SHALL HAVE BEVELED END SECTIONS TO MATCH THE SIDE SLOPE.
- 15. IF DRAINAGE OUTLETS (STUB-OUTS) ARE TO BE PROVIDED FOR EACH INDIVIDUAL LOT, THE STUB-OUTS SHALL CONFORM TO THE FOLLOWING:
- A. EACH OUTLET SHALL BE SUITABLY LOCATED AT THE LOWEST ELEVATION ON THE LOT, SO AS TO SERVICE ALL FUTURE ROOF DOWNSPOUTS, FOOTING DRAINS, DRIVEWAYS, YARD DRAINS, AND ANY OTHER SUFFACE OR SUBSURFACE DRAINS NECESSARY TO RENDER THE LOTS SUITABLE FOR THEIR INTENDED USE. EACH OUTLET SHALL HAVE FREE-FLOWING, POSITIVE DRAINAGE TO AN APPROVED STORM WATER CONVEYANCE SYSTEM OR TO AN APPROVED OUTFALL LOCATION.
- B. OUTLETS ON EACH LOT SHALL BE LOCATED WITH A FIVE-FOOT-HIGH, 2-INCH BY 4-INCH STAKE MARKED "STORM" OR "DRAIN." THE STUB-OUT SHALL WSIBLY EXTEND ABOVE SUFFACE LEVEL AND BE SECURED TO THE STAKE.
- C. DRAINAGE EASEMENTS ARE REQUIRED FOR DRAINAGE SYSTEMS DESIGNED TO CONVEY FLOWS THROUGH INDIVIDUAL LOTS. D. THE DEVELOPER AND/OR CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE LOCATIONS OF ALL STUB-OUT
- CONVEYANCE LINES WITH RESPECT TO THE UTILITIES (E.G., POWER, GAS, TELEPHONE, TELEVISION).
- E. ALL INDIVIDUAL STUB-OUTS SHALL BE PRIVATELY OWNED AND MAINTAINED BY THE LOT HOMEOWNER
- 16. THE STORM DRAINAGE SYSTEM SHALL BE CONSTRUCTED ACCORDING TO APPROVED PLANS ON FILE WITH THE CITY. ANY MATERIAL DEVIATION FROM THE APPROVED PLANS WILL REQUIRE WRITTEN APPROVAL FROM THE CITY AND SHALL BE CORRECTED IN THE AS-BUILT DRAWINGS.
- 17. ALL AREAS SUBJECT TO CLEARING AND GRADING THAT HAVE NOT BEEN COVERED BY IMPERVIOUS SURFACE, INCORPORATED INTO A DRAINAGE FACILITY, OR ENGINEERED AS STRUCTURAL FILL OR SLOPE SHALL BE AMENDED IN ACCORDANCE WITH THE CURRENT CITY OF LACEY STORWATER DESIGN MANULA AND THEN SEEDED, PLANTED, AND MULCHED OR SIMILARY STABILIZED AFTER CONSTRUCTION TO THE SATISFACTION OF THE CITY. FOR SITES WHERE GRASS HAS BEEN PLANTED THROUGH HYDROSEEDING, THE PERFORMANCE BOND WILL NOT BE RELEASED UNTIL THE GRASS HAS BEEN THOROUGHLY ESTABILISED, UNLESS OTHERWISE APPROVED BY THE CITY.
- 18. ALL EROSION CONTROL AND STORWWATER FACILITIES SHALL BE REGULARLY INSPECTED AND MAINTAINED BY THE CONTRACTOR DURING THE CONSTRUCTION PHASE OF THE DEVELOPMENT PROJECT.
- 19. NO FINAL CUT OR FILL SLOPE SHALL EXCEED 2:1 WITHOUT STABILIZATION BY ROCKERY OR BY A STRUCTURAL RETAINING WALL.
- 20. THE PROJECT ENGINEER SHALL VERIFY THE LOCATIONS, WIDTHS, THICKNESSES, AND ELEVATIONS OF ALL EXISTING PAVEMENTS AND STRUCTURES, INCLUDING UTILITIES AND OTHER FRONTAGE IMPROVEMENTS, WHICH ARE TO INTERFACE WITH NEW WORK. THE CONTRACTOR SHALL PROVIDE ALL TRIMMING, CUTING, SAW CUTING, GRADUNG, LEVELING, SLOPING, COATING, AND OTHER WORK, INCLUDING MATERIALS AS NECESSARY TO CAUSE THE INTERFACE WITH EXISTING WORKS TO BE PROPER, WITHOUT CONFLICT, ACCEPTABLE TO THE ENGINEER AND THE CITY, COMPLETE IN PLACE, AND READY TO USE.
- 21. COMPACTION OF ALL FILL AREAS SHALL BE PER CURRENT APWA SPECIFICATIONS. FILL SHALL BE PROVIDED IN 6-INCH MAXIMUM LIFTS AND SHALL BE COMPACTED TO 95 PERCENT OF ITS MAXIMUM RELATIVE DENSITY. DEVATION FROM THIS STANDARD MAY BE APPROVED BY THE DIRECTOR, OR DESIGNEE, WHERE RECOMMENDED BY THE LICENSED PROFESSIONAL ENGINEER FOR PLANNED OR EXISTING INFILTRATION FACILITIES.

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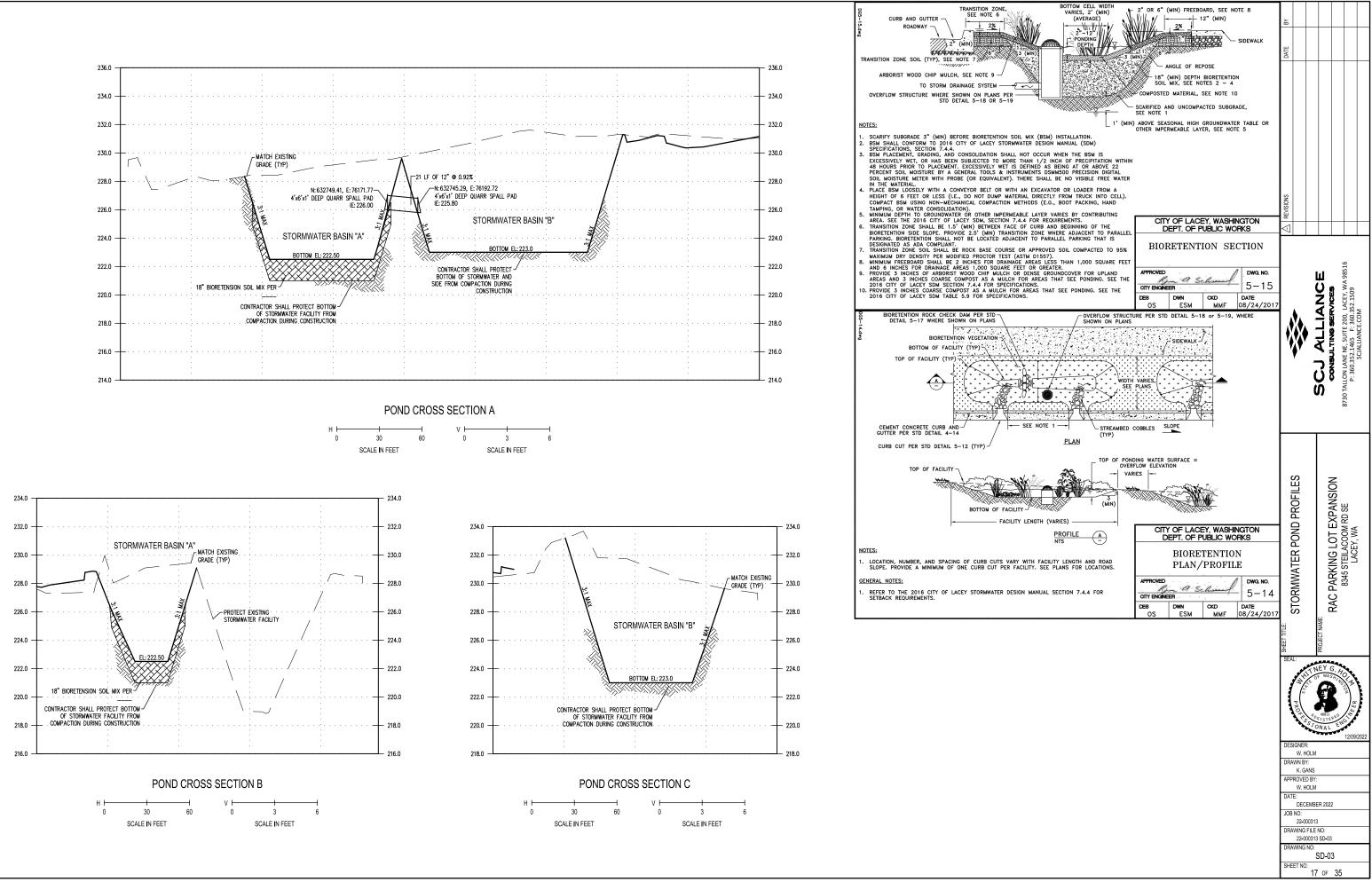
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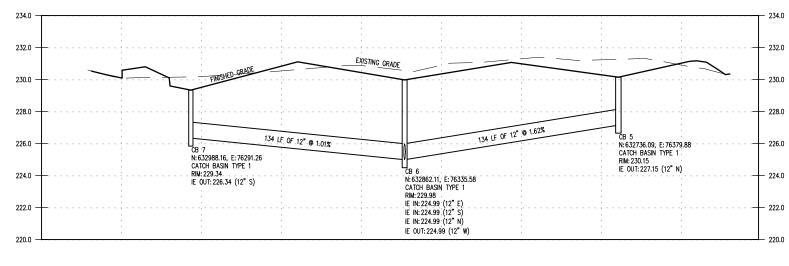




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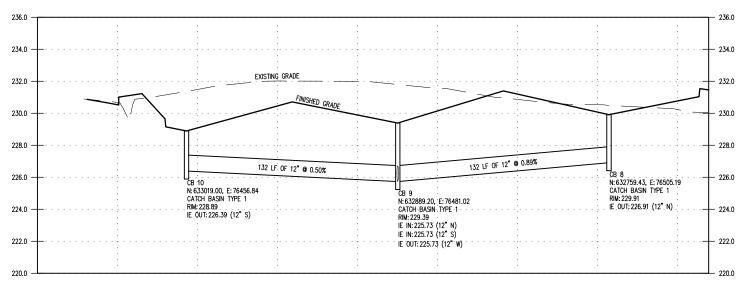
- As acceptable alternatives to the rebar shown in the PRECAST BASE SECTION, fibers (placed according to the Standard Specifications), or wire mesh having a minimum required rebar shown in the ALTERNATIVE PRECAST BASE SECTION. Wire mesh shall not be placed in the knowload.
- 2. The knockout diameter shall not be greater than 18" (in). Knockouts shall have a wall thickness of 2" (in) minimum to 2.5" (in) maximum. Provide a 1.5" (in) minimum gap between the knockout wall and the outside of the pipe. After the pipe is installed, fill the gap with joint mortar in accordance with Standard Specification Section 9-04.3.
- The maximum depth from the finished grade to the lowest pipe invert shall be 5' (ft).
- The frame and grate may be installed with the flange down, or integrally cast into the adjustment section with flange up.
- The Precast Base Section may have a rounded floor, and the walls may be sloped at a rate of 1 : 24 or steeper.
- 6. The opening shall be measured at the top of the Precast Base Section.
- 7. All pickup holes shall be grouted full after the basin has been placed.







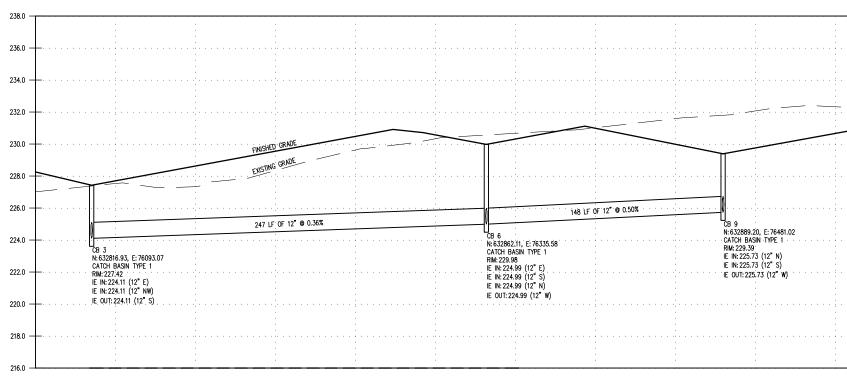


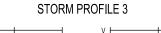


STORM PROFILE 2

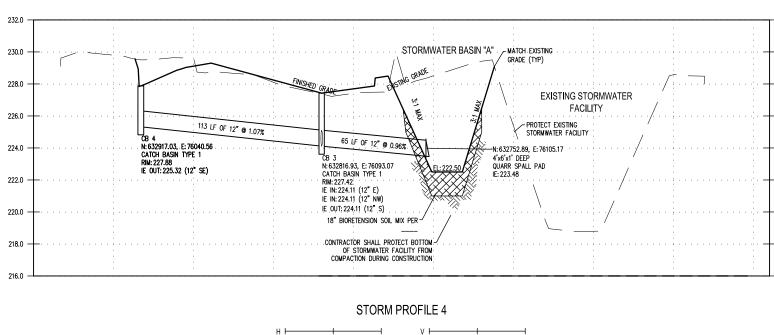


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	SCJ ALLIANCE CONSULTING SERVICES 8730 TALLON LANE NE, SUITE 200, LACEY, WA 98516 P: 560352.1465 F: 360.352.1509 SCIALLIANCE.COM
SHEET TITLE STORM PIPE PROFILES	PROJECT NAME: RAC PARKING LOT EXPANSION 8345 STEILACOOM RD SE LACEY, WA
DESIGNER: W. HOLM DRAWN BY: K. GANS APPROVED BY W. HOLM DATE: DECEMB JOB NO: 22-00031 DRAWING NO: SHEET NO:	: ER 2022 3 NO:









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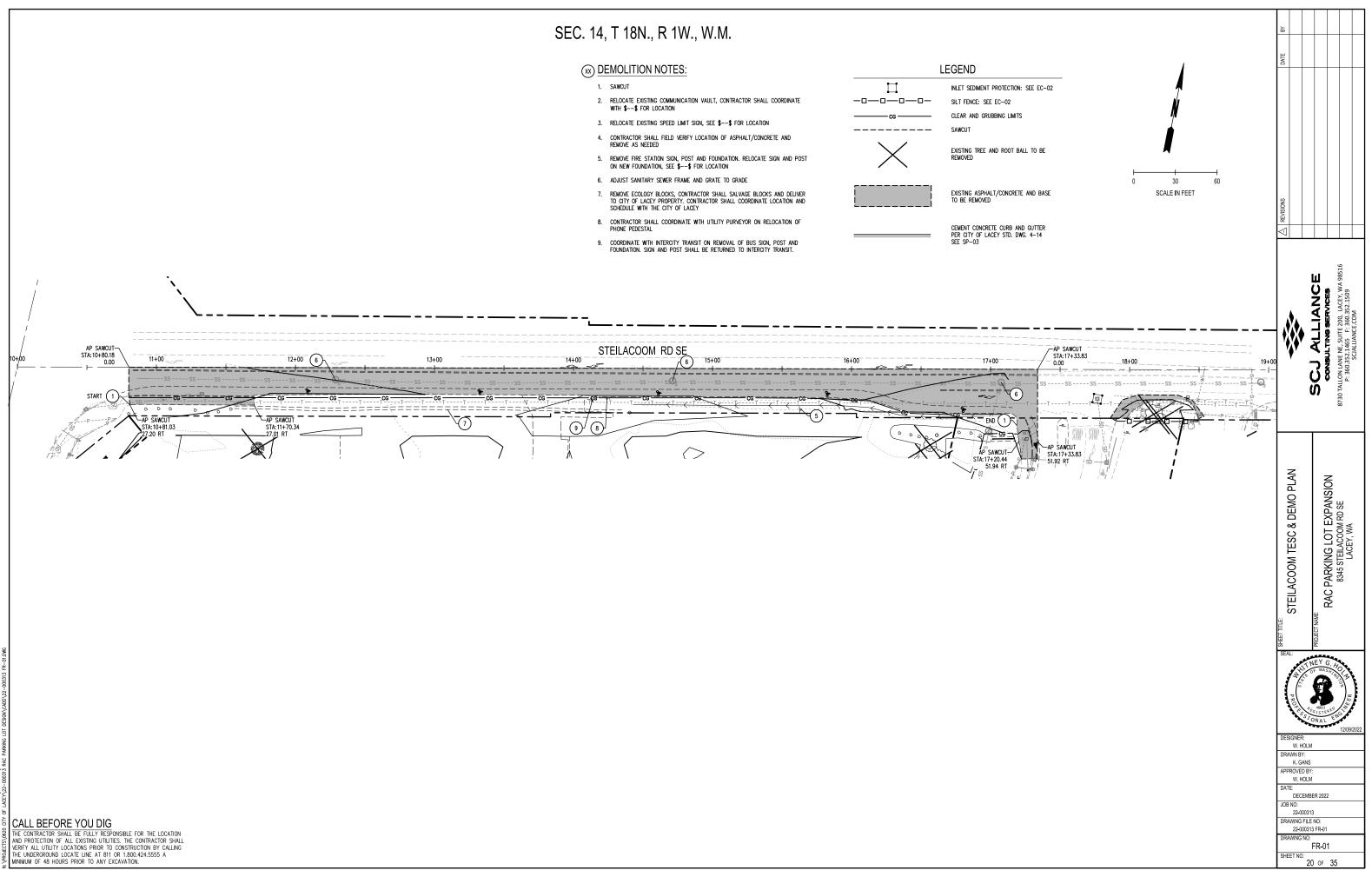
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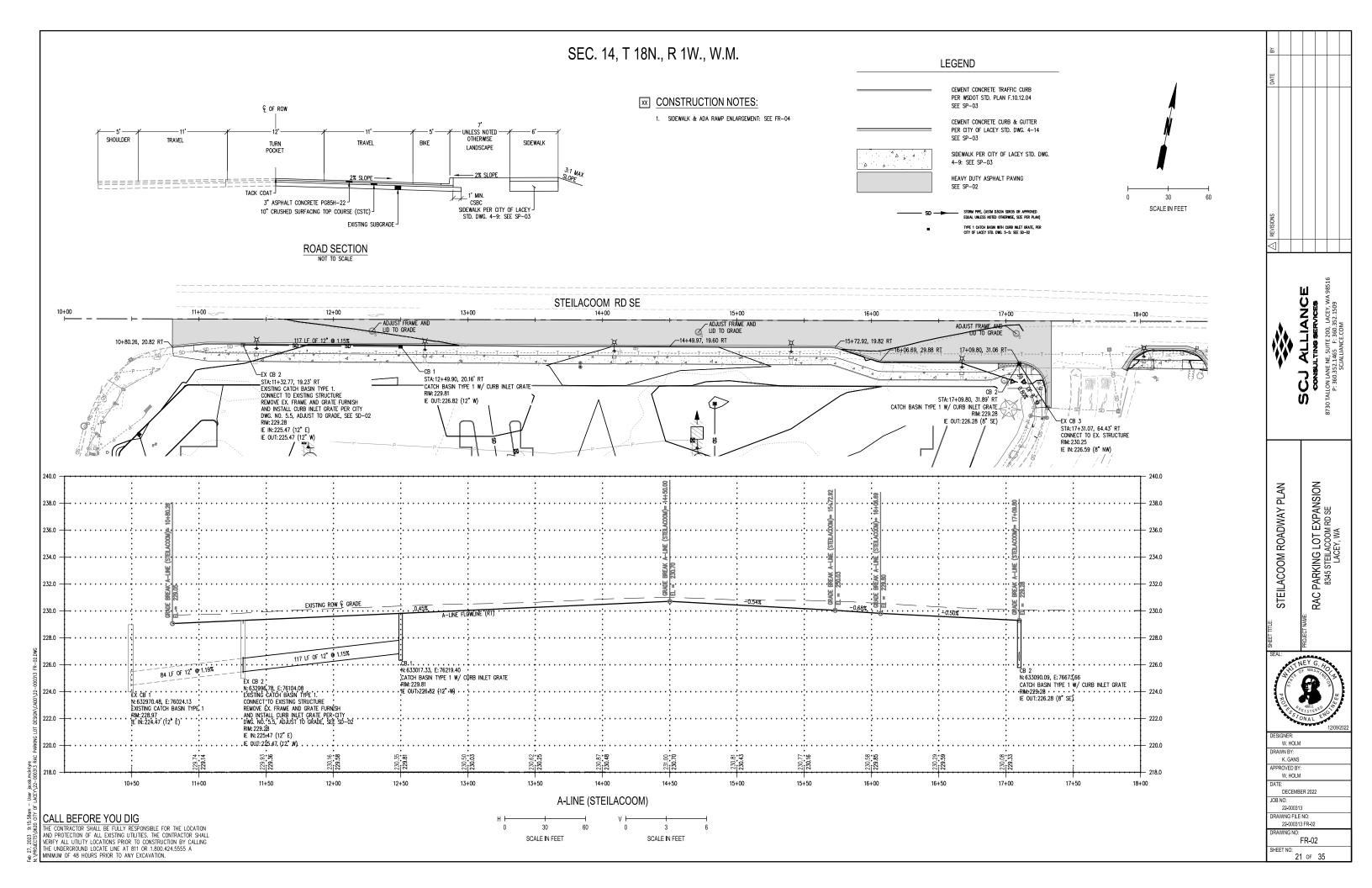
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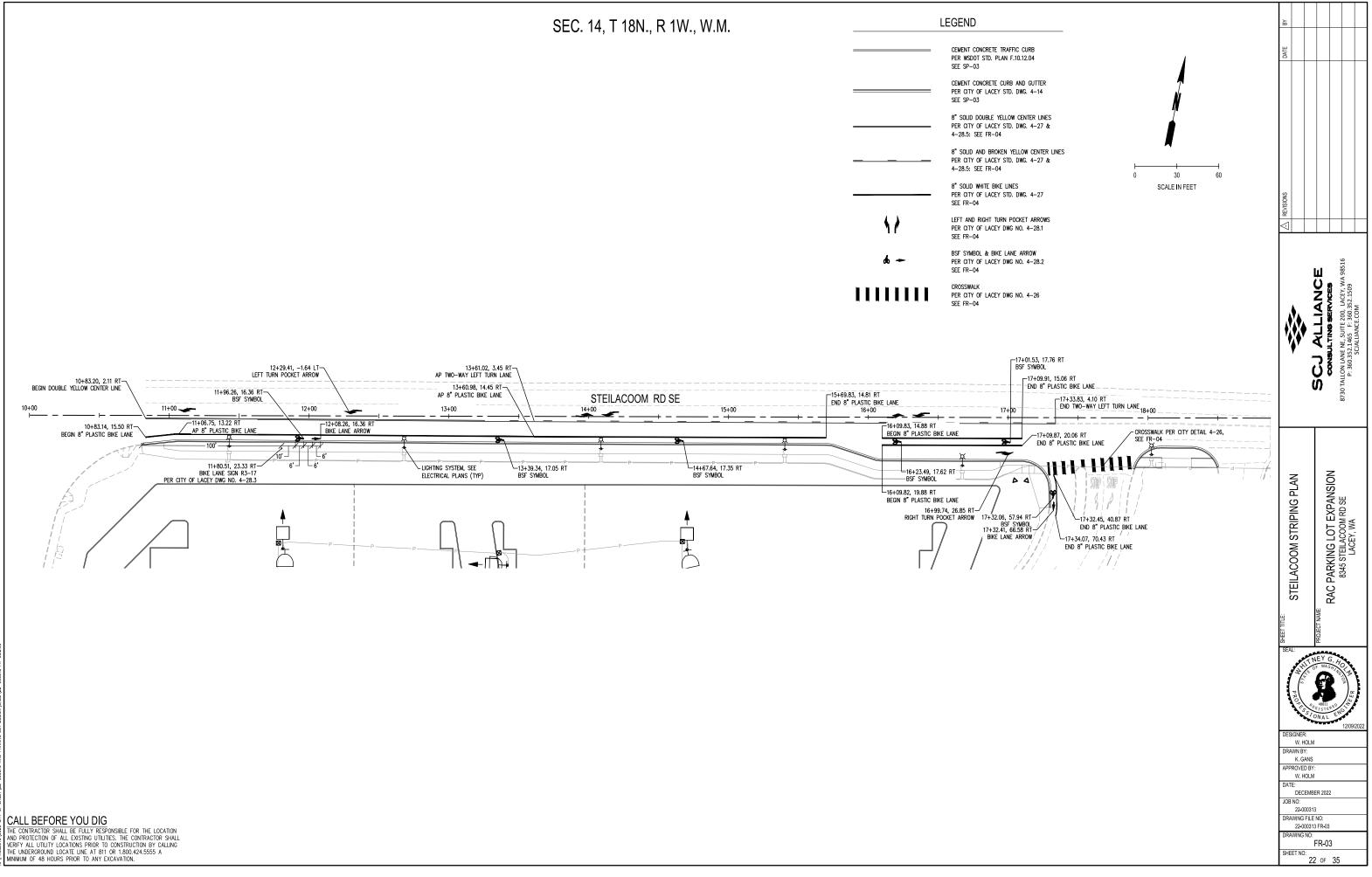
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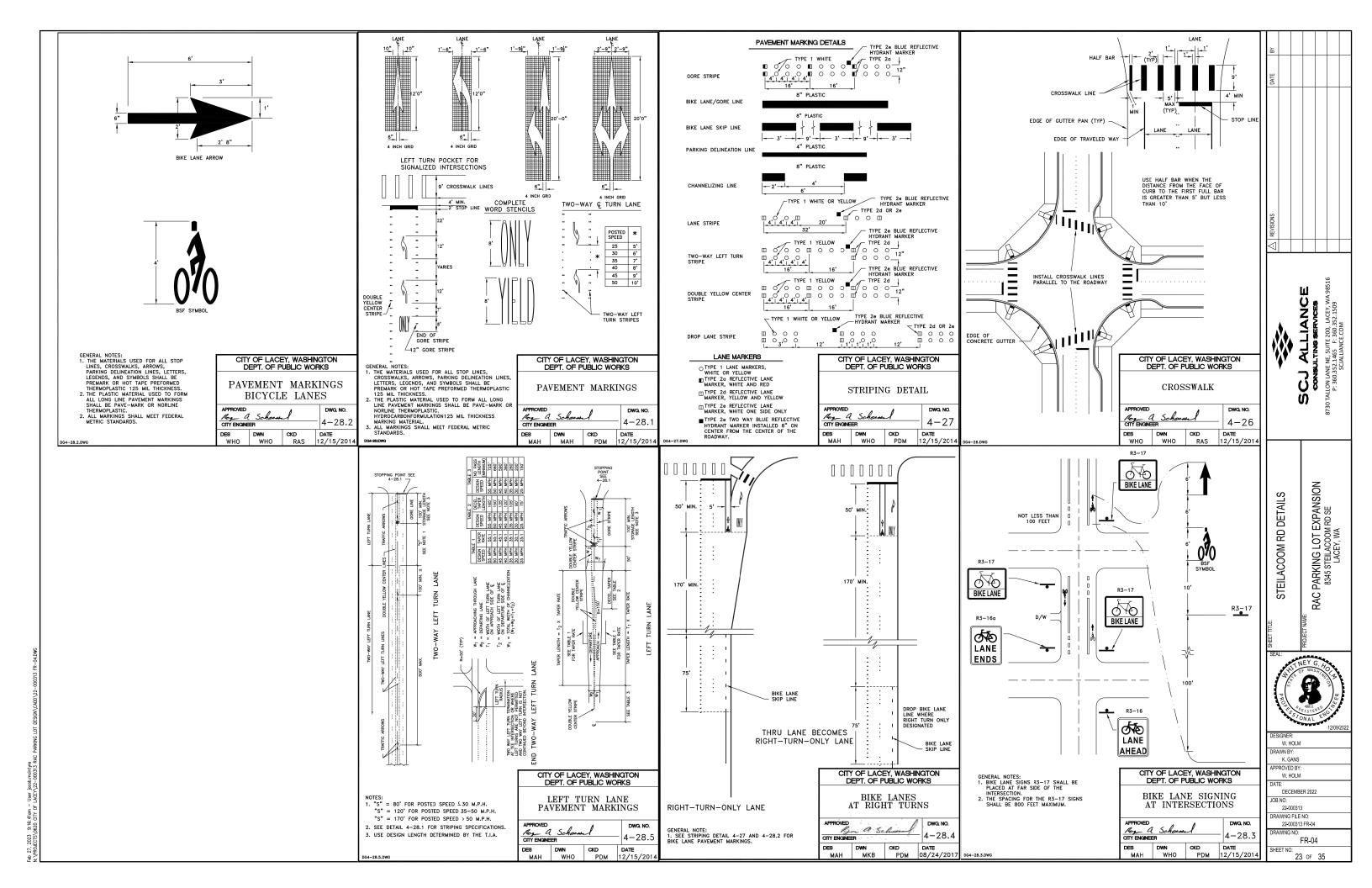
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SHEETTILE STORM PIPE PROFILES	PROJECT NAME: RAC PARKING LOT EXPANSION 8345 STELACOOM RD SE LACEY, WA						
DESIGNER:	HITNEY G. AOLTH HITOF WASK ROOTH PROFESSION						
W. HOLM DRAWN BY: K. GANS							
APPROVED BY W. HOLM DATE:							
JOB NO: 22-00031	DECEMBER 2022 JOB NO: 22-000313						
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SHEET NO:	SD-05 SHEET NO: 19 OF 35						
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Drainage Control Plan Attachment 2

Construction SWPPP Report

Construction Stormwater Pollution Prevention Plan (SWPPP)

Regional Athletic Complex Parking Lot Design Lacey, Washington





Construction SWPPP

Project Information	
Project:	Regional Athletic Complex Parking Lot Design
Site Address:	8323 Steilacoom Rd SE Olympia, WA 98513
Owner/Applicant:	City of Lacey 420 College St SE Lacey, WA 98503 360.413.4340
Contact:	Ashley Smith
Reviewing Agency	
Jurisdiction:	City of Lacey
<u>Project Representative</u> Prepared by:	SCJ Alliance 8730 Tallon Lane NE, Suite 200
	Lacey, WA 98516 360.352.1465
Contact:	Whitney Holm, PE Whitney.Holm@scjalliance.com
Project Reference:	SCJ #22-000313
	Path: N:\Projects\0620 City of Lacey\22-000313 RAC Parking Lot Design\Design\Storm\Appendices\SWPPP\2022-xxxx SWPPP.docx

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1. NARRATIVE

1.1 STORMWATER BMPs

Total Disturbed Area:	5.58 acres
Property Use:	Low-Density Residential (LD 3-6)
Parcel Number:	11814410200
Section, Township, Range:	Section 14, Township 18N, Range 1W, W.M.

The following explains and illustrates the measures to be taken on the site to control erosion and sedimentation problems. The SWPPP is a guideline to follow during construction to prevent erosion and sedimentation. Erosion control measures are not limited to those shown in this SWPPP. Measures shall be installed as necessary to meet the Department of Ecology's (DOE) and the Thurston County's guidelines for stormwater pollution prevention and the requirements of the DOE National Pollutant Discharge Elimination System (NPDES) permit. Further, the SWPPP shall be updated by the contractor as required by the requirements of the DOE NPDES permit.

1.1.1 Element #1 –Mark Clearing Limits

To protect adjacent properties and reduce the area of soil exposed, the limits of the construction will be clearly marked before land-disturbing activities begin. Where possible natural vegetation shall be preserved. The following BMP will be implemented where appropriate:

- BMP C101: Preserving Natural Vegetation
- BMP C103: High Visibility Plastic or Metal Fence
- BMP C233: Silt Fence

1.1.2 *Element #2 – Establish Construction Access*

Access points should be established to minimize the tracking of sediment onto public roads, and wheel washing, street sweeping, and street cleaning shall be employed to prevent sediment from entering state waters. All wash wastewater shall be controlled on site.

Construction access will be granted from the existing fire station accress road on the west side of the parcel.

- BMP C106: Wheel Wash
- BMP C107: Construction Road/Parking Area Stabilization

1.1.3 Element #3 – Control Flow Rates

Properties and waterways downstream from development sites shall be protected from erosion due to increases in the volume, velocity, and peak flow rate of stormwater runoff from the project site. The following BMPs are applicable for this project. If the following BMPs are not shown on the construction plan set, the Engineer reserves the right to direct the Contractor to install, construct, and/or implement said BMPs:

• BMP C240: Sediment Trap

In general, discharge rates of stormwater from the site will be controlled where increases in impervious area or soil compaction during construction could lead to downstream erosion, or where necessary to meet local agency stormwater discharge requirements.

1.1.4 Element #4 – Install Sediment Controls

Prior to leaving a construction site, stormwater runoff must pass through a sediment pond or other appropriate sediment removal BMP. Silt fence barriers shall be installed in accordance with BMP C233. In addition, the following BMP's will be implemented where appropriate:

• BMP C233: Silt Fence

In addition, sediment will be removed from paved areas in and adjacent to work areas manually or using mechanical sweepers, as needed, to minimize tracking of sediments on vehicle tires away from the site and to minimize wash off of sediments from adjacent streets in runoff.

In some cases, sediment discharge in concentrated runoff can be controlled using permanent stormwater BMP's (e.g. infiltration swales, ponds, trenches). Sediment loads can limit the effectiveness of some permanent stormwater BMP's, such as those used for infiltration or biofiltration; however, those BMP's designed to remove solids by settling (wet ponds or detention ponds) can be used. When permanent stormwater BMP's will be used to control sediment discharge, the structure will be protected from excessive sedimentation with adequate erosion and sediment control BMP's. Any accumulated sediment shall be removed after construction is complete and the permanent stormwater BMP will be restabilized with vegetation per applicable design requirements once the remainder of the site has been stabilized. Concentrated runoff is not anticipated for this project.

1.1.5 Element #5 – Stabilize Soils

All exposed and unworked soils shall be stabilized by application of effective BMP's, which protect the soil from the erosive forces of raindrop impact and flowing water and from wind erosion. From October 01 through April 30 of each calender year, no soils shall remain exposed and unworked for more than two (2) days. From May 01 to September 30 of each calender year, no soils shall remain exposed and unworked for more than seven (7) days. This condition applies to all on-site soils, whether at final grade or not. Additionally, except where approved chemical treatment, full dispersion, or infiltration is practiced, clearing, grading, and other soil disturbing activities are prohibited between November 1 and February 28.

In areas where construction activities have temporarily or permanently ceased, seeding and mulching shall be used in accordance with BMP's C120 and C121. Dust control shall be used as needed to prevent wind transport of dust from disturbed soil surfaces and in accordance with BMP C140.

In general, cut slopes will be stabilized as soon as possible and soil stockpiles will be temporarily covered with plastic sheeting. All stockpiled soils shall be stabilized from erosion, protected with sediment trapping measures, and where possible, be located away from storm drain inlets, waterways, and drainage channels.

- BMP C120: Temporary and Permanent Seeding
- BMP C123: Plastic Covering
- BMP C140: Dust Control

1.1.6 Element #6 – Protect Slopes

Slopes shall be constructed in a manner that will minimize erosion. This shall include, but is not limited to: placing excavated material on the uphill side of trenches, collecting drainage at the top of slopes, etc. Slopes will be stabilized as indicated in Element #5 above. In addition, the following BMP's will be implemented where appropriate:

- BMP C130: Surface Roughening
- BMP C131: Gradient Terraces

1.1.7 Element #7 – Protect Drain Inlets

All storm drain inlets made operable during construction shall be protected to prevent unfiltered or untreated water from entering the drainage conveyance system. However, the first priority is to keep all access roads clean of sediment and keep street wash water separate from entering storm drains until treatment can be provided. Storm Drain Inlet Protection (BMP C220) will be implemented for all drainage inlets that could potentially be impacted by sediment-laden runoff on and near the project site. The following inlet protection measures will be applied on this project:

BMP C220: Storm Drain Inlet Protection

1.1.8 Element #8 – Stabilize Channels and Outlets

All temporary on-site conveyance channels shall be constructed and stabilized to prevent erosion. Stabilization, including armoring material, adequate to prevent erosion of outlets, adjacent to streambanks, slopes and downstream reaches shall be provided at the outlets of all conveyance systems. The following BMP's will be implemented where appropriate:

- BMP C202: Channel Lining
- BMP C207: Check Dams
- BMP C209: Outlet Protection

1.1.9 *Element #9 – Control Pollutants*

All pollutants, including waste materials, that occur on-site during construction shall be handled and disposed of in a manner that does not cause contamination of stormwater. Maintenance and repair of heavy equipment and vehicles involving oil changes, hydraulic system drain down, solvent and degreasing cleaning operations, fuel tank drain down and removal, and other activities which may result in discharge or spillage of pollutants to the ground or into stormwater runoff must be conducted using spill prevention measures, such as drip pans. Contaminated surfaces shall be cleaned immediately following any discharge or spill incident. Emergency repairs may be performed on-site using temporary plastic placed beneath and, if raining, over the vehicle. Application of agricultural chemicals, including fertilizers and pesticides, shall be conducted in a manner and at application rates that will not result in loss of chemical to stormwater runoff. Manufacturers' recommendations shall be followed for application rates and procedures. No pH-Modifying sources will be present on-site.

Three source control BMP's will apply to this project:

- A Spill Prevention Plan
- Maintenance of Storm Drainage Facilities
- Street Sweeping

In addition, the following BMP's shall be implemented where appropriate:

- BMP C151: Concrete Handling
- BMP C152: Sawcutting and Surfacing Pollution Prevention
- BMP C153: Material Delivery, Storage and Containment
- BMP C154: Concrete Washout Area

1.1.10 Element #10 – Control Dewatering

Clean, non-turbid de-watering water, as determined by the Certified Professional in Erosion and Sediment Control, can be discharged to systems tributary to state surface waters, provided the dewatering flow does not cause erosion or flooding of receiving waters. These clean waters should not be routed through stormwater sediment ponds.

Highly turbid or otherwise contaminated de-watering water, such as from equipment operation shall be handled separately from stormwater at the site. Some disposal options, depending on site

constraints, may include: 1) transport off-site in vehicle, such as a vacuum flush truck, for legal disposal in a manner that does not pollute state waters, 2) on-site treatment using chemical treatment or other suitable treatment technologies, or 3) sanitary sewer discharge with local sewer district's approval if there is no other option.

1.1.11 Element #11 – Maintain BMP's

All temporary and permanent erosion and sediment control BMP's shall be maintained and repaired as needed to assure continued performance of their intended function. Maintenance and repair shall be conducted in accordance with each particular BMP's specifications. Visual monitoring of the BMP's will be conducted per the inspection schedule in Section 6.

All temporary erosion and sediment control BMP's shall be removed within 30 days after the final site stabilization is achieved or after the temporary BMP's are no longer needed. Trapped sediment shall be removed or stabilized on site. Disturbed soil resulting from removal of BMP's or vegetation shall be permanently stabilized.

• BMP C160: Certified Erosion and Sediment Control Lead

1.1.12 Element #12 – Manage the Project

Erosion and sediment control BMP's for this project have been designed based on the following principles:

- Design the project to fit the existing topography, soils, and drainage patterns.
- Emphasize erosion control rather than sediment control.
- Minimize the extent and duration of the area exposed.
- Keep runoff velocities low.
- Retain sediment on site.
- Thoroughly monitor site and maintain all ESC measures.

In addition, project management will incorporate the key components listed below:

Phasing

Revegetation of exposed areas and maintenance of that vegetation shall be an integral part of the clearing activities during each phase of construction, per the Scheduling BMP (C162).

Inspection and Monitoring

All BMP's shall be inspected, maintained, and repaired as needed to assure continued performance of their intended function. Site inspections shall be conducted by a person who is knowledgeable in the principles and practices of erosion and sediment control. This person has the necessary skills to:

- Assess the site conditions and construction activities that could impact the quality of stormwater, and
- Assess the effectiveness of erosion and sediment control measures used to control the quality of stormwater discharges.

A Certified Erosion and Sediment Control Lead shall be on-site or on-call at all times.

Whenever inspection and/or monitoring reveals that the BMP's identified in this SWPPP are inadequate, due to the actual discharge of or potential to discharge a significant amount of any pollutant, appropriate BMP's or design changes shall be implemented as soon as possible.

Maintaining an Updated SWPPP

This SWPPP shall be retained on-site or within reasonable access to the site.

- The SWPPP shall be modified whenever there is a change in the construction activities that has, or could have, a significant effect on the discharge of pollutants to waters of the state.
- The SWPPP shall be modified if, during inspections or investigations conducted by the owner/operator, or the applicable local or state regulatory authority, it is determined that the SWPPP is ineffective in eliminating or significantly minimizing pollutants in stormwater discharges from the site. The SWPPP shall be modified as necessary to include additional or modified BMP's designed to correct problems identified. Revisions to the SWPPP shall be completed within seven (7) days following the inspection.

1.1.13 *Element #13 – Protect Low Impact Development BMPs*

All temporary and permanent erosion and sediment control BMPs shall be maintained and repaired as needed to assure continued performance of their intended function. All maintenance and repairs shall be completed in accordance with the practices, procedures, and materials for each respective BMP.

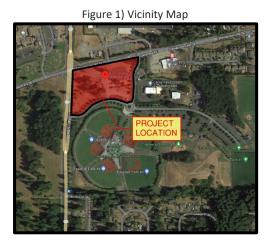
All Bioretention and Rain Garden BMPs shall be protected from sedimentation through installation and maintenance of erosion and sediment control BMPs. Prevent compaction Bioretention and Rain Garden BMPs by excluding construction equipment and foot traffic. Protect completed lawn and landscaped areas from compaction due to construction equipment.

- BMP C102: Buffer Zone
- BMP C103: High Visibility fence
- BMP C200: Interceptor Dike and Swale
- BMP C201: Grass-Lined Channels
- BMP C207: Check Dams
- BMP C208: Triangular Silt Dike (TSD) (Geotextile-Encased Check Dam)

- BMP C231: Brush Barrier
- BMP C233: Silt Fence
- BMP C234: Vegetated Strip

1.2 **PROJECT DESCRIPTION**

The project is located at 8323 Steilacoom Road SE, Olympia, WA 98513. See Vicinity Map below.



The project site is +/- 5.58 acres in size and includes one 4.3 acre parcel and frontage. The site contains an existing gravel parking area, concrete sidewalk along the frontage on Marvin Rd SE, approximately 90' of concrete sidewalk along the frontage on Steilacoom Rd SE, and asphalt sidewalk along the frontages of the RAC entrances. The stormwater runoff from this project is divided into two subbasins. The first subbasin will collect water from the frontage improvements and convey the runoff into the city stormwater system via a catch basin on the southest corner of Marvin Rd and Steilacoom Rd SE. The second subbasin will collect stormwater runoff from on-site of the project area and will be conveyed to a bioretention pond on the southwest corner of the site for treatment, excess stormwater will be conveyed from the bioretention pond to a retention pond on-site to be entirely infiltrated on-site.

1.3 EXISTING SITE CONDITIONS

The site's topography is generally flat and level. The existing site topography is included on the survey in the construction documents. Further, an erosion control plan has also been prepared as part of the construction documents.

The existing vegetation consists of large Douglas fir trees, various fruit trees and a stormwater infiltration pond located in the southwest portion of the site.

There are no known ponding problems located on-site.

1.4 ADJACENT AREAS

The project site is bounded by Steilacoom Rd SE to the north, RAC Entrances to the east and south, and Marvin Rd SE to the west. Stormwater runoff from the project site will be treated and infiltrated onsite. Stormwater runoff from frontage improvements consist of the addition of less than 5,000 SF of stormwater runoff to the City of Lacey's stormwater drainage system, therefore there are no anticipated adverse effects to the downstream systems.

1.5 CRITICAL AREAS

Flood Zones: The project parcel is located with Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 53067C0192E. According to the FIRM Map the project parcel is located within Zone X. Zone X is determined to be an area of minimal flood hazard.

Wellhead Protection Areas: According to Figure 8B.1 of the SDM, the proposed project is located within the 5-year time of travel zone for a wellhead protection area.

1.6 Soil

Landau Associates performed a geotechnical investigation in August of 2022. Subsurface explorations observed the Vashon recessional outwash (Qgo) and Latest Vashon recessional [outwash] sand and minor silt (Qgos). Fill was found between 0.25 to 1.0 ft below ground surface. On-site soils were found to be conducive to infiltration and bioretention facilities. Field infiltration rates measured to be 20-25 inches per hour and a design infiltration rate of 5.5 inches per hour was utilized when sizing infiltration facilities.

1.7 POTENTIAL EROSION

Potential on-site erosion control problems are not anticipated at this time. The Certified Professional in Erosion and Sediment Control will be on-site or on-call during construction activities to identify any erosion control problems. If there is a problem, the Certified Professional in Erosion and Sediment Control will promptly authorize the Contractor to initiate corrective measures.

1.8 CONSTRUCTION PHASING

The BMP implementation schedule will be driven by the construction schedule. The key milestones for each segment are as follows:

- 05/01/2023: Dry season starts
- 06/05/2023: Mobilize equipment on-site
- 06/05/2023: Mobilize and store all erosion and sediment control (ESC) and soil stabilization products (store Materials On Hand BMP C150)
- 06/05/2023: Install ESC measures include stormwater management facility if applicable

- 06/05/2023: Begin implementing soil stabilization and sediment control BMPs throughout the site for the duration of the wet season. Implement Element #12 BMPs and manage site to minimize soil disturbance.
- 06/05/2023: Site inspections and monitoring conducted weekly and for applicable rain events as detailed in Section 1.13 of this SWPPP
- 06/12/2023: Begin clearing and grubbing
- 10/15/2023: Wet season starts
- 05/01/2024: Dry season starts
- 07/29/2024: Construction ends, restoration of the project site completed

1.9 CONSTRUCTION SCHEDULE

Estimated Construction Start Date: June 2023

Estimated Construction End Date: July 2024

1.10 FINANCIAL/OWNERSHIP RESPONSIBILITIES

Lacey Regional Athletic Complex will be the owner of the site and will have full responsibility financially. If or when a new owner takes over the site the new owner will have full financial responsibilities of the site.

1.11 ENGINEERING CALCULATIONS

All the engineering calculations from WWHM are documented in the Drainage and Erosion Control Plan Appendix 1 prepared by SCJ Alliance December 2022.

1.12 POLLUTION PREVENTION TEAM

1.12.1 Roles and Responsibilities

The pollution prevention team consists of personnel responsible for implementation of the SWPPP, including the following:

- Certified Erosion and Sediment Control Lead Primary contractor contact, responsible for site inspections (BMPs, visual monitoring, sampling, etc.); to be called upon in case of failure of any ESC measures.
- Resident Engineer For projects with engineered structures only (sediment pond/traps, sand filters, etc.): site representative for the owner that is the project's supervising eingeer responsible for inspections and issuing instructions and drawings to the contractor's site supervisor or representative.

- Emergency Owner Contact Individual that is the site owner or representative of the site owner to contacted in the case of an emergency.
- Monitoring Personnel Personnel responsible for conducting water quality monitoring; for most sites this person is also the CESCL.

1.12.2 Team Members

Title	Name (s)	Phone Number
Project Engineer	Whitney Holm	360.352.1465
Emergency Owner Contact		
Emergency Ecology Contact	Souhwest Regional Office	360.407.6300
Non-Emergency Ecology Contact	Carol Serdar	360.407.6269
		500.407.0203
Monitoring Personnel		

1.13 SITE INSPECTIONS AND MONITORING

Monitoring includes visual inspection, monitoring for water quality parameters of concern and documentation of the inspection and monitoring findings in a site log book. A site log book will be maintained for all on-site construction activities and will include:

- A record of the implementation of the SWPPP and other permit requirements
- Site inspections; and,
- Stormwater quality monitoring.

For convenience, the inspection form and water quality monitoring forms included in this SWPPP include the required information for the site log book. This SWPPP may function as the site log book if desired, or the forms may be separated and included in a separate site log book. However, if separated, the site log book must be maintained on site or within reasonable access to the site and be made available upon request to Ecology or the local jurisdiction.

1.13.1 Site Inspection

All BMPs will be inspected, maintained, and repaired as needed to assure continued performance of their intended function. The inspector will be a CESCL per BMP C160. The name and contact information for the CESCL is provided in Seciton 1.12.2 of this SWPPP

Site inspection will occur in all areas disturbed by construction activities and at all potential stormwater discharge points. Stormwater will be examined for the presence of suspended sediment, turbidity, discoloration, and oily sheen.

The site inspector will evaluate and document the effectiveness of the installed BMPs and determine if it is necessary to repair or replace any of the BMPs to improve the quality of the stormwater discharges. All maintenance and repairs will be documented in the site log book or forms provided in this document. All new BMPs or design changes will be documented in the SWPPP as soon as possible.

1.13.2 Site Inspection Frequency

Site inspected will be conducted at least once a week and within 24 hours following any discharge from the site. For sites with temporary stabilization measures, the site inspection frequency will be reduced to once every month.

1.13.3 Site Inspection Documentation

The site inspector will record each site inspection using the site log inspection forms provided in Appendix C. The site inspection log forms may be separated from this SWPPP document, but will be maintained on site or within reasonable access to the site and be made available upon request to Ecology or the local jurisdiction.

1.14 STORMWATER QUALITY MONITORING

1.14.1 Turbidity

Turbidity sampling and monitoring will be conducted during the entire construction phase of the project. Samples will be collected weekly at the discharge point nearest the current phase of the project work. If there is no flow at the discharge point, the attempt to sample will be recorded in the site log book and reported to Ecology in the monthly Discharge Monitoring Report (DMR) as "No Discharge". Samples will be analyzed for turbidity using the Hach 2100Q Turbidimeter.

The key benchmark turbidity value is 25 nephelometric turbidity units (NTU) for the downstream receiving water body. If the 25 NTU benchmark is exceeded in any sample collected, the following steps will be conducted:

- 1. Ensure all BMPs specified in this SWPPP are installed and functioning as intended.
- 2. Assess whether additional BMPs should be implemented, and document modified BMPs in the SWPPP as necessary.
- 3. Sample discharge daily util the discharge is 25 NTU or lower.

If the turbidity exceeds 250 NTU at any time, the following steps will be conducted:

- 1. Notify ecology by phone within 24 hours of analysis (see Section 1.12.2 of this SWPPP for contact information).
- Continue sampling daily until the discharge is 25 NTU or lower. Initiate additional treatment BMPs such as off-site treatment, infiltration, filtration and chemical treatment within 24 hours, and implement those additional treatment BMPs as soon as possible, but within a minimum of 7 days.

3. Describe inspection results and remedial actions taken in the site log book and in monthly discharge monitoring reports described in Section 1.15 of this SWPPP.

1.14.2 *pH*

Sampling and monitoring of pH occurs if significant concrete work (> 1,000 cubic yards throughout the life of the project) or use of engineered soils (e.g., cement-treated base) is anticipated. No significant concrete work or engineered soils is planned for this project; therefore , no pH testing will be conducted.

1.15 RECORDKEEPING

1.15.1 Site Log Book

A site log book will be maintained for all on-site construction activities and will include:

- A record of the implementation of the SWPPP and other permit requirements;
- Site inspections; and,
- Stomrwater quality monitoring.

For convenience, the inspection form and water quality monitoring forms included in this SWPPP include the required information for the site log book.

1.15.2 Records Retention

Records of all monitoring information (site log book, inspection reports/checklists, etc.), this Stormwater Pollution Prevention Plan, and any other documentation of compliance with permit requirements will be retained during the life of the construction project and for a minimum of three years following the termination of permit coverage in accordance with permit condition S5.C.

1.15.3 Access to Plans and Records

The SWPPP, General Permit, Notice of Authorization letter, and Site Log Book will be retained on site or within reasonable access to the site and will be made immediately available upon request to Ecology or the local jurisdiction. A copy of this SWPPP will be provided to Ecology within 14 days of receipt of written request for the SWPPP from Ecology. Any other information requested by Ecology will be submitted within a reasonable time. A copy of the SWPPP or acces ot the SWPPP will be provided to the public when requested in writing in accordance with permit condition S5.G.

1.15.4 Updating the SWPPP

In accordance with conditions S3, S4.B, and S.B.3 of the General Permit, this SWPPP will be modified if the SWPPP is ineffective in eliminating of significantly minimizing pollutants in stormwater discharges from the site or there has been a change in design, construction, operation, or maintenance at eh site that has a significant effect on the discharge, or potential for discharge, of pollutants to the waters of the State. The SWPPP will be modified within seven days of determination based on inspection(s) that additional or modified BMPs are necessary to correct problems identified, and an updated timeline for BMP implementation will be prepared.

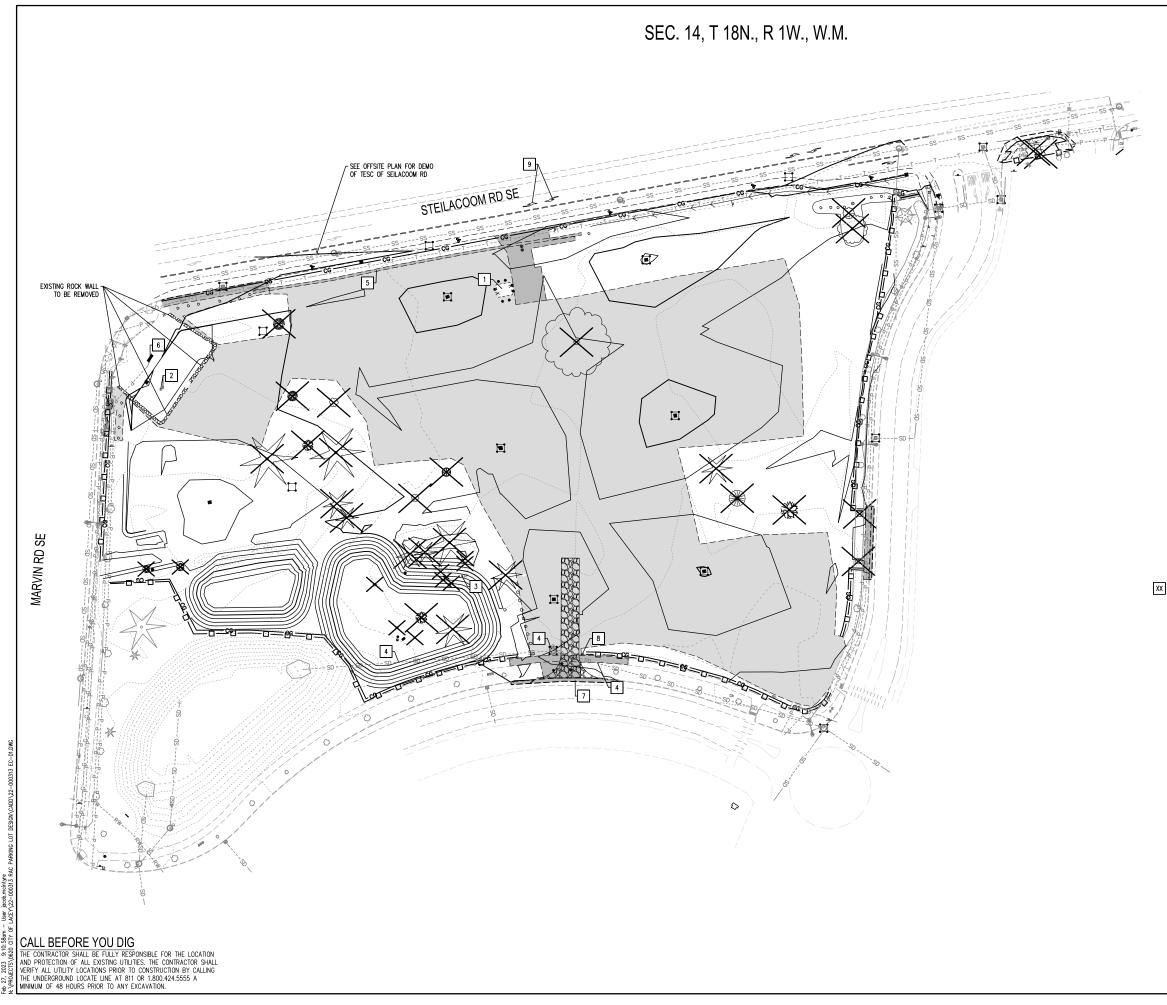
1.16 REPORTING

1.16.1 Notification of Noncompliance

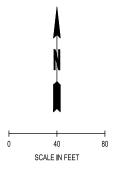
If any of the terms and conditions of this permit is not met, and it causes a threat to human health or the environment, the following steps will be taken in accordance with permit section S5.F:

- 1. Ecology will be immediatedly notified of the failure to comply.
- 2. Immediate action will be taken to control the noncompliance issue and to correct the problem. If applicable, sampling and analysis of any noncompliance will be repeated immediately and submitted to Ecoloby within five days of becoming aware of the violation.
- 3. A detailed report describing the noncompliance will be submitted to Ecology within five days, unless requested earlier by Ecology.

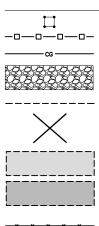
APPENDIX A EROSION CONTROL AND GRADING PLANS



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0343 3 I EILAUOUMI RU SE I ACFY WA	8/30 IALLON LANE NE, SUITE 200, LACEY, WA 36318 P: 360.352.1465 F: 360.352.1509				
	SCJALLIANCE.COM				



LEGEND



INLET SEDIMENT PROTECTION: SEE EC-	02
SILT FENCE: SEE EC-02	

CLEARING AND GRUBBING LIMITS

STABILIZED CONSTRUCTION ENTRANCE: SEE EC-02

SAWCUT

EXISTING TREE AND ROOT BALL TO BE REMOVED AND BACK FILLED

EXISTING GRAVEL TO BE REMOVED

EXISTING ASPHALT/CONCRETE TO BE REMOVED

TREE PROTECTION FENCING SEE EC-02

- 1. PSE GAS AND ELECTRICAL EQUIPMENT AND ASSOCIATED FENCE AND BOLLARDS SHALL BE REMOVED: CONTRACTOR SHALL COORDINATE WITH PSE
- 2. IRRIGATION VALVES TO BE REMOVED AND RELOCATED: SEE LANDSCAPE PLANS
- 3. SPRINKLER HEAD TO BE REMOVED AND RELOCATED: SEE LANDSCAPE PLANS
- 4. PROTECT-IN-PLACE EXISTING STORM LINE AND STRUCTURES
- 5. REMOVE EXISTING ECOLOGY BLOCKS AND RETURN TO CITY OF LACEY. CONTRACTOR SHALL COORDINATE WITH CITY OF LACEY ON DELIVERY LOCATION
- 6. PROTECT-IN-PLACE DIGITAL MESSAGE BOARD AND ASSOCIATED WIRING
- 7. SAWCUT
- 8. SIGN REMOVAL

DESIGNER: W. HOLM DRAWN BY: K. GANS APPROVED BY: W. HOLM DATE: DECEMBER 2022 JOB NO: 22-000313 DRAWING FILE NO: 22-000314 DRAWING FILE NO: 22-000315 DRAWING FILE NO: 22-000315

CITY OF LACEY STANDARD EROSION CONTROL NOTES

A CERTIFIED EROSION AND SEDIMENT CONTROL LEAD (CESCL) IS REQUIRED FOR ALL CONSTRUCTION PROJECTS. THE NAMED PERSON OR FIRM SHALL BE ON-SITE OR ON-CALL AT ALL TIMES. FOR THIS SITE, THE PERSON/FIRM IS _______ AND THEIR TELEPHONE NUMBERS ARE:

(OFFICF:) (CELL:)____-

APPROVAL OF THIS EROSION & SEDIMENT CONTROL (ESC) PLAN DOES NOT CONSTITUTE AN APPROVAL OF PERMANENT ROAD OR DRAINAGE DESIGN (E.G. SIZE AND LOCATION OF ROADS, PIPES, RESTRICTORS, CHANNELS, RETENTION FACILITIES, UTILITIES, ETC.).

THE IMPLEMENTATION OF THESE ESC PLANS AND THE CONSTRUCTION, MAINTENANCE, REPLACEMENT, AND UPGRADING OF THESE ESC FACILITIES IS THE RESPONSIBILITY OF THE APPLICANT/CONTRACTOR UNTIL ALL CONSTRUCTION IS COMPLETED AND APPROVED AND VEGETATION/LANDSCAPING IS ESTABLISHED.

THE CLEARING LIMIT BOUNDARIES SHOWN ON THIS PLAN SHALL BE CLEARLY FLAGGED IN THE FIELD PRIOR TO CONSTRUCTION. DURING THE CONSTRUCTION OF TIME FLOW STURBANCE BEYOND THE FLOWED IN THE FLOW FINITES SHALL BE PERMITTED. THE FLAGGING SHALL BE MAINTAINED BY THE APPLICANT/CONTRACTOR FOR THE DURATION OF CONSTRUCTION

THE ESC FACILITIES SHOWN ON THIS PLAN MUST BE CONSTRUCTED IN CONJUNCTION WITH ALL CLEARING AND GRADING ACTIVITIES. AND IN SUCH A MANNER AS TO ENSURE THAT SEDIMENT AND SEDIMENT-LADEN WATER DO NOT ENTER THE DRAINAGE SYSTEM, ROADWAYS, OR VIOLATE APPLICABLE SURFACE WATER, GROUND WATER, OR DISCHARGE STANDARDS.

THE ESC FACILITIES SHOWN ON THIS PLAN ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS EVENTS AND TO ENSURE THAT SEDIMENT AND SEDIMENT-LADEN WATER DO NOT LEAVE THE SITE.

THE ESC FACILITIES ON ACTIVE SITES SHALL BE INSPECTED DAILY BY THE APPLICANT/CONTRACTOR--AND MAINTAINED, REPAIRED, OR AUGMENTED AS NECESSARY--TO ENSURE THEIR CONTINUED FUNCTIONING.

THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED MONTHLY AND WITHIN 48 HOURS FOLLOWING A MAJOR STORM EVENT (21"RAINFALL IN 24 HOURS) BY THE APPLICANT/CONTRACTOR — AND MAINTAINED, REPAIRED, OR AUGMENTED AS NECESSARY — TO ENSURE THEIR CONTINUED FUNCTIONING, STORM DRAIN INLETS OPERABLE DURING CONSTRUCTION SHALL BE PROTECTED SO THAT STORMWATER RUNOFF DOES NOT ENTER THE CONVEYANCE SYSTEM WITHOUT FIRST BEING FILTERED OR TREATED TO REMOVE SEDIMENT. AT NO TIME SHALL MORE THAN 1 FOOT OR 1/3 OF THE SUMP VOLUME (WHICHEVER IS LESS) OF SEDIMENT BE ALLOWED TO ACCUMULATE WITHIN A CATCH BASIN. ALL CATCH BASINS AND CONVEYANCE LINES SHALL BE CLEANED PRIOR TO PROJECT COMPLETION AND ACCEPTANCE. THE CLEANING OPERATION SHALL NOT FLUSH SEDIMENT-LADEN WATER OFFSITE WITHOUT TREATMENT.

STABILIZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION AND MAINTAINED FOR THE DURATION OF THE PROJECT. ADDITIONAL MEASURES MAY BE REQUIRED TO ENSURE THAT ALL PAVED AREAS ARE KEPT CLEAN FOR THE DURATION OF THE PROJECT.

 ROADS SHALL BE CLEANED THOROUGHLY AS NEEDED TO PROTECT DOWNSTREAM WATER RESOURCES OR STORNWATER
INFRASTRUCTURE. SEDIMENT SHALL BE REMOVED FROM ROADS BY SHOVELING OR PICKUP SWEEPING AND SHALL BE TRANSPORTED TO A CONTROLLED SEDIMENT DISPOSAL AREA.

FROM OCTOBER 1 THROUGH APRIL 30, NO SOILS SHALL REMAIN EXPOSED AND UNWORKED FOR MORE THAN 2 DAYS. FROM WAY TO SEPTEMBER 30, NO SOLS SHALL REMAIN EXPOSED AND UNWORKED FOR MORE THAN 2 DATS. FROM MAY 1 TO SEPTEMBER 30, NO SOLS SHALL REMAIN EXPOSED AND UNWORKED FOR MORE THAN 7 DAYS. SOLS SHALL BE STABILIZED AT THE END OF THE SHIFT BEFORE A HOLDAY OR WEEKEND IF NEEDED BASED ON THE WEATHER FOREOAST. LINEAR CONSTRUCTION ACTIVITES, SUCH AS RIGHT-OF-WAY AND EASEMENT CLEARING, ROADWAY DEVELOPMENT, IPPELINES, AND TEMOCING FOR UTILITES, SHALL COMPLY WITH THESE REQUIREMENTS. THESE STABILIZATION REQUIREMENTS APPLY TO ALL SOLS ON SITE, WHETHER AT FINAL GRADE OR NOT. THE LOCAL PERMITTING AUTHORITY MAY ADJUST THESE TIME LIMITS IF IT CAN BE SHOWN THAT A DEVELOPMENT SITE'S EROSION OR RUNOFF POTENTIAL JUSTIFIES A DIFFERENT STANDARD

FROM OCTOBER 1 THROUGH APRIL 30, CLEARING, GRADING, AND OTHER SOIL-DISTURBING ACTIVITIES SHALL ONLY BE FRMITED IF SHOWN TO THE SATISFACTOR OF THE LOCAL PERMITTING AUTHORITY THAT THE TRANSPORT OF SEDIMEN FROM THE CONSTRUCTION SITE TO RECEIVING WATERS WILL BE PREVENTED.

SOIL STOCKPILES MUST BE STABILIZED AND PROTECTED WITH SEDIMENT-TRAPPING MEASURES.

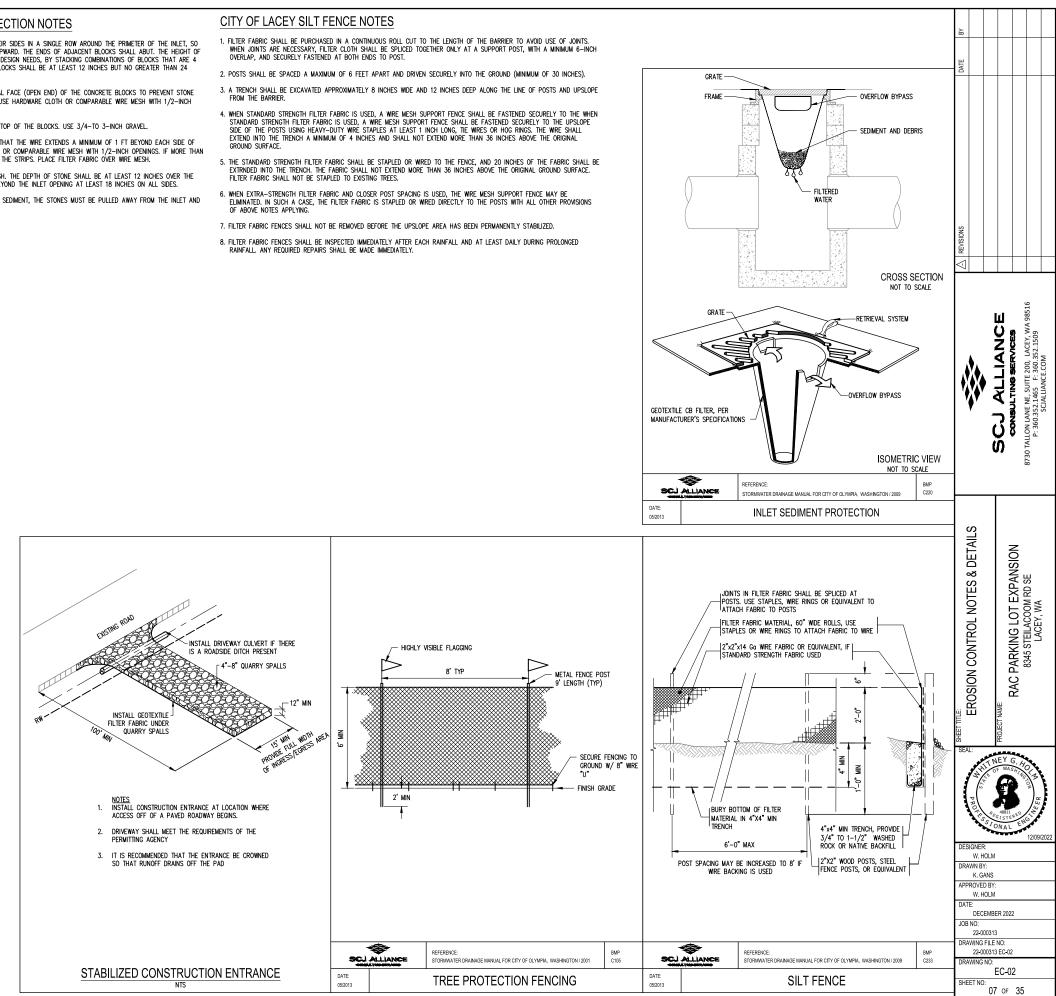
 ALL POLLUTANTS, INCLUDING WASTE MATERIALS AND DEMOLITION DEBRIS, THAT OCCUR ON SITE DURING
CONSTRUCTION SHALL BE HANDLED AND DISPOSED OF IN A MANNER THAT DOES NOT CAUSE CONTAMINATION OF STORMWATER. WOODY DEBRIS MAY BE CHOPPED AND SPREAD ON SITE.

 MAINTENANCE AND REPAIR OF HEAVY EQUIPMENT AND VEHICLES AND OTHER ACTIVITIES WHICH MAY RESULT IN DISCHARGE OR SPILLAGE OF POLLUTANTS TO THE GROUND OR INTO STORMWATER RUNOFF MUST BE CONDUCTED USING SPILL PREVENTION MEASURES, SUCH AS DRIP PANS. REPORT ALL SPILLS TO 911

WATER FROM MOST DEWATERING OPERATIONS SHALL BE DISCHARGED INTO A SEDIMENT TRAP OR POND. CLEAN. NON-TURBID WATER MAY BE DISCHARGED TO STATE SURFACE WATERS, RROWDED THE DISCHARGE DOES NOT CAUSE EROSION OR FLOCOING, HIGHLY TURBID OR CONTAMINATED DEWATERN, WATER FROM CONSTRUCTION EQUIPMENT OPERATION, CLAMSHELL DIGGING, CONCRETE TREMIE POUR, OR WORK INSIDE A COFFERDAM SHALL BE HANDLED SEPARATELY FROM STORWWATER AND PROPERLY DISPOSED.

- PLACE CONCRETE BLOCKS LENGTHWISE ON THEIR SIDES IN A SINGLE ROW AROUND THE PRIMETER OF THE INLET, SO THAT THE OPEN ENDS FACE OUTWARD, NOT UPWARD. THE ENDS OF ADJACENT BLOCKS SHALL ABUT. THE HEIGHT OF THE BARRIER CAN BE VARIED, DEPENDINGE ON DESIGN INEEDS, BY STACKING COMBINATIONS OF BLOCKS THAT ARE 4 INCHES, AND 12 INCHES WIDE, THE ROW OF BLOCKS SHALL BE AT LEAST 12 INCHES BUT NO GREATER THAN 24 INCHES HIGH
- 2. PLACE WIRE MESH OVER THE OUTSIDE VERTICAL FACE (OPEN END) OF THE CONCRETE BLOCKS TO PREVENT STONE FROM BEING WASHED THROUGH THE BLOCKS. USE HARDWARE CLOTH OR COMPARABLE WIRE MESH WITH 1/2-INCH
- 3. PILE STONE AGAINST THE WIRE MESH TO THE TOP OF THE BLOCKS. USE 3/4-TO 3-INCH GRAVEL.
- 4. PLACE WIRE MESH OVER THE DROP INLET SO THAT THE WIRE EXTENDS A MINIMUM OF 1 FT BEYOND EACH SIDE OF THE INLET STRUCTURE, USE HARDWARE CLOTH OR COMPARABLE WIRE MESH WITH 1/2-INCH OPENINGS. IF MORE THAN ONE STRIP OF MESH IS NECESSARY, OVERLAP THE STRIPS. PLACE FILTER FABRIC OVER WIRE MESH.
- 5. PLACE 3/4-INCH GRAVEL OVER THE WIRE MESH. THE DEPTH OF STONE SHALL BE AT LEAST 12 INCHES OVER THE ENTIRE INLET OPENING. EXTEND THE STONE BEYOND THE INLET OPENING AT LEAST 18 INCHES ON ALL SIDES.
- 6. IF THE STONE FILTER BECOMES CLOGGED WITH SEDIMENT, THE STONES MUST BE PULLED AWAY FROM THE INLET AND CLEANED OR REPLACED

- WHEN JOINTS ARE NECESSARY, FILTER CLOTH SHALL BE SPLICED TOGETHER ONLY AT A SUPPORT POST, WITH A MINIMUM 6-INCH OVERLAP, AND SECURELY FASTENED AT BOTH ENDS TO POST.
- FROM THE BARRIE
- STANDARD STRENGTH FILTER FABRIC IS USED, A WIRE MESH SUPPORT FENCE SHALL BE FASTENED SECURELY TO THE UPSLOPE SIDE OF THE POSTS USING HEAVY-DUTY WIRE STAPLES AT LEAST 1 INCH LONG, TIE WIRES OR HOG RINGS. THE WIRE SHALL EXTEND INTO THE TRENCH A MINIMUM OF 4 INCHES AND SHALL NOT EXTEND MORE THAN 36 INCHES ABOVE THE ORIGINAL GROUND SURFACE.
- OF ABOVE NOTES APPLYING
- RAINFALL. ANY REQUIRED REPAIRS SHALL BE MADE IMMEDIATELY



APPENDIX B CONSTRUCTION BMPS

CITY OF LACEY STORMWATER POLLUTION SOURCE CONTROL CHECKLIST

Project Name:

Check all activities that will occur at a proposed site. Only activities common in the City of Lacey are included in this checklist. Other activities may apply to your site. Fill in the blank rows included under each activity grouping if needed based on the complete list of site-specific activities provided in Table 9A.1.

Source	Control BMPs Applicable to All Sites						
BMP #	BMP Name						
S410	Correcting Illicit Discharges to Storm Drains						
S453	Formation of a Pollution Prevention Team						
S454	Preventive Maintenance/Good Housekeeping						
S455	Spill Prevention and Cleanup						
S456	Employee Training						
S457	Inspections						
S458	Record Keeping						
Source	Control BMPs for Specific Activities						
BMP #	BMP Name	Activity Conducted on the Site?					
Cleaning	g or Washing Source Control BMPs						
S431	Washing and Steam Cleaning Vehicles/Equipment/Building Structures	🗆 Yes 🗆 No					
		🗆 Yes 🗆 No					
Roads, I	Ditches, and Parking Lot Source Control BMPs						
S415	Maintenance of Public and Private Utility Corridors and Facilities	🗆 Yes 🗆 No					
S416	Maintenance of Roadside Ditches	🗆 Yes 🗆 No					
S417	Maintenance of Stormwater Drainage and Treatment Systems	🗆 Yes 🗆 No					
S421	Parking and Storage of Vehicles and Equipment	🗆 Yes 🗆 No					
S430	Urban Streets	🗆 Yes 🗆 No					
		🗆 Yes 🗆 No					
Soil Ero	sion, Sediment Control, and Landscaping Source Control BMPs						
S407	Dust Control at Disturbed Land Areas and Unpaved Roadways and Parking Lots	🗆 Yes 🗆 No					
S408	Dust Control at Manufacturing Areas	🗆 Yes 🗆 No					
S411	Landscaping and Lawn/Vegetation Management	🗆 Yes 🗆 No					
S425	Soil Erosion and Sediment Control at Industrial Sites	🗆 Yes 🗆 No					
S435	Pesticides and an Integrated Pest Management Program	🗆 Yes 🗆 No					

BMP #	BMP Name	Activity Conducted on the Site?				
Soil Erosion, Sediment Control, and Landscaping Source Control BMPs (continued)						
S444	Storage of Dry Pesticides and Fertilizers	🗆 Yes 🗆 No				
S449	Nurseries and Greenhouses	🗆 Yes 🗆 No				
S450	Irrigation	🗆 Yes 🗆 No				
		🗆 Yes 🗆 No				
Storage	and Stockpiling Source Control BMPs	·				
S427	Storage of Liquids, Food Waste, or Dangerous Waste Containers	🗆 Yes 🗆 No				
S428	Storage of Liquids in Permanent Aboveground Tanks	🗆 Yes 🗆 No				
S429	Storage or Transfer (Outside) of Solid Raw Materials, Byproducts or Finished Products	🗆 Yes 🗆 No				
		🗆 Yes 🗆 No				
Transfe	r of Liquid or Solid Materials Source Control BMPs					
S409	Fueling at Dedicated Stations	🗆 Yes 🗆 No				
S412	Loading and Unloading Areas for Liquid or Solid Material	🗆 Yes 🗆 No				
S419	Mobile Fueling of Vehicles and Heavy Equipment	🗆 Yes 🗆 No				
S426	Spills of Oil and Hazardous Substances	🗆 Yes 🗆 No				
		🗆 Yes 🗆 No				
Other Se	ource Control BMPs					
S404	Commercial Printing Operations	🗆 Yes 🗆 No				
S414	Maintenance and Repair of Vehicles and Equipment	🗆 Yes 🗆 No				
S418	Manufacturing Activities – Outside	🗆 Yes 🗆 No				
S420	Painting/Finishing/Coating of Vehicles/Boats/Buildings/ Equipment	🗆 Yes 🗆 No				
S423	Recyclers and Scrap Yards	🗆 Yes 🗆 No				
S424	Roof/Building Drains at Manufacturing and Commercial Buildings	🗆 Yes 🗆 No				
S432	Wood Treatment Areas	🗆 Yes 🗆 No				
S433	Pools, Spas, Hot Tubs, and Fountains	□ Yes □ No				
S438	Construction Demolition	□ Yes □ No				
S443	Fertilizer Application	🗆 Yes 🗆 No				
S447	Roof Vents	🗆 Yes 🗆 No				
S451	Building, Repair, Remodeling, Painting, and Construction	🗆 Yes 🗆 No				
		🗆 Yes 🗆 No				

	Table 9A.1. All Site-Specific Source Control BMPs.					
BMP #	BMP Name					
Cleaning	g or Washing Source Control BMPs					
S431	Washing and Steam Cleaning Vehicles/Equipment/Building Structures					
S434	Dock Washing					
S441	Potable Water Line Flushing, Water Tank Maintenance, and Hydrant Testing					
Roads, I	Ditches, and Parking Lot Source Control BMPs					
S405	Deicing and Anti-Icing Operations for Airports					
S406	Streets and Highways					
S415	Maintenance of Public and Private Utility Corridors and Facilities					
S416	Maintenance of Roadside Ditches					
S417	Maintenance of Stormwater Drainage and Treatment Systems					
S421	Parking and Storage of Vehicles and Equipment					
S430	Urban Streets					
Soil Ero	sion, Sediment Control, and Landscaping Source Control BMPs					
S407	Dust Control at Disturbed Land Areas and Unpaved Roadways and Parking Lots					
S408	Dust Control at Manufacturing Areas					
S411	Landscaping and Lawn/Vegetation Management					
S425	Soil Erosion and Sediment Control at Industrial Sites					
S435	Pesticides and an Integrated Pest Management Program					
S444	Storage of Dry Pesticides and Fertilizers					
S449	Nurseries and Greenhouses					
S450	Irrigation					
Storage	and Stockpiling Source Control BMPs					
S427	Storage of Liquids, Food Waste, or Dangerous Waste Containers					
S428	Storage of Liquids in Permanent Aboveground Tanks					
S429	Storage or Transfer (Outside) of Solid Raw Materials, Byproducts or Finished Products					
S445	Temporary Fruit Storage					
Transfei	· of Liquid or Solid Materials Source Control BMPs					
S409	Fueling at Dedicated Stations					
S412	Loading and Unloading Areas for Liquid or Solid Material					
S419	Mobile Fueling of Vehicles and Heavy Equipment					
S426	Spills of Oil and Hazardous Substances					
S439	In-Water and Over-Water Fueling					

Other S	Source Control BMPs
S401	Building, Repair, and Maintenance of Boats and Ships
S402	Commercial Animal Handling Areas
S403	Commercial Composting
S404	Commercial Printing Operations
S413	Log Sorting and Handling
S414	Maintenance and Repair of Vehicles and Equipment
S418	Manufacturing Activities – Outside
S420	Painting/Finishing/Coating of Vehicles/Boats/Buildings/Equipment
S422	Railroad Yards
S423	Recyclers and Scrap Yards
S424	Roof/Building Drains at Manufacturing and Commercial Buildings
S432	Wood Treatment Areas
S433	Pools, Spas, Hot Tubs, and Fountains
S436	Color Events
S438	Construction Demolition
S440	Pet Waste
S442	Labeling Storm Drain Inlets On Your Property
S443	Fertilizer Application
S446	Well, Utility, Directional and Geotechnical Drilling
S447	Roof Vents
S451	Building, Repair, Remodeling, Painting, and Construction
S452	Goose Waste

APPENDIX C SITE INSPECTION FORMS (AND SITE LOG)

	Stormw	vater/Ero	sion Control Insp	ecti	on F	orm		
DATE:			TIME:		NTA			
SITE:					RES:			
LOCATION:				AOI	<u>\L</u> O.			
WEATHER:	1							
PERMIT ON SITE			PERMIT NO.					
SWPPP ON SITE			CONTRACTOR:					
	Best		nent Practices in the	e SW	'PPP	:		1
	In Plan	Properly		Ini	Plan		berly	
Control BMPs	III I IaII	installed	Treatment BMPs		an	Insta	alled	Other:
Preserv Nat Veg			Intercept Dike/Swale					
Buffer Zones			Grass Lines Swales					
High Vis Fence			Channel Lining					
Stake & Wire Fence			Water bars	1				
Stabilized Entrance			Pipe Slope Drains	1				
Wheel Wash			Subsurface Drains	1				
Road/Pk Area Stable			Level Spreader					1
Temp/Perm Seeding			Check Dams					
Mulching			Triangular Silt Dike					
Nets/Blankets			Outlet Protection					
Plastic Covering			Sto Drain Inlet Protec					
Sodding			Straw Bale Barrier					
Topsoiling			Brush Barrier					
Polyacrylamides			Gravel Filter Berm					
Surface roughing			Silt Fence					
Gradient Terraces			Vegetated Strip					
Dust Control			Straw Wattles					
Materials On Hand			Sediment trap					
Concrete Handling			Temp Sed Pond					
Sawcut & Surface			SW Chem Treatment					
Erosion Control Lead			SW Filtration					
Pay Erosion Work			Is the site stabilized?		Par	tially		
Scheduling			Turbid Water is being o	lischa				
Small Project			Receiving Water:		igou.			
Water Quality Samples	Taken?		Results:	pН	TUR	CON	TFM	Other:
Sample ID:			LOC:			0011		
Sample ID:			LOC:	+				
Sample ID:			LOC:					1
This site is IN/OUT of co	mpliance	with the term		mit	1	I	1	I
Summary of remedial ac								
<u> </u>								
								
<u> </u>								
I certify under penalty of la	w that this r	eport is true.	accurate and complete, to	the b	est of I	ny knoi	wledge	e and belief.
Name:			Title:					
Signature:								

Drainage Control Plan Attachment 3

Soils Report

то:	Whitney Holm, PE (Project Manager), SCJ Alliance
FROM:	Lance Levine, PE, and Steven R. Wright, PE
DATE:	September 1, 2022
RE:	Summary of Geotechnical Engineering Services Regional Athletic Complex Parking Lot Design Lacey, Washington Project No. 1174068.010.011

Introduction

This memorandum summarizes the results from the geotechnical engineering services provided by Landau Associates, Inc. (Landau) in support of the Regional Athletic Complex (RAC) Parking Lot Design project, located at 8323 Steilacoom Road Southeast in Lacey, Washington (site; Figure 1). Services were provided in accordance with the scope outlined in the services agreement between Landau and SCJ Alliance (SCJ, project civil engineer) dated July 26, 2022.

This memorandum has been prepared with information provided by SCJ and with data collected during Landau's field exploration and laboratory testing programs.

Project Understanding

The City of Lacey (City) Parks, Culture & Recreation Department (project owner) proposes to expand and pave a gravel lot to accommodate 350 to 400 parking stalls covering approximately 2.9 acres. Other proposed site improvements include the addition of landscaping, irrigation, fire protection, curbs, striping, lighting, security cameras, and electrical utilities.

Site Conditions

The planned development area includes Thurston County parcel no. 11814410200 and the northwest portion of Thurston County parcel no. 11814410300. The site is primarily surfaced with gravel and asphalt along with smaller areas of grass and trees. Large Douglas fir trees, various fruit trees, and a stormwater infiltration pond are located in the southwest portion of the site. Most of the site is flat and level. The infiltration pond slopes are inclined at approximately 40 percent.

The site is bordered by Marvin Road Southeast to the west, Steilacoom Road Southeast to the north, and internal access drives for the park to the east and south. A southern entrance provides access to the site.

Geologic Setting

Geologic information for the site and the surrounding area was obtained from the *Geologic Map of the Lacey 7.5-minute Quadrangle, Thurston County, Washington* (Logan et al., 2003). Surficial deposits



at the site are mapped as Vashon recessional outwash (Qgo) and Latest Vashon recessional [outwash] sand and minor silt (Qgos). Both materials consist of sand and gravel with cobbles and silt deposited during deglaciation by streams and floods. The subsurface conditions observed in Landau's August 2022 explorations were generally consistent with the mapped geology for the site.

Subsurface Conditions

On August 17, 2022, Landau's excavating subcontractor advanced six test pits (TP-1 through TP-6) 9.3 to 10.5 feet (ft) below ground surface (bgs). The approximate locations of the explorations are shown on Figure 2.

Landau personnel coordinated and monitored the field explorations, collected representative soil samples, and maintained detailed logs of the subsurface soil and groundwater conditions observed. Subsurface conditions were described using the soil classification system shown on Figure 3, and in general accordance with ASTM International (ASTM) standard D2488, *Standard Practice for Description and Identification of Soils (Visual-Manual Procedures).* Summary exploration logs are presented on Figures 4 through 9.

At the completion of laboratory testing, the results will be issued with the final memorandum on Figures 10 through 12.

Soil Conditions

The soils observed in Landau's explorations were categorized into two general units:

- **Fill:** Fill was observed in all the test pits and typically consisted of asphalt and crushed gravel. The fill also consisted of sand with gravel with variable asphalt and plastic content. The fill was in a dense and damp to moist condition extending 0.25 to 1.0 ft bgs at the locations explored.
- **Recessional outwash:** Recessional outwash was observed beneath the fill in all the test pits. The recessional outwash typically consisted of light brown to dark brown or gray to dark gray sand and gravel with variable silt and cobble content in a medium dense to dense and damp tomoist condition. The recessional outwash extended to the maximum depth explored (10.5 ft bgs).

Groundwater Conditions

During Landau's August 2022 field investigation, groundwater was not observed to a maximum depth of 10.5 ft bgs. Site groundwater levels will vary depending on local subsurface conditions, weather conditions, and other factors. Site groundwater levels are expected to fluctuate seasonally, with maximum levels occurring during late winter and early spring.

Conclusions and Recommendations

Based on the subsurface conditions observed in Landau's explorations, site soils will provide adequate support for the proposed improvements, provided the following recommendations are incorporated into the project design.

Pavement

Pavement sections should be constructed on a firm, unyielding subgrade. Landau recommends constructing pavement on the existing fill material or on properly compacted structural fill that extends to the existing fill. Design recommendations for flexible pavement sections are provided in Table 1.

Asphalt pavement sections should be constructed on a compacted subgrade (i.e., on existing fill, recessional outwash, or structural fill), prepared as recommended herein. The light-duty pavement section should be used in areas that will not experience high traffic volume or heavy vehicle loads.

Table 1. Recommended Asphalt Pavement Design Sections

Pavement Section Type ^(a)	Assumed ESALs	•	nent Thickness hes)	Crushed Surfacing Thickness (inches)
Light duty	100,000		3	6
Heavy duty	500,000		3	10

(a) = Assumes pavement section will be founded on a subbase consisting of 12 inches of compacted native soils or fill extending to such soils.

ESALs = equivalent single-axle loads

Base course material should be compacted to at least 95 percent of the maximum dry density, determined in accordance with ASTM standard test method D1557, *Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft³ [2,700 kN-m/m³]).* Compacted base course should meet the requirements for Crushed Surfacing Base Course in Section 9-03.9(3) of the Washington State Department of Transportation's 2022 *Standard Specifications for Road, Bridge, and Municipal Construction (2022 WSDOT Standard Specifications*). To facilitate fine grading of the surface, the upper 2 inches of crushed surfacing could consist of crushed surfacing top course. Prevention of road-base saturation is essential for pavement durability; efforts should be made to limit the amount of water entering the base course.

Asphalt concrete should be Class B aggregate material or hot-mix asphalt class ½-inch and PG58H-22 binder, conforming to the requirements in Section 5-04 of the 2022 WSDOT Standard Specifications. The asphalt should be compacted to at least 91 percent of the Rice density.

Stormwater Infiltration

Infiltration testing was completed using the U.S. Environmental Protection Agency falling head percolation test procedure as provided in Thurston County's 2016 *Drainage Design and Erosion Control Manual* (DDECM). With this procedure, water is infiltrated through a 6-inch-diameter pipe, installed approximately 6 inches below the existing pond's base. In accordance with the DDECM, correction factors to account for test method ($F_{testing} = 0.5$), soil plugging ($F_{plugging} = 1.0$), and pond geometry ($F_{geometry} = 0.49$) were applied to the field infiltration rates to calculate the factored infiltration rates in Table 2.

Table 2. Factored Infiltration Rates

Exploration	Field Infiltration Rate (inches/hour)	Factored Infiltration Rate (inches/hour)
Infil-1	20	4.9
Infil-2	25	6.1

Stormwater facilities should be designed in accordance with local stormwater codes. Permanent stormwater pond slopes should be no steeper than 3 horizontal to 1 vertical (3H:1V). Permanent and temporary slopes should be protected from erosion and reseeded, or revegetated as soon as practical.

Construction Considerations

The following key points should be reviewed when developing project specifications:

- **Stripping:** Topsoil was not observed in the test pits; however, the few inches of grass covering the existing gravel and pavement must be removed. Although not observed in the test pits, the western portion of the site may have topsoil that should be stripped if encountered during construction of pavement sections. Stripped soils are not suitable for reuse as structural fill.
- Subgrade preparation: Landau recommends that pavements are constructed on existing fill, compacted native soils, or structural fill extending to such soils. Before structural fill or pavement base course is placed, the prepared subgrade should be proof-rolled in the presence of a qualified geotechnical engineer who is familiar with the site and can check for soft/disturbed areas. Areas of limited access can be evaluated with a steel T-probe. If probing or proof-rolling reveals loose and/or disturbed subgrades, the upper 1 ft of subgrade should be scarified, moisture-conditioned, and compacted to a firm, unyielding condition. Alternatively, unsuitable soils can be over-excavated and replaced with properly compacted import structural fill.
- Utility trench excavation and backfill: Landau anticipates that utility trenches will be excavated in medium dense to dense, sand and gravel with cobbles. A heavy-duty hydraulic excavator should be able to reach the required trench depths (up to 10 ft bgs). A smooth-

bladed bucket should be used to remove loose and/or disturbed soil from the trench bottom. The final trench bottom should be firm and free of roots, topsoil, lumps of silt and clay, construction debris, and organic and inorganic debris.

- Site soil: The site soils are generally suitable for use as structural fill. The contractor should be prepared to moisture condition site soils before compacting. Earthwork should be avoided during heavy and/or extended periods of precipitation.
- **Oversized material:** Cobbles were observed in the explorations and throughout the site at the surface. The contractor should be prepared to manage cobbles and boulders, as well as debris or rubble from previous site development and/or filling activities.
- Import structural fill: Gravel Borrow, as described in Section 9-03.14(1) of the 2022 WSDOT Standard Specifications, is a suitable source of import structural fill. During periods of wet weather, the fines content should not exceed 5 percent, based on the minus ³/₄-inch fraction.
- Fill placement and compaction: Structural fill should be placed on an approved subgrade that consists of uniformly firm and unyielding native soils, existing fill, or on previously placed and compacted structural fill extending to such soils. Structural fill should be placed and compacted in accordance with the requirements in Section 2-03.3(14)C, Method C of the 2022 WSDOT Standard Specifications. Method A is appropriate for non-structural areas such as landscaping. Each layer of structural fill should be compacted to at least 95 percent of the maximum dry density, determined in accordance with the compaction control tests in Section 2-03.3(14)D of the 2022 WSDOT Standard Specifications. Alternatively, the maximum dry density can be determined using ASTM standard test method D1557.
- **Construction dewatering:** During Landau's August 2022 field investigation, groundwater was not observed to 10.5 ft bgs. Groundwater was not observed at the base of the existing pond during infiltration testing. Temporary excavations that encounter groundwater should be dewatered to allow construction to be completed in the dry. Where shallow groundwater seepage is encountered, the use of conventional sumps and pumps should be sufficient to dewater excavations. The contractor should be responsible for the design, monitoring, and maintenance of any dewatering systems.
- Temporary slopes: Temporary excavations should be completed in accordance with Section 2-09 of the 2022 WSDOT Standard Specifications. The contractor should be responsible for actual excavation configurations and the maintenance of safe working conditions including temporary excavation stability. Temporary excavations in excess of 4 ft should be shored or sloped in accordance with the requirements outlined in Safety Standards for Construction Work, Part N (Washington Administrative Code Chapter 296-155). The soil likely to be exposed in the excavations should be considered Type C, with a maximum allowable excavation inclination of 1½H:1V. All applicable local, state, and federal safety codes should be followed.

Permanent slopes: Permanent cut-and-fill slopes should be no steeper than 2H:1V. This design recommendation does not apply to stormwater pond slopes, which are typically 3H:1V or flatter. Stormwater pond slopes should be designed in accordance with local stormwater codes. Permanent and temporary slopes should be protected from erosion and reseeded or revegetated as soon as practical.

Use of This Technical Memorandum

Landau Associates, Inc. has prepared this technical memorandum for the exclusive use of SCJ Alliance for specific application to the RAC Parking Lot Design project in Lacey, Washington. No other party is entitled to rely on the information, conclusions, and recommendations included in this document without the express written consent of Landau. Reuse of the information, conclusions, and recommendations provided herein for extensions of the project or for any other project, without review and authorization by Landau shall be at the user's sole risk. Landau warrants that, within the limitations of scope, schedule, and budget, its services have been provided in a manner consistent with that level of skill and care ordinarily exercised by members of the profession currently practicing in the same locality, under similar conditions as this project. Landau makes no other warranty, either express or implied.

Closing

We trust that this memorandum provides you with the information needed to proceed with the project. If you have questions or comments, or if we can be of further service, please contact Lance Levine at 360.791.3178 or at llevine@landauinc.com.

LANDAU ASSOCIATES, INC.

Lance Levine, PE Senior Engineer

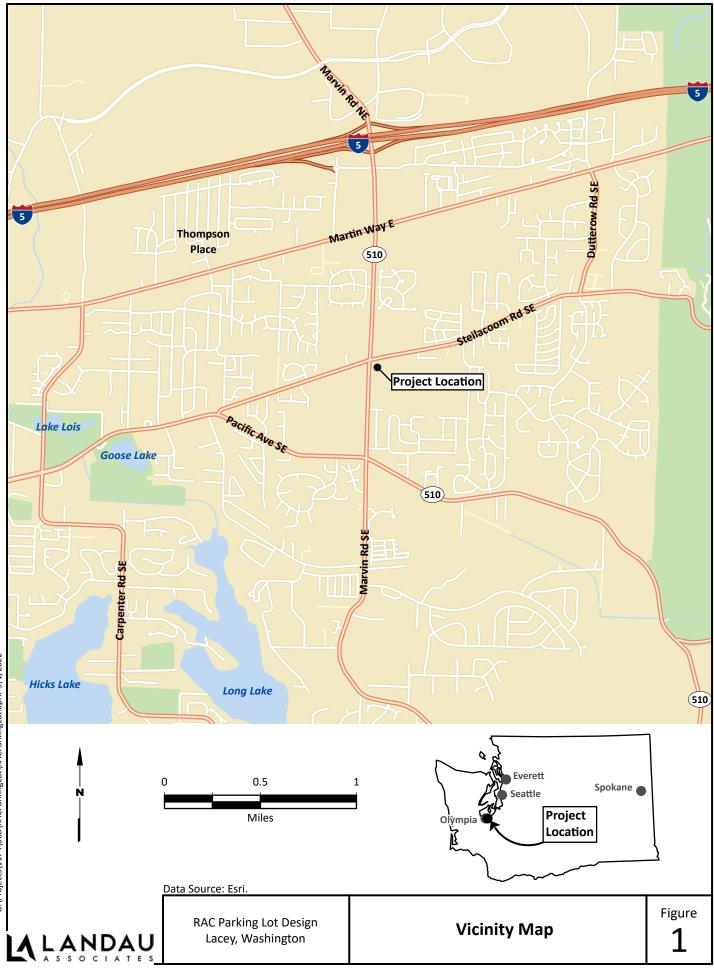
Steven R. Wright, PE Principal

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Attachments: Figure 1. Vicinity Map Figure 2. Site Exploration and Location Plan Figure 3. Soil Classification System and Key Figures 4–9. Logs of Test Pits TP-1 through TP-6 Figures 10–12. Grain Size Distribution—To be provided after completion of lab testing

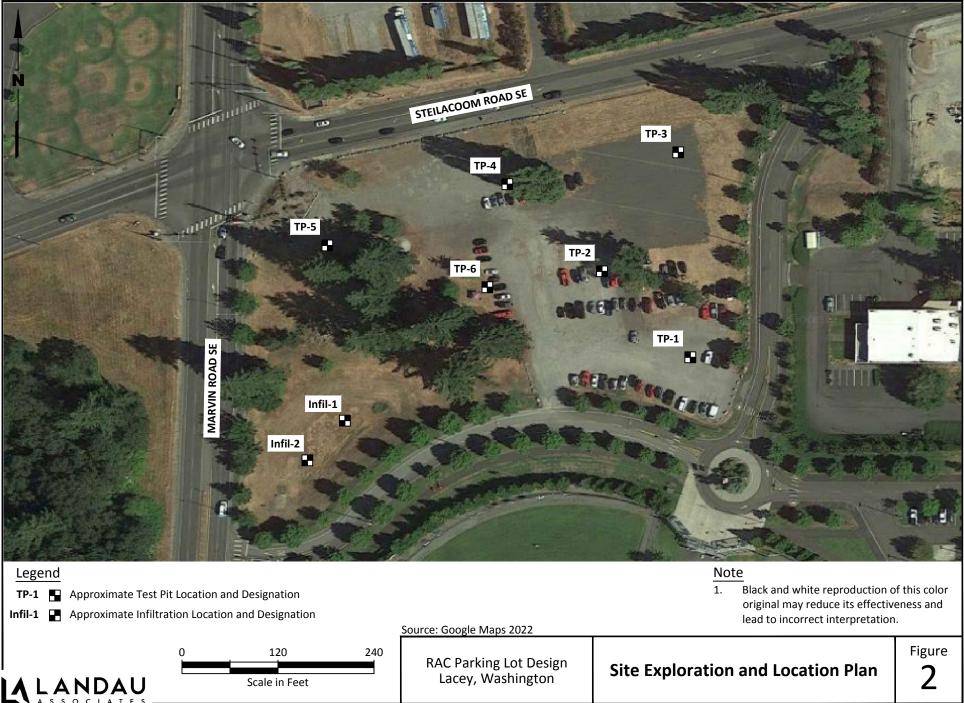
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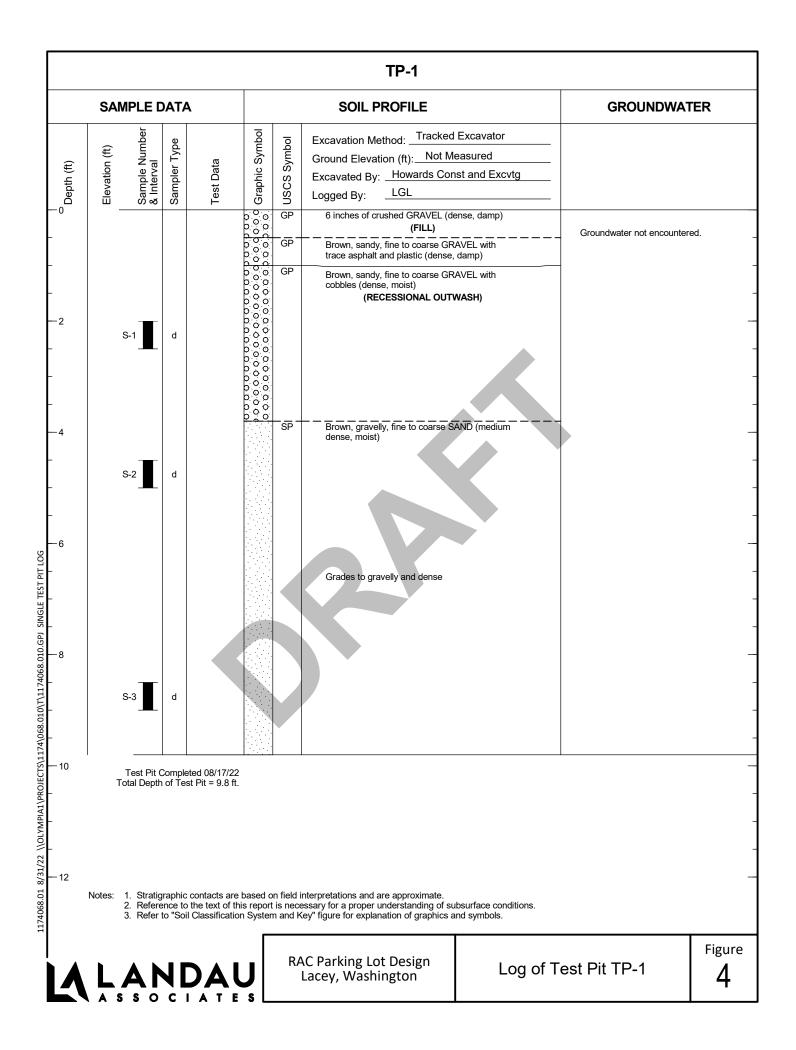


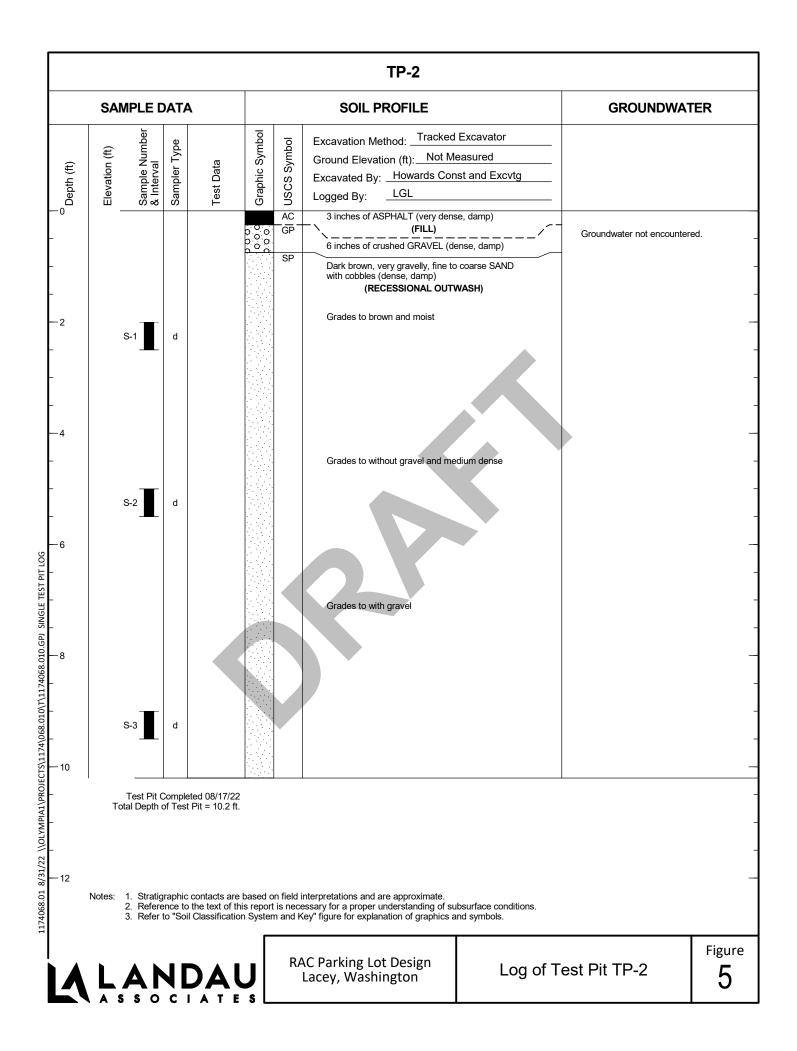
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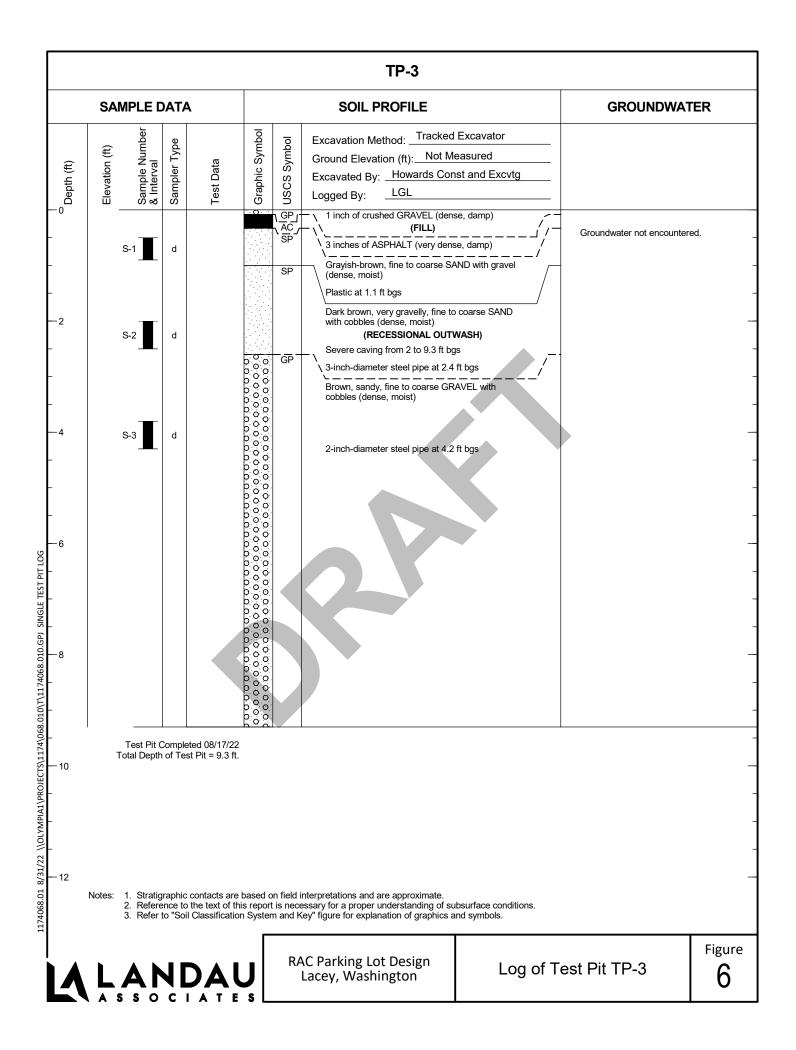
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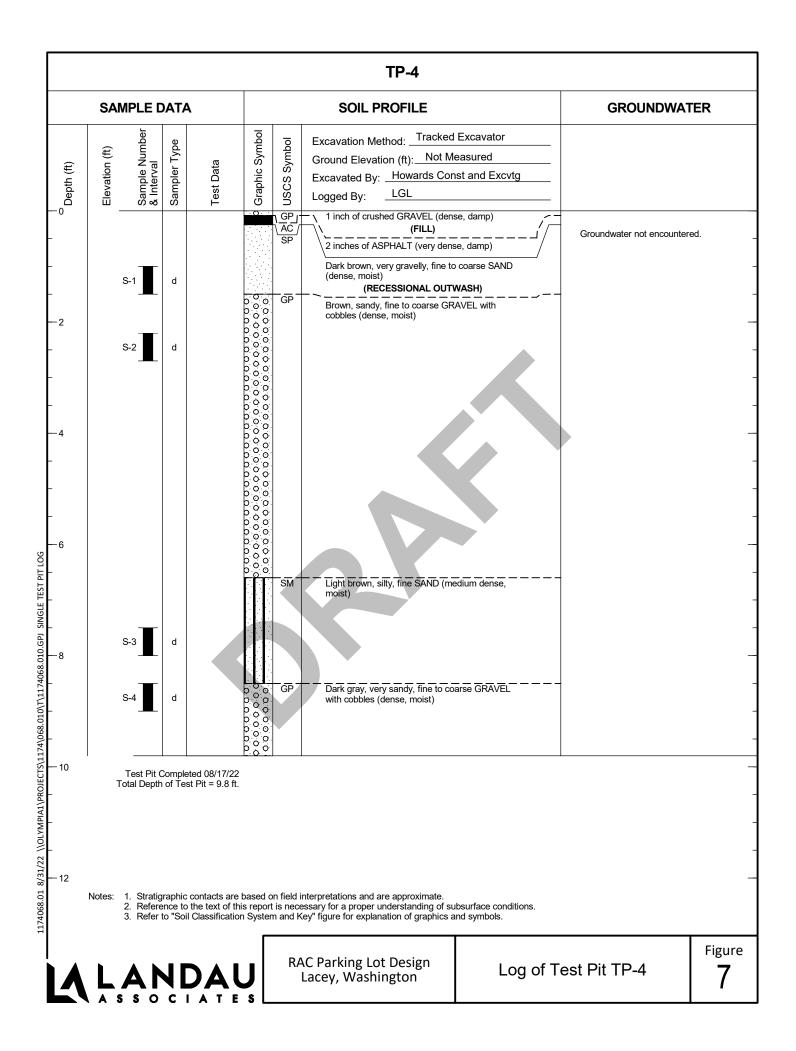


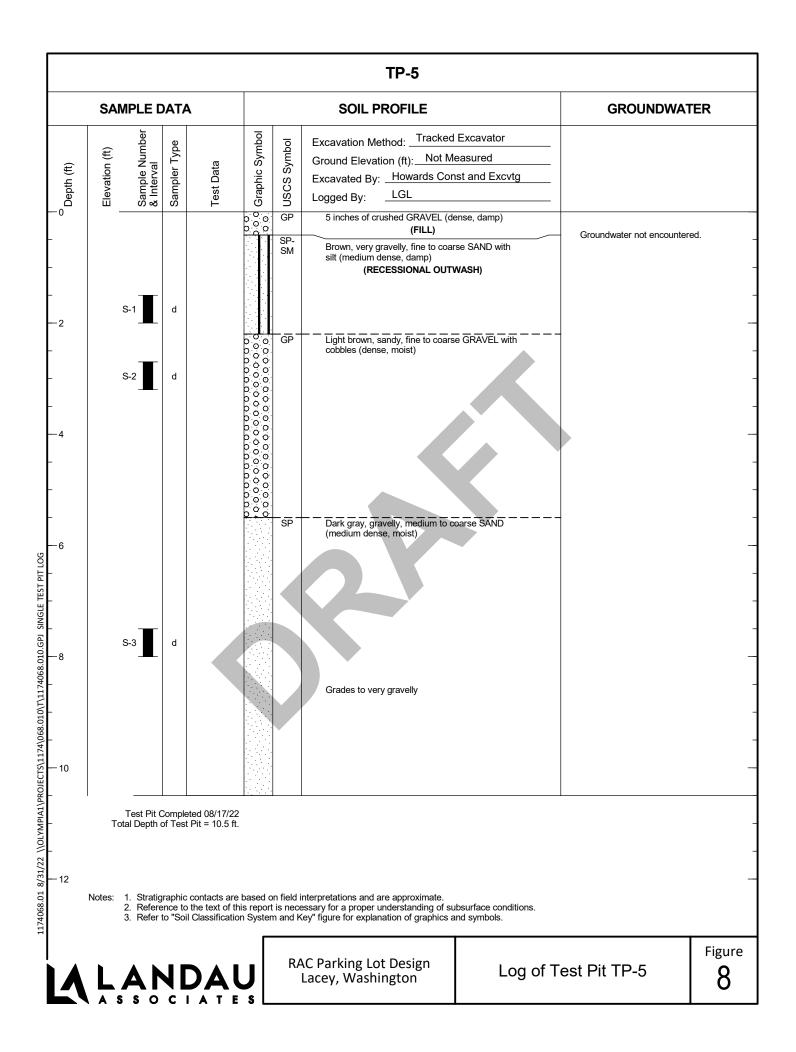
	MAJOR DIVISIONS		GRAPHIC SYMBOL	USCS LETTER SYMBOL ⁽¹⁾	DE	TYPICAL ESCRIPTIONS ⁽²⁾⁽³⁾	
	GRAVEL AND	CLEAN GRAVEL		GW		vel; gravel/sand mixture(s); little or no fi	nes
COARSE-GRAINED SOIL (More than 50% of material is arger than No. 200 sieve size)	GRAVELLY SOIL	(Little or no fines)		GP	Poorly graded gr	avel; gravel/sand mixture(s); little or no	fines
ED S lieve	(More than 50% of	GRAVEL WITH FINES	EFEEE	GM	Silty gravel; grav	rel/sand/silt mixture(s)	
AINE of m 200 s	coarse fraction retained on No. 4 sieve)	(Appreciable amount of fines)	11/1	GC	Clayey gravel; gr	ravel/sand/clay mixture(s)	
COARSE-GRAINED SOIL (More than 50% of material is larger than No. 200 sieve size)	SAND AND	CLEAN SAND		SW	Well-graded san	d; gravelly sand; little or no fines	
RSE than than	SANDY SOIL	(Little or no fines)		SP	Poorly graded sa	and; gravelly sand; little or no fines	
:OAF More trger	(More than 50% of coarse fraction passed	SAND WITH FINES (Appreciable amount of		SM	Silty sand; sand/	silt mixture(s)	
	through No. 4 sieve)	fines)		SC		nd/clay mixture(s)	
SOIL of r than ize)	SILT A	ND CLAY	<u>IIIII</u>	ML		l very fine sand; rock flour; silty or claye It with slight plasticity	
D S 0% o ller tl e size	(Liquid limit	t less than 50)		CL	Inorganic clay of clay; silty clay; le	low to medium plasticity; gravelly clay; an clay	sandy
UNE an 5 sma sieve				OL	Organic silt; orga	anic, silty clay of low plasticity	
FINE-GRAINED SOIL (More than 50% of material is smaller than No. 200 sieve size)	SILT A	ND CLAY		MH	0	caceous or diatomaceous fine sand	
NE No. No.	Liauid limit (greater than 50)		CH	· ·	high plasticity; fat clay	
				OH	0 ,	nedium to high plasticity; organic silt	
	HIGHLY OF	RGANIC SOIL		PT	Peat; humus; sw	amp soil with high organic content	
	OTHER MAT	ERIALS	GRAPHIC SYMBOL		ТҮРІС	CAL DESCRIPTIONS	
	PAVEME	NT	•	AC or PC	Asphalt concrete	e pavement or Portland cement paveme	ent
	ROCK	(RK	Rock (See Rock	Classification)	
	WOOL)		WD	Wood, lumber, w	vood chips	
	DEBRI	S		DB	Construction deb	oris, garbage	
Me 3. Soil	ethod for Classification of So I description terminology is follows: Primary (bils for Engineering Purposes based on visual estimates (ir	s, as outlined in In the absence o	ASTM D 2487. of laboratory tes	t data) of the perce	ns are based on the Standard Test ntages of each soil type and is defined	
4 5-	Additional Co	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5	9% - "very grave 9% - "gravelly," 5% - "with grave 5% - "with trace	elly," "very sand "sandy," "silty," el," "with sand," gravel," "with ti	etc. "with silt," etc. race sand," "with tra	ace silt," etc., or not noted.	
	Additional Co	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5	9% - "very grave 9% - "gravelly," 5% - "with grave 5% - "with trace	elly," "very sand "sandy," "silty," el," "with sand," gravel," "with ti	y," "very silty," etc. etc. "with silt," etc. race sand," "with tra	ace silt," etc., or not noted. blow counts, drilling or excavating	
	Additional Co I density or consistency des nditions, field tests, and labo Drilling a	$\begin{array}{rrrr} \text{source} &> 30\% \text{ and} \leq 50\\ &> 15\% \text{ and} \leq 30\\ \text{constituents:} &> 5\% \text{ and} \leq 15\\ &\leq 5\\ \text{constituents:} &> 5\% \text{ and} \text{ or } 15\\ &\leq 5\\ \text{constituents:} &= 5\\ cons$	9% - "very grave 1% - "gravelly," 5% - "with grave 5% - "with trace ment using a c y	elly," "very sand "sandy," "silty," I," "with sand," gravel," "with tr ombination of s	y," "very silty," etc. etc. "with silt," etc. race sand," "with tra ampler penetration		
cor	Additional Co I density or consistency des nditions, field tests, and labo Drilling a SAMPLER TYPE & M	$\begin{array}{llllllllllllllllllllllllllllllllllll$	9% - "very grave 1% - "gravelly," 1% - "with grave 1% - "with trace 1% - "with trace	elly," "very sand "sandy," "silty," I," "with sand," gravel," "with tr ombination of s	ly," "very silty," etc. etc. "with silt," etc. race sand," "with tra ampler penetration Fie	blow counts, drilling or excavating	
cor Graphic	Additional Co I density or consistency des nditions, field tests, and labo Drilling a SAMPLER TYPE & M Code Desc a 3.25-in OD, 2.42-in	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5 coriptions are based on judge pratory tests, as appropriate. ID Sampling Ke SAMPLE N D Split Spoon	1% - "very grava 1% - "gravelly," 1% - "with grave 1% - "with trace ment using a c y NUMBER & I	elly," "very sand "sandy," "silty," I," "with sand," gravel," "with tr ombination of s	y," "very silty," etc. etc. "with silt," etc. race sand," "with tra- ampler penetration Fiel Code PP = 1.0	blow counts, drilling or excavating	
cor Graphic	Additional Control density or consistency des nditions, field tests, and labor Drilling a SAMPLER TYPE & M Code Desc a 3.25-in OD, 2.42-in b 2.00-in OD, 1.50-in c Shelby Tube	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5 coriptions are based on judge pratory tests, as appropriate. ID Sampling Ke SAMPLE N D Split Spoon	1% - "very grave % - "gravelly," % - "with grave % - "with trace ment using a c y NUMBER & I Sample Identifi	elly," "very sand "sandy," "silty," el," "with sand," gravel," "with tr ombination of s NTERVAL	y," "very silty," etc. etc. "with silt," etc. "ace sand," "with tra- ampler penetration Fiel - Code PP = 1.0	blow counts, drilling or excavating	ning, ppm
cor Graphic	Additional Co I density or consistency des nditions, field tests, and labo Drilling a SAMPLER TYPE & M Code Desc a 3.25-in OD, 2.42-in b 2.00-in OD, 1.50-in c Shelby Tube d Grab Sample e Single-Tube Core E	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5 criptions are based on judge pratory tests, as appropriate. Ind Sampling Ke ETHOD SAMPLE N D Split Spoon ID Split Spoon Barrel	1% - "very grave 1% - "gravelly," 1% - "with grave 1% - "with trace ment using a c y <u>VUMBER & I</u> Sample Identifi Sampler Gra	elly," "very sand "sandy," "silty," el," "with sand," gravel," "with tr ombination of s NTERVAL cation Number	y," "very silty," etc. etc. "with silt," etc. race sand," "with tra- ampler penetration Fiel - - Code PP = 1.0 TV = 0.5 PID = 100 W = 10	blow counts, drilling or excavating Id and Lab Test Data Description Pocket Penetrometer, tsf Torvane, tsf Photoionization Detector VOC scree Moisture Content, %	ning, ppm
Graphic X X G	Additional Control I density or consistency des inditions, field tests, and labor Drilling a SAMPLER TYPE & M Code Desc a 3.25-in OD, 2.42-in b 2.00-in OD, 1.50-in c Shelby Tube d Grab Sample e Single-Tube Core E f Double-Tube Core g 2.50-in OD, 2.00-in	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5 corriptions are based on judge pratory tests, as appropriate. ID Split Spoon ID Split Spoon ID Split Spoon Barrel Barrel ID WSDOT	1% - "very grave % - "gravelly," % - "with grave % - "with trace ment using a c 	elly," "very sand "sandy," "silty," el," "with sand," gravel," "with tri ombination of s <u>NTERVAL</u> cation Number aphic (variable)	y," "very silty," etc. etc. "with silt," etc. "ace sand," "with tra- ampler penetration Fiel - - Code PP = 1.0 TV = 0.5 PID = 100 W = 10 D = 120 -200 = 60	blow counts, drilling or excavating Id and Lab Test Data Description Pocket Penetrometer, tsf Torvane, tsf Photoionization Detector VOC scree Moisture Content, % Dry Density, pcf Material smaller than No. 200 sieve,	%
cor Graphic	Additional Control density or consistency desinditions, field tests, and labor Drilling a SAMPLER TYPE & M Code Desc a 3.25-in OD, 2.42-in b 2.00-in OD, 1.50-in c Shelby Tube d Grab Sample e Single-Tube Core E f Double-Tube Core E f Double-Tube Core E g 2.50-in OD, 2.00-in h 3.00-in OD, 2.37-in j Other - See text if a 1 300-lb Hammer, 30	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5 coriptions are based on judge pratory tests, as appropriate. ID Split Spoon ID Split Spoon ID Split Spoon	% - "very grave" % - "gravelly," % - "with grave % - "with trace ment using a c y <u>VUMBER & I</u> Sample Identifi Sample Identifi Recovery] ← Sample - Portion of Sa	elly," "very sand "sandy," "silty," el," "with sand," gravel," "with tri ombination of s <u>NTERVAL</u> cation Number aphic (variable)	y," "very silty," etc. etc. "with silt," etc. "ace sand," "with tra- ampler penetration Fiel - Code PP = 1.0 TV = 0.5 PID = 100 W = 10 D = 120 -200 = 60 GS AL GT	blow counts, drilling or excavating Id and Lab Test Data Description Pocket Penetrometer, tsf Torvane, tsf Photoionization Detector VOC scree Moisture Content, % Dry Density, pcf Material smaller than No. 200 sieve, Grain Size - See separate figure for Atterberg Limits - See separate figure Other Geotechnical Testing	% data
Graphic	Additional Control density or consistency designations, field tests, and labor Drilling a SAMPLER TYPE & M Code Desc a 3.25-in OD, 2.42-in b 2.00-in OD, 1.50-in c Shelby Tube d Grab Sample e Single-Tube Core E f Double-Tube Core g 2.50-in OD, 2.00-in h 3.00-in OD, 2.03-in h 3.00-in OD, 2.37-in i Other - See text if a 1 300-lb Hammer, 30 2 140-lb Hammer, 30 3 Pushed Sample	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5 comptions are based on judge pratory tests, as appropriate. nd Sampling Ke ETHOD SAMPLE N Comption ID Split Spoon ID Mod. Calif. pplicable -inch Drop -inch Drop	% - "very grave", % - "gravelly," % - "with grave % - "with trace ment using a o y NUMBER & I Sample Identifi Sample Identifi Sample Identifi Sample Identifi Sample Gra Recovery] ← Sample	elly," "very sand, "sandy," "silty," el," "with sand," gravel," "with tri ombination of s <u>NTERVAL</u> cation Number aphic (variable) Depth Interval Depth Interval umple Retained ive or Analysis	y," "very silty," etc. etc. "with silt," etc. race sand," "with tra- ampler penetration Fiel - Code PP = 1.0 TV = 0.5 PID = 100 W = 10 D = 120 -200 = 60 GS AL	blow counts, drilling or excavating Id and Lab Test Data Description Pocket Penetrometer, tsf Torvane, tsf Photoionization Detector VOC scree Moisture Content, % Dry Density, pcf Material smaller than No. 200 sieve, Grain Size - See separate figure for Atterberg Limits - See separate figure	% data
Graphic	Additional Control density or consistency desinditions, field tests, and labor Drilling a SAMPLER TYPE & M Code Desco a 3.25-in OD, 2.42-in b 2.00-in OD, 1.50-in c Shelby Tube d Grab Sample e Single-Tube Core E f Double-Tube Core E g 2.50-in OD, 2.00-in h 3.00-in OD, 2.37-in i Other - See text if a 1 300-lb Hammer, 30 2 140-lb Hammer, 30	onstituents: > 30% and ≤ 50 > 15% and ≤ 30 onstituents: > 5% and ≤ 15 ≤ 5 criptions are based on judge pratory tests, as appropriate. ID Split Spoon ID Split Spoon ID Split Spoon	 % - "very grave", "gravelly," "gravelly," "with grave "with grave "with grave ment using a complexity of the second seco	elly," "very sand "sandy," "silty," el, "with sand," gravel," "with sand," gravel," "with tr ombination of s <u>NTERVAL</u> cation Number aphic (variable) Depth Interval Depth Interval ive or Analysis atter r level at time c	y," "very silty," etc. etc. "with silt," etc. race sand," "with tra- ampler penetration Fiel - Code PP = 1.0 TV = 0.5 PID = 100 W = 10 D = 120 -200 = 60 GS AL GT CA	blow counts, drilling or excavating Id and Lab Test Data Description Pocket Penetrometer, tsf Torvane, tsf Photoionization Detector VOC scree Moisture Content, % Dry Density, pcf Material smaller than No. 200 sieve, Grain Size - See separate figure for Atterberg Limits - See separate figur Other Geotechnical Testing Chemical Analysis	% data

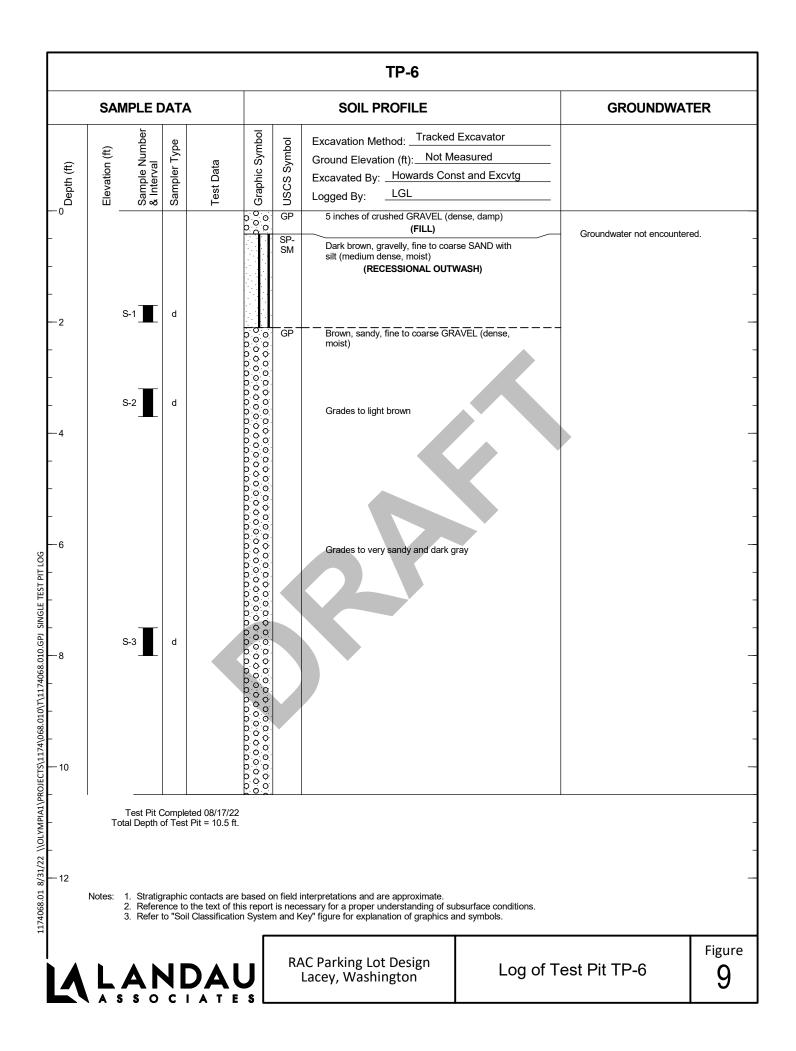












Drainage Control Plan Attachment 4

Maintenance and Source Control Manual (Not Included, Attached Separately)

Drainage Control Plan Attachment 5

Establishment of Maintenance Covenant

Appendix 1 Design Calculations

<section-header>

General Model Information

Project Name:	22-000313 RAC
Site Name:	
Site Address:	
City:	
Report Date:	1/23/2023
Gage:	Fairgrounds (Kaiser)
Data Start:	1955/10/01
Data End:	2011/09/30
Timestep:	15 Minute
Precip Scale:	1.000
Version Date:	2021/08/18
Version:	4.2.18

POC Thresholds

Low Flow Threshold for POC1:	50 Percent of the 2 Year
High Flow Threshold for POC1:	50 Year

Landuse Basin Data Predeveloped Land Use

Basin 1

Surface

Bypass:	No
GroundWater:	No
Pervious Land Use A B, Forest, Flat	acre 5.04
Pervious Total	5.04
Impervious Land Use	acre
Impervious Total	0
Basin Total	5.04
Element Flows To:	

Interflow

Groundwater

Mitigated Land Use

Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use A B, Lawn, Flat	acre 1.55
Pervious Total	1.55
Impervious Land Use ROADS FLAT	acre 3.49
Impervious Total	3.49
Basin Total	5.04

Element Flows To:		
Surface	Interflow	Groundwater
Gravel Trench Bed 1	Gravel Trench Bed 1	

Routing Elements Predeveloped Routing

Mitigated Routing

Gravel Trench Bed 1 ← Bioretention Bottom Length: Bottom Width: Trench bottom slope 1: Trench Left side slope 0: Trench right side slope 2: Material thickness of first layer: Pour Space of material for first layer: Material thickness of second layer: Pour Space of material for second layer: Pour Space of material for second layer: Naterial thickness of third layer: Pour Space of material for third layer: Infiltration On Infiltration rate: Infiltration safety factor: Wetted surface area On Total Volume Infiltrated (ac-ft.): Total Volume Through Riser (ac-ft.): Percent Infiltrated: Total Precip Applied to Facility:	50.00 ft. 20.00 ft. 3 To 1 3 To 1 3 To 1 1.5 0.4 0 0 0 0 0 0 0 0 0 0 0 0 0) 12 in/hr with a factor of safety of 4 per SDM design standards 91% infiltrated per treatment design standards in chapter 8 of the SDM
Total Volume Infiltrated (ac-ft.): Total Volume Through Riser (ac-ft.): Total Volume Through Facility (ac-ft.): Percent Infiltrated:	62.973 709.405 91.12 ←	

Gravel Trench Bed Hydraulic Table

Stage(feet) 0.0000	Area(ac.) 0.023	Volume(ac-ft.) 0.000	0.000	0.000
0.0500	0.023	0.000	0.000	0.070
0.1000	0.023 0.024	0.000	0.000	0.072
0.1500 0.2000	0.024	0.001 0.001	0.000 0.000	0.073 0.075
0.2500	0.024	0.002	0.000	0.075
0.2000	0.025	0.002	0.000	0.078
0.3500	0.026	0.002	0.000	0.080
0.4000	0.026	0.004	0.000	0.081
0.4500	0.027	0.004	0.000	0.083
0.5000	0.028	0.005	0.000	0.084
0.5500	0.028	0.005	0.000	0.086
0.6000	0.029	0.006	0.000	0.087
0.6500	0.029	0.006	0.000	0.089
0.7000	0.030	0.007	0.000	0.091
0.7500	0.030	0.008	0.000	0.092
0.8000	0.031	0.008	0.000	0.094
0.8500	0.031	0.009	0.000	0.096
0.9000	0.032	0.009	0.000	0.097
0.9500	0.032	0.010	0.000	0.099
1.0000	0.033	0.011	0.000	0.101
1.0500 1.1000	0.034 0.034	0.011 0.012	0.000 0.000	0.102 0.104
1.1000	0.034	0.012	0.000	0.104

1.1500 1.2000 1.2500 1.3000 1.3500 1.4000 1.4000 1.4500 1.5000 1.5000 1.6000 1.6500 1.7000 1.7500 1.8000 1.9000 2.0000 2.0500 2.0000 2.0500 2.1500 2.2500 2.3000 2.3500 2.3500 2.4000 2.5500 2.6500 2.5500 2.6500 2.7500 2.8500 2.9500 3.0000 3.0500 3.1500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.2500 3.25	0.035 0.036 0.036 0.037 0.038 0.039 0.039 0.039 0.040 0.041 0.041 0.042 0.043 0.043 0.043 0.044 0.045 0.046 0.046 0.046 0.046 0.047 0.048 0.048 0.048 0.048 0.049 0.050 0.051 0.052 0.052 0.052 0.052 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.056 0.057 0.057 0.058 0.059 0.060 0.061 0.062 0.063 0.063 0.064 0.065 0.066 0.067 0.069 0.070	0.013 0.014 0.014 0.015 0.016 0.016 0.017 0.021 0.023 0.025 0.027 0.029 0.032 0.034 0.036 0.038 0.040 0.043 0.045 0.047 0.050 0.052 0.055 0.057 0.060 0.062 0.065 0.067 0.070 0.070 0.073 0.076 0.070 0.073 0.076 0.078 0.081 0.084 0.081 0.084 0.084 0.087 0.090 0.099 0.102 0.105 0.105 0.108 0.111 0.124 0.124 0.138 0.141	0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0	0.106 0.108 0.109 0.111 0.113 0.115 0.117 0.120 0.122 0.124 0.126 0.128 0.130 0.132 0.133 0.135 0.137 0.139 0.141 0.143 0.145 0.147 0.149 0.141 0.145 0.147 0.149 0.151 0.153 0.155 0.160 0.162 0.164 0.168 0.170 0.175 0.177 0.179 0.181 0.183 0.188 0.190 0.192 0.202 0.204 0.209 0.214
3.6000 3.6500	0.068 0.069	0.131 0.134	0.276 0.497	0.206 0.209

4.0500 4.1000 4.1500	0.075 0.076 0.077	0.163 0.167 0.171	1.622 1.694 1.763	0.228 0.231 0.233
4.2000	0.078	0.175	1.830	0.236
4.2500	0.078	0.179	1.894	0.238
4.3000	0.079	0.183	1.956	0.241
4.3500	0.080	0.187	2.016	0.243
4.4000	0.081	0.191	2.075	0.246
4.4500	0.082	0.195	2.131	0.248
4.5000	0.083	0.199	2.187	0.251

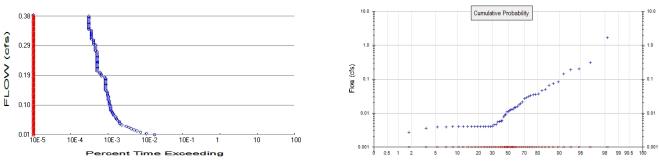
Trapezoidal Pond 1 Bottom Length: Bottom Width: Depth: Volume at riser head: Infiltration On Infiltration rate: Infiltration safety factor: Wetted surface area On Total Volume Infiltrated (ac Total Volume Through Rise Total Volume Through Rise Total Volume Through Fac Percent Infiltrated: Total Precip Applied to Fac Total Evap From Facility: Side slope 1: Side slope 2:	80.00 ft. 79.30 ft. 4 ft. 0.5480 acre-feet. 5.5 1 -ft.): er (ac-ft.): ility (ac-ft.): ility: 3 To 1 3 To 1 3 To 1	ntion facility 62.972 0 62.972 100 ← 0 0	Retention facility infiltrates 100% of the remaining storm water runoff
	ility:		
	3 To 1		
	3 To 1		
Side slope 3:	3 To 1		
Side slope 4:	3 To 1		
Discharge Structure			
Riser Height:	3 ft.		
Riser Diameter: Element Flows To:	12 in.		
Outlet 1 Outl	et 2		

Pond Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)		
0.0000	0.145	0.000	0.000	0.000
0.0444	0.146	0.006	0.000	0.813
0.0889	0.147	0.013	0.000	0.818
0.1333	0.148	0.019	0.000	0.824
0.1778	0.149	0.026	0.000	0.829
0.2222	0.150	0.032	0.000	0.835
0.2667	0.151	0.039	0.000	0.840
0.3111	0.152	0.046	0.000	0.846
0.3556	0.153	0.053	0.000	0.851
0.4000	0.154	0.060	0.000	0.857
0.4444	0.155	0.066	0.000	0.862
0.4889	0.156	0.073	0.000	0.868
0.5333	0.157	0.080	0.000	0.873
0.5778	0.158	0.087	0.000	0.879
0.6222	0.159	0.094	0.000	0.885
0.6667	0.160	0.102	0.000	0.890
0.7111	0.161	0.109	0.000	0.896
0.7556	0.162	0.116	0.000	0.902
0.8000	0.163	0.123	0.000	0.908
0.8444	0.164	0.131	0.000	0.913
0.8889	0.165	0.138	0.000	0.919
0.9333	0.166	0.145	0.000	0.925
0.9778	0.167	0.153	0.000	0.931
1.0222	0.168	0.160	0.000	0.936
1.0667	0.170	0.168	0.000	0.942
1.1111	0.171	0.175	0.000	0.948
1.1556	0.172	0.183	0.000	0.954
1.2000	0.173	0.191	0.000	0.960

3.8222	0.241	0.732	2.856	1.339
3.8667	0.242	0.743	2.932	1.346
3.9111	0.244	0.753	3.006	1.353
3.9556	0.245	0.764	3.078	1.360
4.0000	0.246	0.775	3.149	1.367
4.0444	0.247	0.786	3.218	1.374

Analysis Results POC 1



+ Predeveloped



Predeveloped Landuse	Totals for POC #1
Total Pervious Area:	5.04
Total Impervious Area:	0

Mitigated Landuse Totals for POC #1 Total Pervious Area: 1.55 3.49 **Total Impervious Area:**

Flow Frequency Method: Log Pearson Type III 17B

Flow Frequency Return Periods for Predeveloped. POC #1 **Return Period** Flow(cfs) 0.01156 2 year 0.040618 5 year 10 year 0.086265 25 year 0.207785 50 year 0.382628 100 year 0.682267

Flow Frequency Return Periods for Mitigated. POC #1 Return Period Flow(cfs) 2 year 0

5 year	0
10 year	0
25 year	0
50 year	0
100 year	0

Annual Peaks

Annual Peaks for Predeveloped and Mitigated. POC #1 ed

Year	Predeveloped	Mitigate
1956	0.035	0.000
1957	0.009	0.000
1958	0.006	0.000
1959	0.013	0.000
1960	0.012	0.000
1961	0.016	0.000
1962	0.004	0.000
1963	0.199	0.000
1964	0.050	0.000
1965	0.018	0.000

1966 1967 1968 1969 1970 1971 1972 1973 1974 1975 1976 1977 1978 1979 1980 1981 1982 1983 1984 1985 1986 1987 1988 1989 1990 1991 1992 1993 1994 1995 1996 1997 1998 1999 2000 2001 2002 2003 2004 2005 2006 2007 2008 2009 2010	0.004 0.015 0.005 0.006 0.011 0.075 0.066 0.004 0.028 0.006 0.033 0.004 0.027 0.004 0.027 0.004 0.004 0.014 0.004 0.004 0.004 0.004 0.004 0.005 0.310 1.667 0.189 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.003 0.004 0.004 0.004 0.003 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0	0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000
2011	0.019	0.000

Ranked Annual Peaks

 Ranked Annual Peaks for Predeveloped and Mitigated. POC #1

 Rank
 Predeveloped Mitigated

 1
 1.6665
 0.0000

 2
 0.3100
 0.0000

 3
 0.1990
 0.0000

 4
 0.1893
 0.0000

 5
 0.1441
 0.0000

 6
 0.0838
 0.0000

 7
 0.0747
 0.0000

 8
 0.0657
 0.0000

$\begin{array}{c} 9\\ 10\\ 11\\ 12\\ 13\\ 14\\ 15\\ 16\\ 17\\ 18\\ 19\\ 20\\ 21\\ 22\\ 23\\ 24\\ 25\\ 26\\ 27\\ 28\\ 29\\ 30\\ 31\\ 32\\ 33\\ 34\\ 35\\ 36\\ 37\\ 38\\ 39\\ 40\\ 41 \end{array}$	0.0503 0.0464 0.0369 0.0360 0.0349 0.0329 0.0305 0.0285 0.0271 0.0211 0.0191 0.0191 0.0157 0.0157 0.0148 0.0144 0.0133 0.0129 0.0125 0.0112 0.0112 0.0112 0.0091 0.0091 0.0087 0.0058 0.0057 0.0054 0.0046 0.0041 0.0041 0.0040	0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
38	0.0044	0.0000
42	0.0040	0.0000
43	0.0040	0.0000
44 45	0.0040 0.0040	0.0000
46	0.0040	0.0000
47	0.0040	0.0000
48	0.0040	0.0000
49	0.0040	0.0000
50	0.0040	0.0000
51	0.0040	0.0000
52	0.0039	0.0000
53	0.0039	0.0000
54	0.0036	0.0000
55	0.0027	0.0000
55 56	0.0027	0.0000

Duration Flows

The Facility PASSED

Flow(cfs) 0.0058 0.0096 0.0134 0.0172 0.0210 0.0248	Predev 347 210 146 115 97 83	Mit 0 0 0 0 0 0	Percentage 0 0 0 0 0 0 0	Pass/Fail Pass Pass Pass Pass Pass Pass Pass
0.0286	72	0	0	Pass
0.0324	60	0	0	Pass
0.0362	46	0	0	Pass
0.0400	41	0	0	Pass
0.0438	40	0	0	Pass
0.0477	39	0	0	Pass
0.0515	36	0	0	Pass
0.0553	33	0	0	Pass
0.0591	32	0	0	Pass
0.0629	31	0	0	Pass
0.0667	28	0	0	Pass
0.0705	27	0	0	Pass
0.0743	26	0	0	Pass
0.0781 0.0819	25 24 23	0 0 0	0 0 0	Pass Pass
0.0857 0.0895 0.0933	23 23	0 0	0 0	Pass Pass Pass
0.0971	22	0	0	Pass
0.1009	22	0	0	Pass
0.1048	21	0	0	Pass
0.1086	21	0	0	Pass
0.1124	21	0	0	Pass
0.1162	20	0	0	Pass
0.1200	20	0	0	Pass
0.1238	20	0	0	Pass
0.1276	20	0	0	Pass
0.1314	19	0	0	Pass
0.1352	19	0	0	Pass
0.1390	19	0	0	Pass
0.1428	18	0	0	Pass
0.1466	17	0	0	Pass
0.1504	17	0	0	Pass
0.1542	17	0	0	Pass
0.1580	17	0	0	Pass
0.1618	17	0	0	Pass
0.1657	17	0	0	Pass
0.1695	17	0	0	Pass
0.1733	17	0	0	Pass
0.1771	17	0	0	Pass
0.1809	17	0	0	Pass
0.1847	17	0	0	Pass
0.1885	16	0	0	Pass
0.1923	13	0	0	Pass
0.1961	13	0	0	Pass
0.1999	12	0	0	Pass
0.2037	12	0	0	Pass

$\begin{array}{llllllllllllllllllllllllllllllllllll$	000000000000000000000000000000000000000		Pass Pass Pass Pass Pass Pass Pass Pass
------------------------------------------------------	-----------------------------------------	--	--------------------------------------------------------------

Water QualityWater Quality BMP Flow and Volume for POC #1On-line facility volume:0 acre-feetOn-line facility target flow:0 cfs.Adjusted for 15 min:0 cfs.Off-line facility target flow:0 cfs.Adjusted for 15 min:0 cfs.

LID Report

LID Technique	Used for Treatment ?	Needs	Volume Through Facility (ac-ft)	Infiltration Volume (ac-ft)	Cumulative Volume Infiltration Credit	Percent Volume Infiltrated	Water Quality	Percent Water Quality Treated	Comment
Trapezoidal Pond 1 POC		57.30				100.00			
Gravel Trench Bed 1		645.56				91.12			
Total Volume Infiltrated		702.86	0.00	0.00		91.85	0.00	0%	No Treat. Credit
Compliance with LID Standard 8% of 2-yr to 50% of 2-yr									Duration Analysis Result = Passed

Model Default Modifications

Total of 0 changes have been made.

PERLND Changes

No PERLND changes have been made.

IMPLND Changes

No IMPLND changes have been made.

Appendix Predeveloped Schematic

Basin 5.04ac	1			

Mitigated Schematic



Disclaimer

Legal Notice

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Clear Creek Solutions, Inc. 6200 Capitol Blvd. Ste F Olympia, WA. 98501 Toll Free 1(866)943-0304 Local (360)943-0304

www.clearcreeksolutions.com

	•	
Project Description		
Estation Mathematic	Manning	
Friction Method	Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.010	
Channel Slope	0.005 ft/ft	
Diameter	12.0 in	
Discharge	2.27 cfs	
Results		
Normal Depth	7.4 in	
Flow Area	0.5 ft ²	
Wetted Perimeter	1.8 ft	
Hydraulic Radius	3.4 in	
Top Width	0.97 ft	
Critical Depth	7.7 in	
Percent Full	61.3 %	
Critical Slope	0.004 ft/ft	
Velocity	4.50 ft/s	
Velocity Head	0.31 ft	
Specific Energy	0.93 ft	
Froude Number	1.103	
Maximum Discharge	3.52 cfs	
Discharge Full	3.27 cfs	
Slope Full	0.002 ft/ft	
Flow Type	Supercritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description	N/A	
Profile Headloss	0.00 ft	
Average End Depth Over Rise	0.0 %	
Normal Depth Over Rise	61.3 %	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	7.4 in	
Critical Depth	7.7 in	
Channel Slope	0.005 ft/ft	
Critical Slope	0.004 ft/ft	

RAC Parking Lot Design: Conveyance Calculations

Untitled1.fm8 11/8/2022 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 FlowMaster [10.03.00.03] Page 1 of 1

Appendix 2 Soil Management Plan

SOIL MANAGEMENT PLAN

REGIONAL ATHLETIC COMPLEX PARKING LOT DESIGN DECEMBER, 2022

APPLICANT / OWNER:

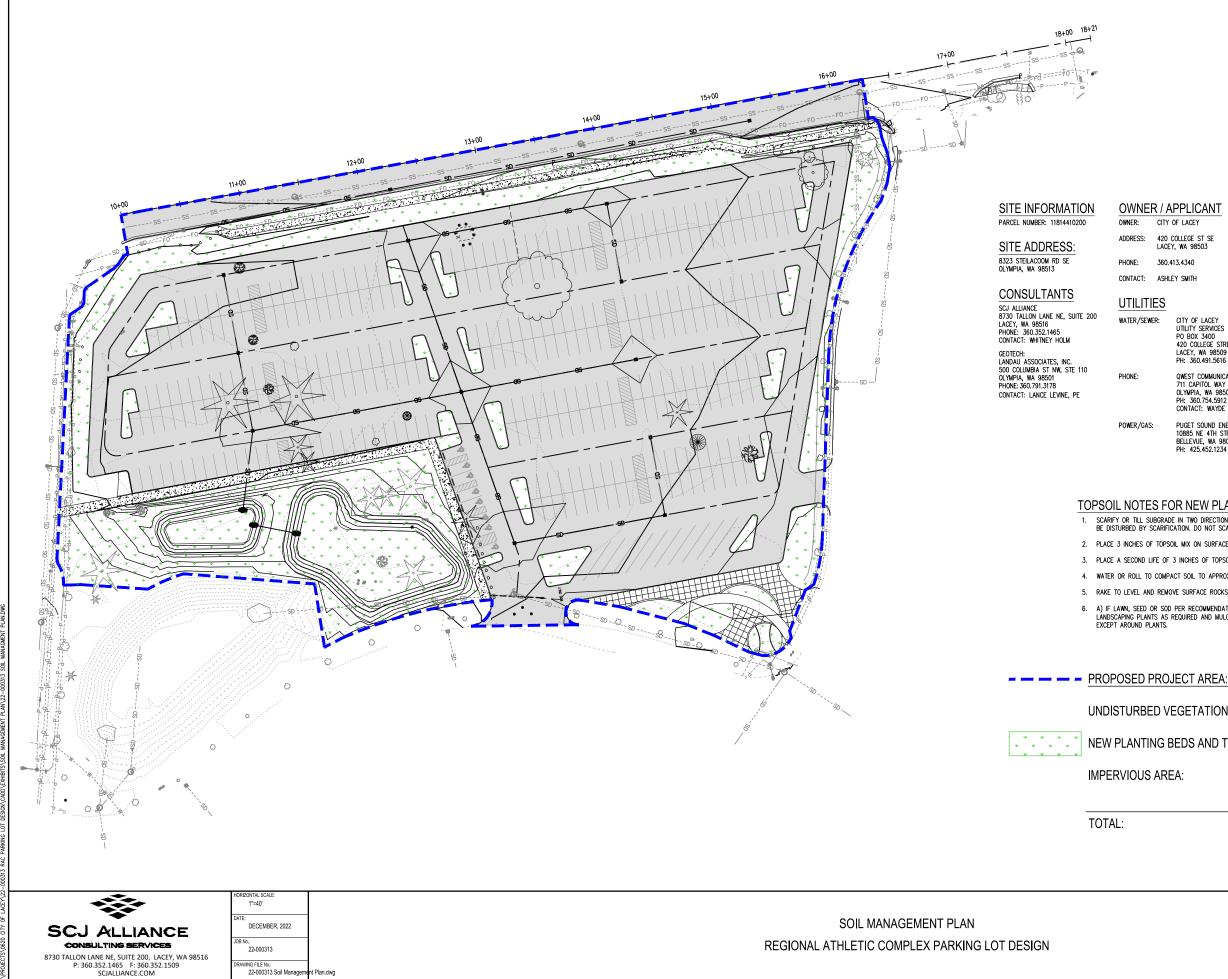
CITY OF LACEY 420 COLLEGE ST SE LACEY, WA 98503 360.413.4340

CONTACT:

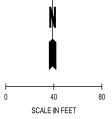
ASHLEY SMITH

PROJECT ENGINEER:

WHITNEY HOLM, P.E. SCJ ALLIANCE 8730 TALLON LANE NE, SUITE 200 LACEY, WA 98516 360.352.1465



2023



OWNER / APPLICANT

CITY OF LACEY 420 COLLEGE ST SE LACEY, WA 98503

360.413.4340

CITY OF LACEY UTILITY SERVICES PO BOX 3400 420 COLLEGE STREET SE LACEY, WA 98509 PH: 360.491.5616

QWEST COMMUNICATIONS 711 CAPITOL WAY S, STE 307 OLYMPIA, WA 98501 PH: 360.754.5912 CONTACT: WAYDE HOLQUIST

PUGET SOUND ENERGY 10885 NE 4TH STREET BELLEVUE, WA 98009 PH: 425.452.1234

LEGAL DESCRIPTION:

SECTION 13 / 14 TOWNSHIP 18 RANGE 1W QUARTER NW SW / NE SE BLA13101203 TR A DOCUMENT 4335765

SECTION 13 / 14 TOWNSHIP 18 RANGE 1W QUARTER NW SW / NE SE BLA13101203LA TR B DOCUMENT 4335765

BASIS OF BEARINGS:

MERIDIAN IS WASHINGTON COORDINATE SYSTEM OF 1983/91 – SOUTH ZONE DERIVED FROM TIES TO HPGN STATIONS SANDERSON, MCKENNA AND CBL1110 AND TO WSDOT GPS STATIONS C259R, CP34005-2, CP34005-4, CP34101-32, CP34101-39, HC34-2, LUNR RW2, TS34-33, TS34-59 AND TO THURSTON COUNTY GPS STATIONS U-531, AT-194, AT-352, AT-355, AT-447, AT449 AND CUDITI GPS STATIONS U=231, AT=194, AT=32, AT=325, AT=355, AT=447, AT449 ANU AT=478. DISTANCES SHOWN ARE GROUND SCALE U.S. SURVEY FEET. COMBINED SCALE FACTOR (GROUND TO GRID) IS 0.999935701. SURVEY AF# 3111152 DATED 09-24-1997.

DATUM:

THE CITY OF LACEY BENCHMARKS ARE BASED ON NGVD 29 DATUM FROM PRIMARY CONTROL ESTABLISHED BY FEDERAL AND STATE AGENCIES TO FIRST OR SECOND ORDER ACCURACY. THE CITY OF LACEY'S BENCHMARKS ARE GENERALLY TO THIRD ORDER ACCURACY.

TOPSOIL NOTES FOR NEW PLANTING BEDS AND TURF AREAS:

1. SCARIFY OR TILL SUBGRADE IN TWO DIRECTIONS TO A DEPTH OF 6-INCHES. THE ENTIRE SURFACE SHOULD BE DISTURBED BY SCARIFICATION. DO NOT SCARIFY WITHIN DRIP LINE OF EXISTING TREES TO BE RETAINED.

2. PLACE 3 INCHES OF TOPSOIL MIX ON SURFACE AND TILL INTO 2 INCHES OF SOIL.

3. PLACE A SECOND LIFE OF 3 INCHES OF TOPSOIL MIX ON SURFACE.

4. WATER OR ROLL TO COMPACT SOIL TO APPROXIMATELY 85% OF MAXIMUM DRY DENSITY.

5. RAKE TO LEVEL AND REMOVE SURFACE ROCKS OR DEBRIS GREATER THAN 1 INCH IN DIAMETER.

6. A) IF LAWN, SEED OR SOD PER RECOMMENDATIONS OF SEED/SOD PROVIDER. B) IF PLANTING BED, PLANT LANDSCAPING PLANTS AS REQUIRED AND MULCH PLANTING BED WITH 2 INCHES OF ORGANIC MULCH EXCEPT AROUND PLANTS.

VEGETATION:	N/A
G BEDS AND TURF AREAS :	1.66 ACRES (72310 SF)
AREA:	3.92 ACRES (170755 SF)

5.58 ACRES (243065 SF)

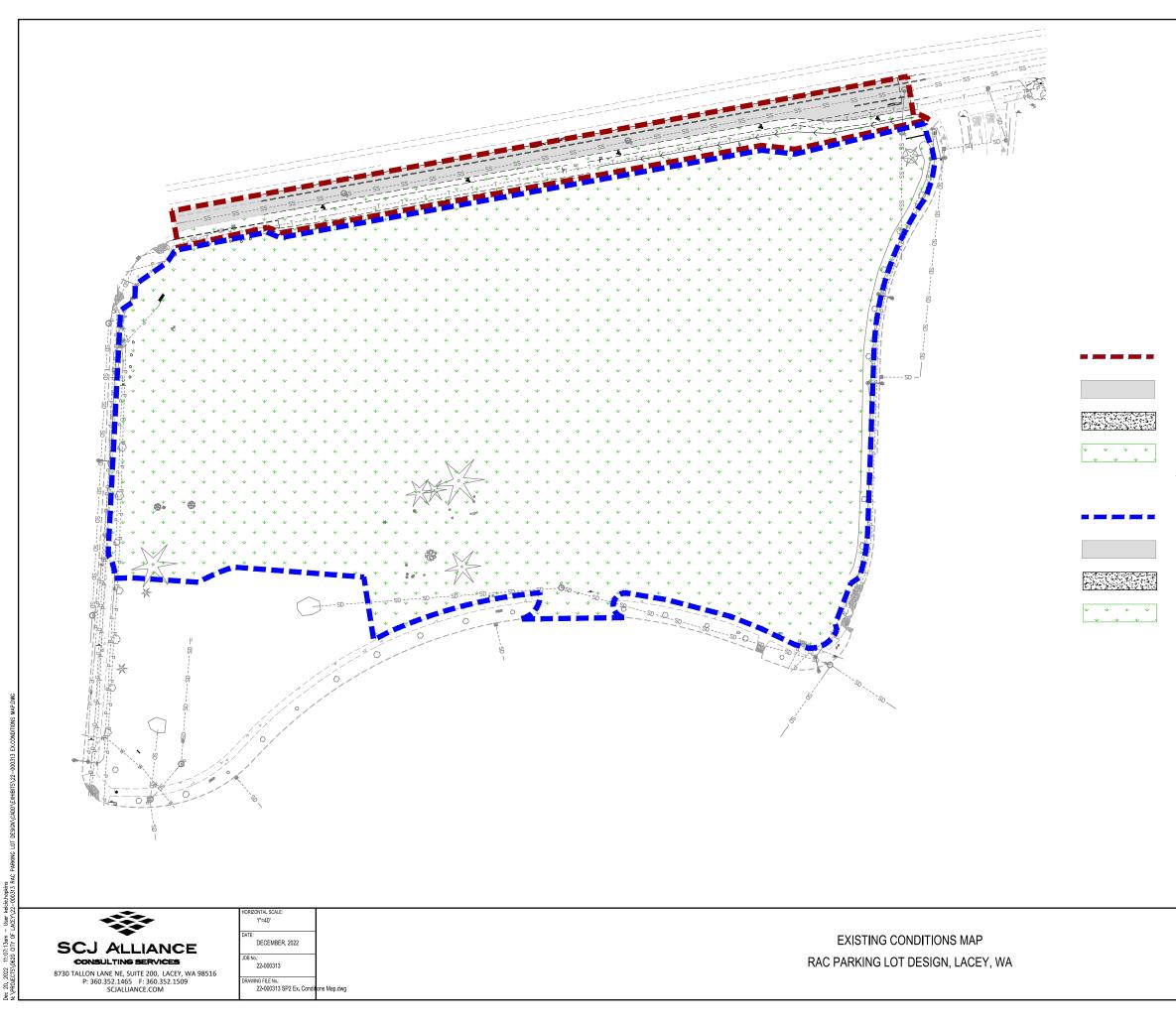
HIBIT No

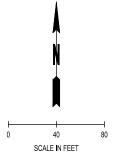
Soil Management Plan for BMP LID.02

PROJECT INFORMATION Complete all information on page 1; only site address and permit number on additional pages.					nal nages	Page #	of	_ pages
Site Address /]	Lot No.: 032	3 STEILAU	COOM RD SE, OLYM	PIA, WA 96	013			
D			D					
Permit Type: Permit Number:								
Permit Holder: CITY OF LACEY Phone: 360.413.4340								
Mailing Address: 420 COLLEGE ST SE, LACEY, WA 98503								
Contact Person: ASHLEY SMITH Phone: 360.413.4340								
	By: SCJ ALLIA							
ATTACHMEN	NTS REQUIRED (Check off req	uired items that are attacl	hed to this plar	a)			
X Site Plan showing, to scale: X Areas of undisturbed native vegetation (no amendment required)								
	\underline{X} New planting beds and turf areas (amendment required)							
		<u>_</u> Х_Туро	e of soil improvement prop	posed for each	area			
			stom amendment rates)					
Product te	st results for prop	osed amendr	nents					
AREA #	(should match	Area # on Sit	e Plan)					
PLANTING T			Undisturbed nativ	ve vegetation				
		ing Beds	Other:	e vegetation				
SOUARE FOO	DTAGE OF THIS	AREA: 72.3	310 square feet					
SCARIFICAT			(depth) of scarification nee	ded to achieve	finished total 1	2" loosened	depth.	
	l be scarified	<u> </u>						
PRE-APPROV		3 inches	of compost or imported to	psoil applied				
AMENDMEN'	Г METHOD:		<i>iversion factor, inches to cubic yards</i>) PRODUCT		PRODUCT:	FOP SOIL		
			9.3 = cu. yards per 1,000 sq. ft.					
			72,310 sq.ft. in this area					
			cubic yards of amendment $\rightarrow \rightarrow \rightarrow \rightarrow \rightarrow$ QUANTITY: <u>675</u> CU		675 CU	. YDS.		
			(needed to cover this area to designated depth)					
CUSTOM AMENDMENT Attach to		Attach test	t results and calculations.					
Topsoil import		inches organic matter or topsoil import		PRODUCT:				
Topsoil & compost lift X 3.		X <u>3.1</u>						
Amend		= cu. yards / 1,000 sq. ft.						
Stockpile and amend X		Х,000	,000s sq.ft. in this area					
					QUANTITY:		. YDS.	
MULCH		,000 sq			PRODUCT:			
			version, to give 2 inch mu					
			bic yards of mulch $\rightarrow \rightarrow$		QUANTITY:		. YDS.	
TOTAL AMENDMENT/TOPSOIL/MULCH FOR ALL AREAS (complete on page 1 only, totaling all areas/pages in this Plan)								
					🛛 Quantity:		yds.	
			matterC:N ratio	<25:1 (except	mulch, or <35:1	for native p	lants) _	
	"stable" (yes/no)						
□ Product #2: □ Quantity:cu. yds.								
			matterC:N ratio	<25:1 (except	mulch, or <35:1	for native p	lants) _	
	"stable" (yes/no)						
Product #3: Quantity:cu. yds.								
Test Results: % organic matter C:N ratio <25:1 (except mulch, or <35:1 for native plants)								
	"stable" (yes/no)						
Date:	Inspector:		Approved:	Revisions Required:				
Dan.	mprecor.		Approven.	AUTOID IN	quireu.			
Date:	Inspector:		Approved:	Revisions Re	couired:			
And								

Appendix 3

Supplemental Reports and Information

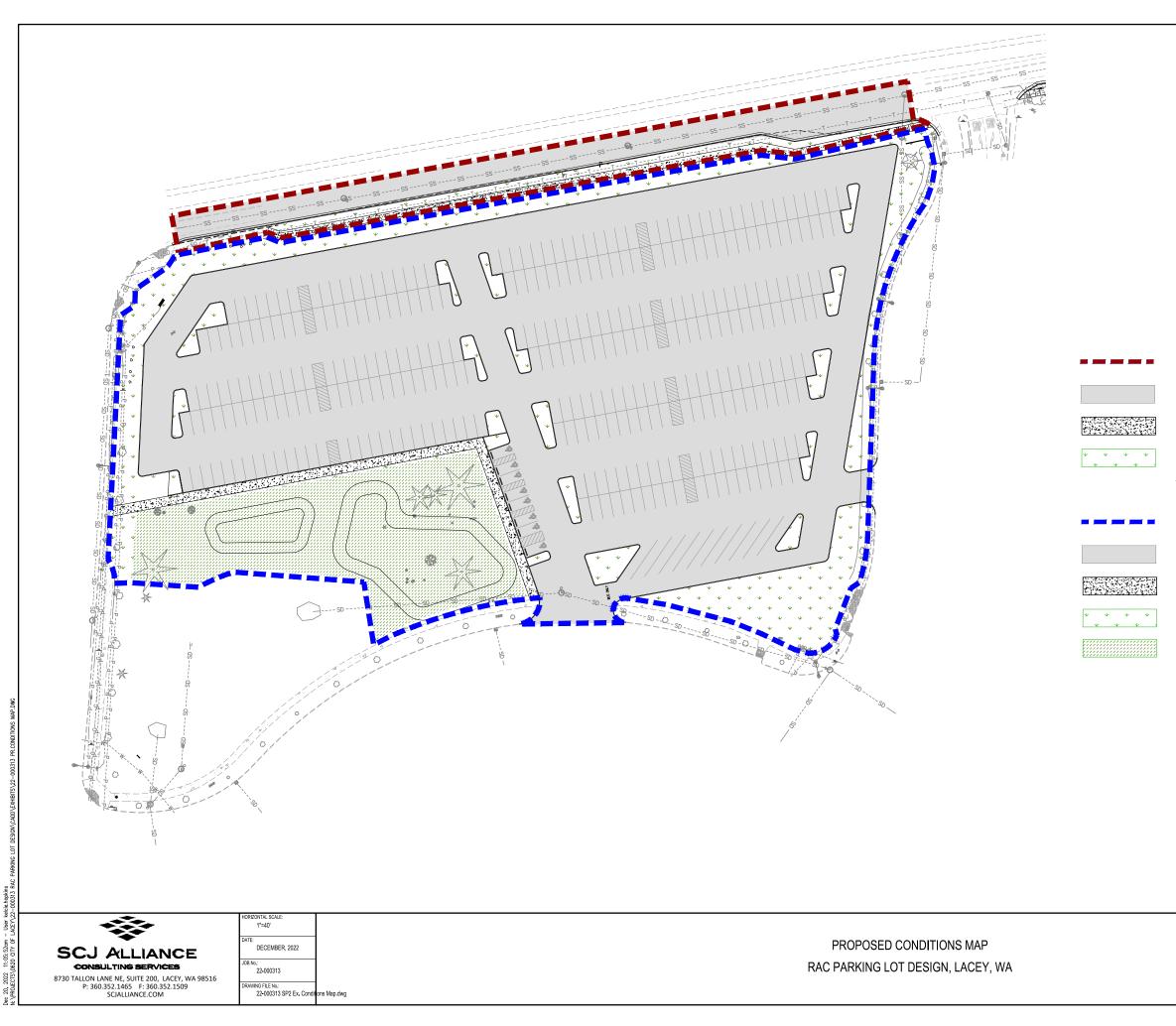


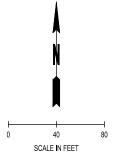


PROPOSED BASIN AREAS:

BASIN 1:	
ASPHALT PAVEMENT AREA:	0.26 ACRES
SIDEWALK AREA:	0.00 ACRES
PERVIOUS AREA:	0.28 ACRES
TOTAL:	0.54 ACRES
BASIN 2:	
<u>BASIN 2:</u> ASPHALT PAVEMENT AREA:	0.00 ACRES
	0.00 ACRES 0.00 ACRES
ASPHALT PAVEMENT AREA:	

EXHIBIT NO:





PROPOSED BASIN AREAS:

BASIN 1:	
ASPHALT PAVEMENT AREA:	0.35 ACRES
SIDEWALK AREA:	0.08 ACRES
PERVIOUS AREA:	0.11 ACRES
TOTAL:	0.54 ACRES
BASIN 2:	
ASPHALT PAVEMENT AREA:	3.42 ACRES
SIDEWALK AREA:	0.07 ACRES
PERVIOUS AREA:	0.88 ACRES
RETAINED PERVIOUS POND AREA:	0.67 ACRES
TOTAL:	5.04 ACRES

EXHIBIT No:

EX-02

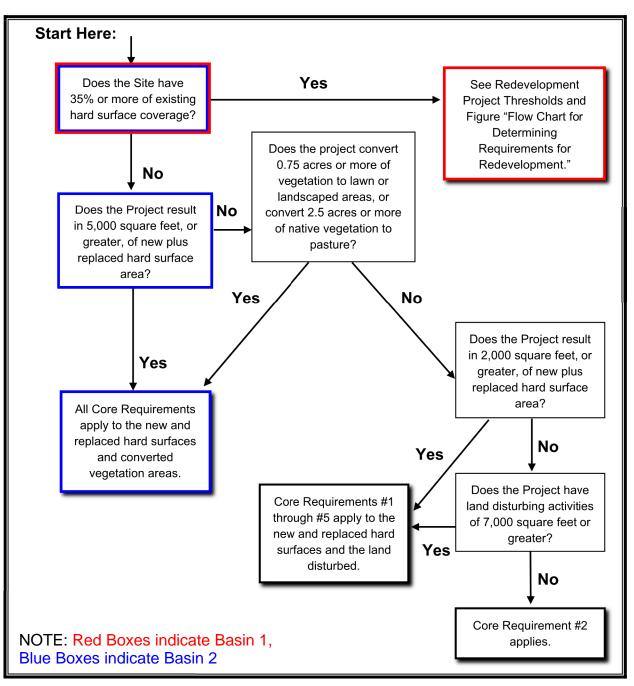


Figure 2.1. Flow Chart for Determining Requirements for New Development.

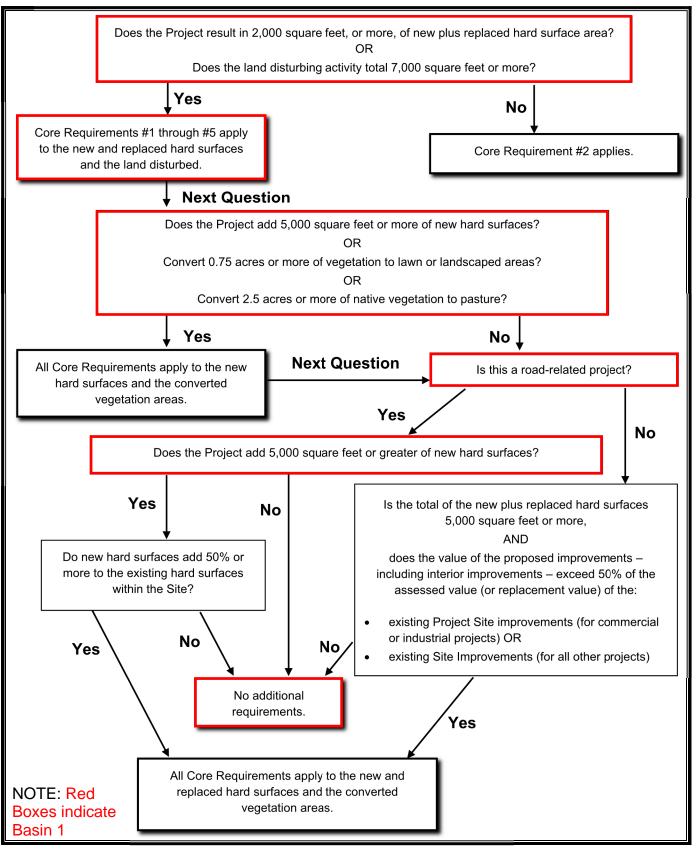
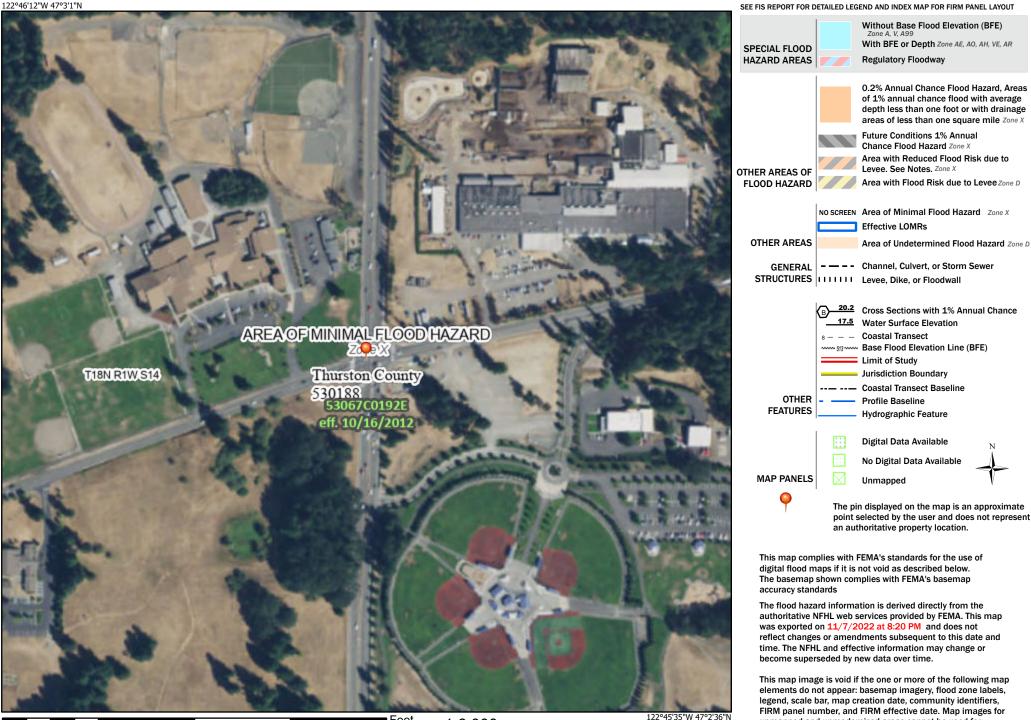


Figure 2.2. Flow Chart for Determining Requirements for Redevelopment.

National Flood Hazard Layer FIRMette



Legend



250 500

n

1,500

1,000

Feet 1:6.000 2.000

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

unmapped and unmodernized areas cannot be used for

regulatory purposes.

