

Drainage Control Plan Report for MOREL MEADOWS

8322 Steilacoom Road SE, Olympia WA, 98513

Project Applicant

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06.13.2023

Project Engineer's Certification

"I hereby state that this Drainage Control Plan Report for Morel Meadows has been prepared by me or under my supervision and meets the standard of care and expertise which is usual and customary in this community for professional engineers. I understand that the City of Lacey does not and will not assume liability for the sufficiency, suitability, or performance of drainage BMPs prepared by me."

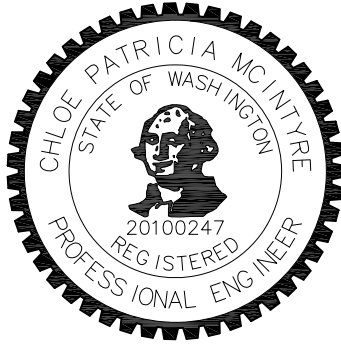


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Section 1. Project Overview

1.1 Site Information

Parcel number(s)	11814140500
Address or legal description of site property	8322 Steilacoom Road SE, Olympia WA, 98513
Current zoning	Low Density Residential 3-6
Streets/general vicinity	Steilacoom Road SE, Marvin Road SE
Property owner(s)	Ostrom Mushroom Co Inc.
Total project site area	32.08 ac
Surrounding land uses	Single-family homes, school, city-owned athletic complex

1.2 Project Description

The Morel Meadows project is located northeast of the intersection of Marvin Road and Steilacoom Road SE in the City of Lacey in Section 14, Township 18, Range 1 West, W.M. on tax parcel number 11814140500. The project proposes to construct 179 single-family homes and associated driveways, 5.76 acres of roadway and 1.52 acres of associated sidewalk, with three stormwater ponds and four bioretention cells, and required landscaping, sewer, and water service improvements. See proposed Site Plan in Appendix 1.

The proposed project will require grading, encroachment, building, and utility permits. Water and sewer will be provided via connections to the City of Lacey's utilities. Zoning for the property is Low Density Residential 3-6.

1.3 Proposed Stormwater Drainage Design

The site will be split into five basins for stormwater mitigation. The northwest basin will include 12.22 total acres with 63% impervious, the southwest basin is 3.82 acres with 52% impervious, and the east basin is 16.04 acres with 46% impervious. Each of these basins includes houses, roads, sidewalk, and landscaping. Runoff for each of these three basins will be routed first to one of three bioretention cells for treatment and then one of three retention ponds to infiltrate the remainder of the runoff. The Southeast Frontage basin consists of 0.88 acres (57% impervious) of frontage along Steilacoom Road SE and a bit of the southeast portion of the site. Runoff from this basin will be routed to a bioretention cell for treatment and full infiltration. The north bypass basin will consist of 1.39 acres of mostly forest with some landscaped areas, and runoff will continue to sheet flow to the north as in the predeveloped condition. See Area Summary table for basins in Appendix 3.

Table 1.1. Proposed Permanent Stormwater BMPs.		
Permanent Stormwater BMP	Location	Proposed Ownership
Bioretention Cell 1	Northwest Basin	HOA
Retention Pond 1	Northwest Basin	HOA
Bioretention Cell 2	Southwest Basin	HOA
Retention Pond 2	Southwest Basin	HOA
Bioretention Cell 3	East Basin	HOA
Retention Pond 3	East Basin	HOA
Bioretention Cell 4	Southeast Frontage Basin	HOA

1.4 Subarea Data Tabulation

The project is split into five basins in the developed condition. The northwest basin includes the northwest portion of the site and Marvin Road frontage and goes to Bioretention Cell 1 for treatment and Retention Pond 1 for infiltration. The southwest basin includes the southwest portion of the site and goes to Bioretention Cell 2 for treatment and Retention Pond 2 for infiltration. The east basin includes the east of the site, and goes to Bioretention Cell 3 for treatment and Retention Pond 3 for infiltration. The southeast frontage basin includes Steilacoom Road frontage and goes to Bioretention Cell 4 for treatment and infiltration. The north bypass basin includes undisturbed areas in the north of the site and will continue to sheet flow as in the existing condition. See Basin Map in Appendix 1 and basin area summary in Appendix 3.

Tables 1.2 and 1.3 summarize the existing and proposed areas for this project, respectively.

Table 1.2. Existing Site Land Coverage Tabulation.			
Existing Surface	Surface Type	Area (square feet)	Area (acres)
Asphalt	Hard/Impervious	135,213	3.10
Concrete	Hard/Impervious	268,335	6.16
Roof	Hard/Impervious	219,867	5.05
Forested/Trees	Pervious	386,995	8.88
Pasture/Landscaping	Pervious	386,995	8.88
Total Site/Parcel Area		1,397,405	32.08

Table 1.3. Proposed Site Land Use Coverage Tabulation.				
Proposed Surface	Surface Type	Pollutant Generating	Area (square feet)	Area (acres)
Roadway	Hard/Impervious	Yes - PGIS	258,746	5.94
Driveway	Hard/Impervious	Yes - PGIS	89,298	2.05
Walkway	Hard/Impervious	No	66,211	1.52
Roof	Hard/Impervious	No	295,772	6.79
Pond	Hard/Pervious	Yes - PGPS	40,075	0.92
Landscaping	Pervious	Yes - PGPS	548,856	12.60
Undisturbed (e.g., tree tract)	Pervious	No	181,645	4.17
Bioretention:	Pervious	No	15,246	0.35
Total Project Area*			1,495,849	34.36

PGIS = Pollution Generating Impervious Surface

PGPS = Pollution Generating Pervious Surface

*Total Project Area includes offsite frontage tributary to proposed stormwater facilities.

Section 2. Development Conditions and Requirements

2.1 Project Vesting

<input checked="" type="checkbox"/>	2022 SDM
<input type="checkbox"/>	SDM from another year
<input checked="" type="checkbox"/>	2017 City of Lacey Development Guidelines and Public Works Standards (DG&PWS)
<input type="checkbox"/>	DG&PWS from another year

2.2 Permits Required

Table 2.1. Permits Required.			
Permit Title	Agency Requiring Permit	Permit Requirements that Impact the Project	Present Status
Right-of-Way Access Permit	City of Lacey	Frontage improvement areas to be factored into site storm design	Not Applied

2.3 Project Type and Size

Table 2.2 lists the new, replaced, and disturbed areas for the project.

Table 2.2. New, Replaced, and Disturbed Areas.	
Area Type	Area
Total New Hard Surfaces	1.18
Total Replaced Hard Surfaces	15.12
Total Area Disturbed	27.91

A Drainage Control Plan is required for projects that must address all Core Requirements. **Select “yes” or “no” for each statement below.** If the answer is “yes” for any statement, a Drainage Control Plan is required for the project.

Yes	No	Statement
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Does the project convert 0.75 acres or more of vegetation to lawn or landscaped areas to pasture?
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Does the project convert 2.5 acres or more of native vegetation to pasture?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Does the Project result in 5,000 square feet or more of new plus replaced hard surface area?

New Development:

Include markup of flow chart from Figure 2.1

Redevelopment:

Include markup of flow chart from Figure 2.2

If this is a redevelopment project, include valuation of proposed improvements, including interior improvements:

179 single-family homes with associated roads, sidewalks, and utilities. Four storm ponds.

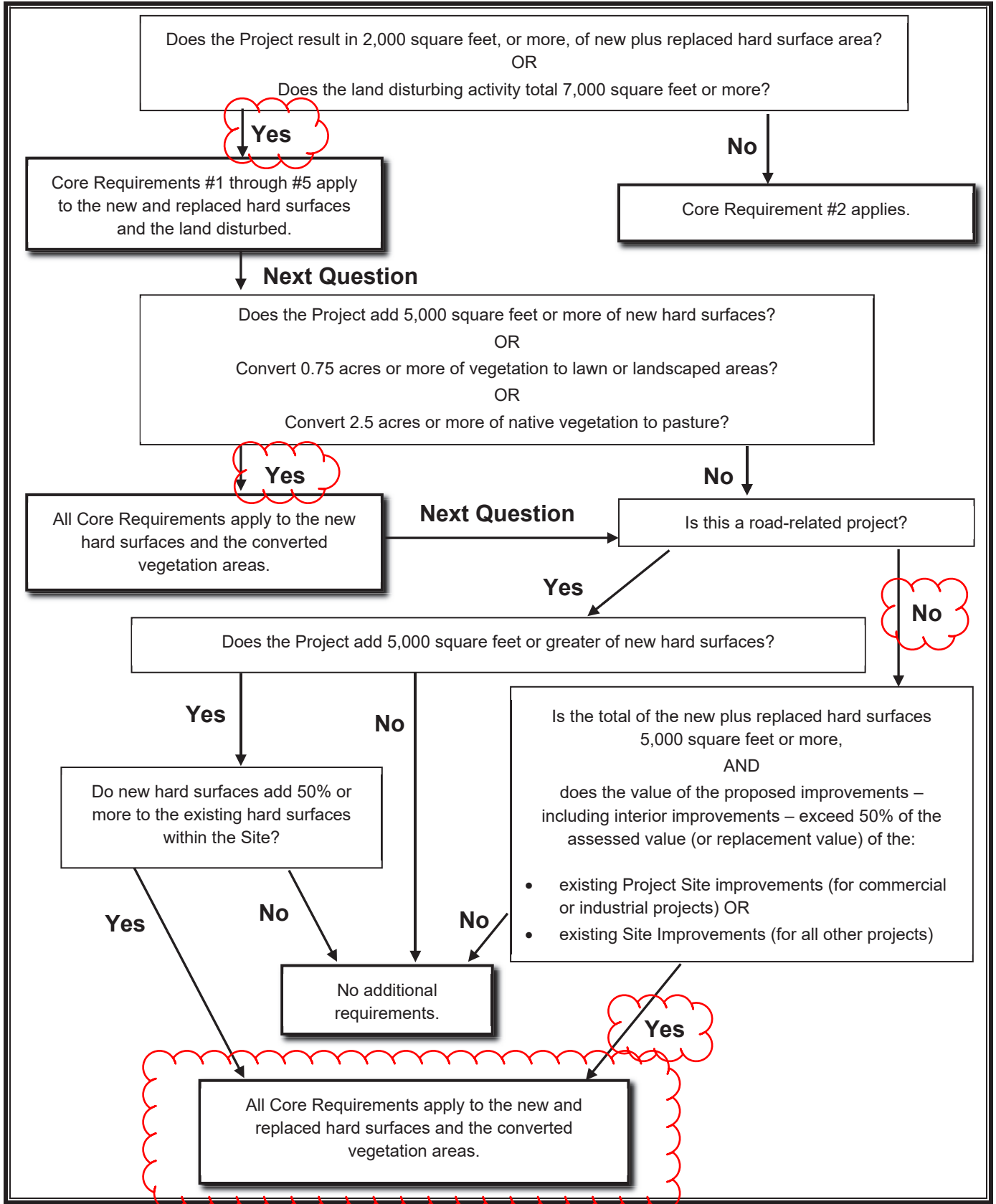


Figure 2.2. Flow Chart for Determining Requirements for Redevelopment.

2.4 Critical Areas

Does this project involve work in or near critical areas? Yes No

Describe the presence of any critical areas or environmentally sensitive areas, including the following:

<input type="checkbox"/>	Wetlands	<input type="checkbox"/>	Endangered Species Act (ESA) species habitat
<input checked="" type="checkbox"/>	Wellhead Protection Areas (WHPAs)	<input checked="" type="checkbox"/>	Critical Aquifer Recharge Areas (CARAs)
<input type="checkbox"/>	Geologically hazardous areas	<input type="checkbox"/>	Steep slopes
<input type="checkbox"/>	Other: _____	<input type="checkbox"/>	Other: _____

2.5 Core Requirements

Core Requirement #1: Preparation of Stormwater Site Plans

The main components of Stormwater Site Planning are Construction Stormwater Pollution Prevention Planning and Permanent Stormwater Control Planning. This Drainage Report, a Construction Stormwater Pollution Prevention Plan, Soils Report, Maintenance and Source Control Manual, and copy of the proposed Maintenance Covenant for stormwater facilities are submitted as part of the Morel Meadows Drainage Control Plan to meet this requirement.

Core Requirement #2: Construction Stormwater Pollution Prevention

A Construction Stormwater Pollution Prevention Plan (C-SWPPP) has been developed to address erosion and sediment control anticipated during construction. A Construction NPDES permit will be obtained prior to construction. The C-SWPPP will address all thirteen elements as required by the Department of Ecology.

Core Requirement #3: Source Control of Pollution

Source control BMPs are used to prevent stormwater from coming in contact with pollutants and are used as a cost-effective means of reducing pollutants in stormwater. The selection of permanent source control BMPs is based on the activities likely to occur on the site and the pollutants associated with those activities.

The Stormwater Pollution Source Control Checklist and Worksheet found in Appendix 9A of Chapter 9 of 2022 SDM has been completed to determine the applicable post-construction activities with required Source Control BMPs on this site. Methods to address source control of pollution from these activities are provided in the Maintenance and Source Control Manual submitted as part of the Drainage Control Plan for this project. Construction source control BMPs are addressed in the C-SWPPP to be included in a later submittal. A copy of the Stormwater Pollution Source Control Checklist and Worksheet can be found in Appendix C of the Maintenance and Source Control Manual included in a later submittal.

Core Requirement #4: Preservation of Drainage Systems and Outfalls

Low-impact development techniques will be used to preserve existing site runoff patterns to the maximum extent feasible. In the predeveloped condition, stormwater runoff from the site generally sheet flows from south to north of the site towards adjacent properties. Soil in the disturbed lawn/landscape areas will be amended per BMP T5.13 to increase treatment and infiltration capacity and to reduce runoff from the site. Runoff in the developed condition will be fully infiltrated in onsite infiltration ponds, per WWHM Site Analysis model attached in Appendix 3. Runoff from the northwest basin will go to Retention Pond 1, runoff from the southwest basin to Retention Pond 2, runoff from the east basin to Retention Pond 3, and runoff from the southeast frontage basin to Bioretention Cell 4. Stormwater runoff from the preserved native areas of the project will continue to sheet flow onto adjacent properties, matching existing conditions.

Core Requirement #5: On-Site Stormwater Management

Which of the following options was implemented for CR #5?

<input type="checkbox"/>	List #1
<input type="checkbox"/>	List #2
<input type="checkbox"/>	List #3
<input checked="" type="checkbox"/>	LID performance standard

This project proposes to implement Postconstruction Soil Quality and Depth (Ecology BMP T5.13) in all new and disturbed lawn/landscape areas to retain greater stormwater functions, including increased infiltration potential and treatment of pollutants and sediments resulting from development. This project also proposes the use of bioretention cells (BMP T7.30) and infiltration basins (BMP T7.10) to treat and infiltrate 100% of tributary stormwater runoff from the proposed improvements. The combination of stormwater BMPs used for this project results in the site meeting the Low-Impact Development Performance Standard as illustrated on Page 44 of the 21-100 Site Analysis WWHM report. See attached report in Appendix 3. By implementing these BMPs, this project fully satisfies the criteria of Core Requirement #5.

Core Requirement #6: Runoff Treatment

Table 2.3. Thresholds for Core Requirement #6: Runoff Treatment	
	Required to Comply
< 5,000 sf of total effective pollution-generating hard surface (PGHS)	
≥ 5,000 sf of total effective pollution-generating hard surface (PGHS)	X
< ¾ acres of pollution-generating pervious surface (PGPS) from which there will be a surface discharge in a natural or artificial conveyance system from the site	
≥ ¾ acres of pollution-generating pervious surface (PGPS) from which there will be a surface discharge in a natural or artificial conveyance system from the site	X

Table 2.3 above summarizes the thresholds for construction of stormwater treatment facilities. This project will add 350,000 sf of PGHS; therefore, treatment is required.

This project proposes to provide enhanced treatment by infiltrating 100% of stormwater from pollution-generating surfaces through one of four bioretention cells. Bioretention cells have been sized to filter at least 91% of the water quality volume in WWHM and are used for the northwest, southwest, southeast frontage, and east basins. See further explanation of water quality facility sizing in Section 6 of this Drainage Report.

Core Requirement #7: Flow Control

Table 2.4. Thresholds for Core Requirement #7: Flow Control	
	Required to Comply
< ¾ acres of native vegetation converted to lawn/landscape or < 2.5 acres converted to pasture from which there is a surface discharge in a natural or artificial conveyance system from the site	
≥ ¾ acres of native vegetation converted to lawn/landscape or ≥ 2.5 acres converted to pasture from which there is a surface discharge in a natural or artificial conveyance system from the site	X
< 10,000 sf of effective impervious area	
≥ 10,000 sf of effective impervious area	X
≥ 0.10 cfs increase in the 100-year storm flow frequency using 1-hour time steps or ≥ 0.15 cfs increase in the 100-year storm flow frequency using 15-minute time steps	X

Table 2.4 above summarizes the thresholds for achievement of the standard flow control requirement for Western Washington. This project will add 775,000 sf of effective impervious surface. Flow control is required. This project proposes to provide flow control through the use of three infiltration ponds and four bioretention cells sized to

infiltrate 100% of tributary runoff. See further explanation of flow control facility sizing in Section 6 of this Drainage Report.

Core Requirement #8: Wetlands Protection

This project does not propose to discharge stormwater to a wetland.

Core Requirement #9: Operation and Maintenance

Proper operation and maintenance of proposed stormwater facilities is a vital component to the success of stormwater mitigation. A Maintenance and Source Control Manual and Operation and Maintenance Agreement will be included with the final design for the Morel Meadows project.

Section 3. Site and Vicinity Description

3.1 Existing Physiography

The existing site has a general slope from an elevation of around 230 in the south to 212 in the north. It includes oak trees near the edge of the property in the north and east.

3.2 Existing Improvements

The existing site is developed with Ostrom Mushroom Farm, consisting of around 5 acres of roof, 9 acres of concrete and asphalt, and several storm ponds and water storage tanks as well as a well in the southeast.

3.3 Drainage Patterns

In the existing condition, drainage generally flows south to north across the site. There are several ponds used for stormwater storage. Existing soil is Type A/B with a high hydraulic conductivity.

3.4 Qualitative Analysis

The project proposes to use infiltration BMPs and post-construction soil enhancements to reduce overall site runoff from the pre-developed condition. A majority of onsite runoff will be conveyed via catch basins and piping to bioretention cells and infiltration ponds.

Conveyance capacity is not anticipated to be a problem. Onsite conveyance systems are to be designed to convey the 25-year 24-hour flow in accordance with the SDM. Flows tributary to downstream conveyances will be reduced by development of this site. See Section 7 for analysis. Erosion impacts are not anticipated. No stream channel or surface flow impacts are anticipated as the runoff is being fully infiltrated.

Overflow structures were sized for the northwest and east ponds to convey the 100-year inflow to the pond. Due to topography, the southwest infiltration pond and southeast bioretention pond cannot have overflow structures. Instead, excess runoff would back up

in the conveyance systems directed towards these ponds and eventually rise through catch basins to join the conveyance systems of the northwest and east basins.

Stormwater will be treated in bioretention cells prior to infiltration, ensuring no negative impact to subgrade aquifers or downstream conveyances.

The project will reduce overall runoff from the site, improving any existing capacity or flooding issues that may exist. No adverse downstream impacts are expected due to the development of the site.

3.5 Quantitative Analysis

N/A

Section 4. Soils and Infiltration Analysis

4.1 Summary of Soils and Geotechnical Data

The soil is mapped as Vashon recessional outwash (Qgo.) Soil conditions are primarily gravelly sandy loam with high hydraulic conductivities. See Table 4.1 for soil types by area, and Appendix 1 for NRCS soils map.

The Riley Group conducted a geotechnical investigation, with a report dated June 28, 2022. They encountered up to 12 feet of loose to medium dense fill made up of gravel with some sand and trace silt and silty sandy gravel over medium dense native deposits of silty sandy gravel, sandy gravel with trace silt, and gravel with trace to some sand and silt. Under the gravel deposits was dense silty sand with gravel. See Riley Group geotechnical report in Appendix 2.

Table 4.1. Soil Characteristics.			
Soil description		Area (acres)	Percent of Site
Geologic/glacial (e.g., Vashon till or recessional outwash)	NRCS Soil Units (e.g., Indianola series, Hydrologic Soil Group A)		
Sandy and Gravelly Glacial Outwash	Everett very gravelly sandy loam, 8 to 15 % slopes, Hydrologic Soil Group A	0.2	0.8
Glacial Outwash	Spana gravelly loam, Hydrologic Soil Group B	6.9	21.6
Volcanic ash over gravelly outwash	Spanaway gravelly sandy loam, 0 to 3% slopes, Hydrologic Soil Group A	24.9	77.7

4.2 Subsurface Factors

Light groundwater seepage was found by the Riley Group at depths of 13.5 and 23.5 feet at two explorations in the northwest of the site. Infiltration was deemed feasible for the site.

4.3 Infiltration Rates

A Pilot Infiltration Test was done in TP-1 with a measured rate of 140 inches per hour. After a total correction factor was applied, the Riley Group estimated a design rate of 21 inches per hour. To be conservative, a 10 inch per hour rate was used in design for the project.

Section 5. On-Site Stormwater Management and Low Impact Development

(Core Requirement #5)

5.1 LID Site Design

Oak trees near the north edge of the site are to be preserved to the maximum extent feasible (BMP T5.40). The subdivision will be laid out to maximize lots per foot of roadway and limit impervious coverage, per Better Site Design (BMP T5.41).

5.2 Methodology

The project will meet the LID Performance Standard per WWHM modeling. This will be achieved through 100% infiltration using three infiltration ponds (T7.10) and four bioretention cells (T7.30). Roughly 13% of the site will remain in the native condition and runoff in this portion will match existing drainage patterns.

5.3 LID Practices

LID BMPs to be used are:

- Postconstruction Soil Quality and Depth (T5.13)
- Bioretention Cells (T7.30)
- Infiltration Basins (T7.10)

5.4 Post-Construction Soil Quality and Depth

Which of the implementation options will be used? Select all that apply.

<input checked="" type="checkbox"/>	Retain and protect undisturbed soil
<input checked="" type="checkbox"/>	Amend soil
<input checked="" type="checkbox"/>	Stockpile soil
<input checked="" type="checkbox"/>	Import soil

Soil in undisturbed forested areas will be retained. Soil in disturbed open space and lawn areas will be amended to meet the BMP requirements. Approximately 12.4 acres of soil will be amended. Soil will be stockpiled during grading and imported soil will meet BMP requirements.

5.5 Retained Trees and Aesthetics

N/A

Section 6. Runoff Treatment and Flow Control (Core Requirements #6 & #7)

6.1 Runoff Treatment Selection

Step 1: The receiving waters for this project will be groundwater, which is in a category 1 and category 2 critical aquifer recharge area as well as the 5-year time-of-travel area for a Lacey System A well.

Step 2: It was determined that oil control is not required due to the site not being a commercial/industrial or high-traffic site.

Step 3: Infiltration into the native soil for treatment is not practicable due to high infiltration rates in the soil.

Step 4: It was determined that phosphorus treatment is not required because the site does not discharge to freshwater bodies.

Step 5: Enhanced treatment is required because the site infiltrates in a Category 1 critical aquifer recharge area.

Bioretention is proposed for treatment for this project.

Which of the following runoff treatment performance goals are required for the project site?

<input type="checkbox"/>	Oil control (SDM Chapter 8, Section 8.3.2)
<input type="checkbox"/>	Phosphorus Treatment (SDM Chapter 8, Section 8.3.3)
<input checked="" type="checkbox"/>	Enhanced Treatment (SDM Chapter 8, Section 8.3.4)
<input type="checkbox"/>	Basic Treatment (SDM Chapter 8, Section 8.3.6)
<input type="checkbox"/>	Additional Runoff Treatment (SDM Chapter 8, Section 8.3.5): <ul style="list-style-type: none"> • Bacteria • Dissolved Oxygen • Temperature

Specify the basis for the selected BMPs and provide details below.

<input type="checkbox"/>	Watershed or Basin Plans
<input type="checkbox"/>	Water Clean-up Plans
<input checked="" type="checkbox"/>	Groundwater Management Areas (Wellhead Protection and Critical Aquifer Recharge)
<input type="checkbox"/>	Lake Management Plans

The site is located in a Category 1 Critical Aquifer Recharge Area.

6.2 BMP Types & Descriptions

Bioretention cells (BMP T7.30) are proposed to treat over 91% of the runoff in the northwest, southwest, east, and southeast frontage basins. In the northwest, southwest, and east basins the remaining runoff will be infiltrated in an infiltration basin (BMP T7.10). In the southeast frontage basin, runoff will be fully infiltrated in the bioretention cell.

6.3 Facility Selection and Design Data

Facilities were sized to filter at least 91% of tributary runoff, and, along with the corresponding infiltration pond, to fully infiltrate tributary runoff. See Site Analysis WWHM model in Appendix 3.

6.4 Design Calculations

Bioretention facilities were sized per WWHM to meet treatment requirements. See Site Analysis model in Appendix 3. An infiltration rate of 10 inches per hour was used for the bioretention cells, which was conservatively selected when given an estimated design infiltration rate of 21 inches per hour from the Riley Group. See Appendix 2 for Riley Group calculations.

Stage numbers in Table 6.1 come from WWHM modeling. See calcs in Appendix 3. The storage volume in ac-ft was found using the equation for volume of a trapezoid:

$$V = \frac{LWD + (L + W)ZD^2 + \frac{4}{3}Z^2D^3}{43560}$$

L = bottom length (ft), W = bottom width (ft), Z = side slope (ft/ft), and D = stage (ft)

Table 6.1A Design Stage for Bioretention Cell 1		
Recurrence Interval	Stage (ft)	Storage Volume (ac-ft)
2-year	1.7408	0.165
5-year	1.7411	0.165
10-year	1.7413	0.165
25-year	1.7414	0.165
50-year	1.7415	0.165
100-year	1.7415	0.165
Table 6.1B Design Stage for Bioretention Cell 2		
Recurrence Interval	Stage (ft)	Storage Volume (ac-ft)
2-year	1.4645	0.015
5-year	1.4680	0.015
10-year	1.4695	0.015
25-year	1.4709	0.015
50-year	1.4717	0.015
100-year	1.4723	0.015

Table 6.1C Design Stage for Bioretention Cell 3		
Recurrence Interval	Stage (ft)	Storage Volume (ac-ft)
2-year	1.7357	0.122
5-year	1.7359	0.122
10-year	1.7360	0.122
25-year	1.7361	0.122
50-year	1.7361	0.122
100-year	1.7361	0.122

Table 6.1D Design Stage for Bioretention Cell 4		
Recurrence Interval	Stage (ft)	Storage Volume (ac-ft)
2-year	1.5528	0.037
5-year	1.6068	0.039
10-year	1.6370	0.040
25-year	1.6707	0.041
50-year	1.6935	0.041
100-year	1.7145	0.042

Table 6.2A Elevations for Bioretention Cell 1	
Key Features	Elevation (ft)
Infiltration Surface (Base/Bottom)	212
Lowest Orifice	213.5
BMP Overflow	214.5
BMP Rim (Overflow + Freeboard)	215

Table 6.2B Elevations for Bioretention Cell 2	
Key Features	Elevation (ft)
Infiltration Surface (Base/Bottom)	222
Lowest Orifice	223.5
BMP Overflow	224.5
BMP Rim (Overflow + Freeboard)	225

Table 6.2C Elevations for Bioretention Cell 3	
Key Features	Elevation (ft)
Infiltration Surface (Base/Bottom)	218
Lowest Orifice	219.5
BMP Overflow	220.5
BMP Rim (Overflow + Freeboard)	221

Table 6.2D Elevations for Bioretention Cell 4	
Key Features	Elevation (ft)
Infiltration Surface (Base/Bottom)	215
Lowest Orifice	216.5
BMP Overflow	217.5
BMP Rim (Overflow + Freeboard)	218

Section 7. Runoff Collection and Conveyance System

7.1 System Design and Layout

Runoff will be collected in catch basins and conveyed via pipes in the roadways to the stormwater facility for the corresponding basin.

7.2 Conveyance System Calculations Summary

Onsite conveyance and piping will be sized to convey the 25-year flow in accordance with the 2022 Lacey SDM at final design.

Section 8. Source Control (Core Requirement #3)

8.1 Potential Sources of Pollutants

Potential sources of pollutants include automobile washing and maintenance, storage of solid wastes and food wastes, composting, yard hazardous material use, storage and disposal, pet waste management, and maintenance of stormwater drainage and treatment facilities.

8.2 Source Control BMPs

Source control BMPs will be described in the Maintenance and Source Control Plan as part of final submittal.

8.3 Source Control Checklist and Worksheet

Source control checklist included in Appendix 2.

Section 9. Covenants, Dedications, Easements, Agreements, and Guarantees

9.1 Covenants, Dedications, and Easements

All stormwater facilities located on private property shall be owned, operated and maintained by the property owners, their heirs, successors and assigns. The property owners shall enter into an agreement with the governing body, a copy of which agreement is included in the Maintenance and Source Control Manual of the Drainage Control Plan. The agreement requires maintenance of the stormwater facilities in accordance with the maintenance plan provided and shall grant easement for access to the governing body to inspect the stormwater facilities. The agreement also makes provisions for the governing body to make repairs, after due notice is given to the owners, if repairs are necessary to ensure proper performance of the stormwater system and if the owners fail to make the necessary repairs. The cost of said repairs shall be borne by the property owners, their heirs, successors and assigns.

9.2 Agreements and Guarantees

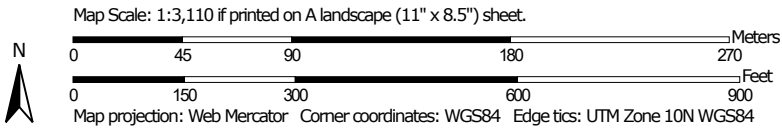
The property owner is required to enter into a Stormwater Maintenance Agreement to maintain stormwater facilities and implement a Pollution Source Control Plan. A copy of the maintenance agreement is included in the Maintenance and Source Control Manual.

The owner is required to provide a Performance Guarantee to the City of Lacey in the amount of 150% of the estimated cost of onsite drainage systems and public utilities (sidewalk, planter strip, sewer, and watermain) construction. A Stormwater Maintenance Guarantee in the amount of 20% of the estimated cost of stormwater improvements for two years is required for maintenance and repair of drainage facilities.

Drainage Control Plan Appendices

Appendix 1: Maps and Plans

Soil Map—Thurston County Area, Washington
(Ostrom Residential)



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Thurston County Area, Washington

Survey Area Data: Version 16, Sep 8, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 31, 2022—Aug 8, 2022

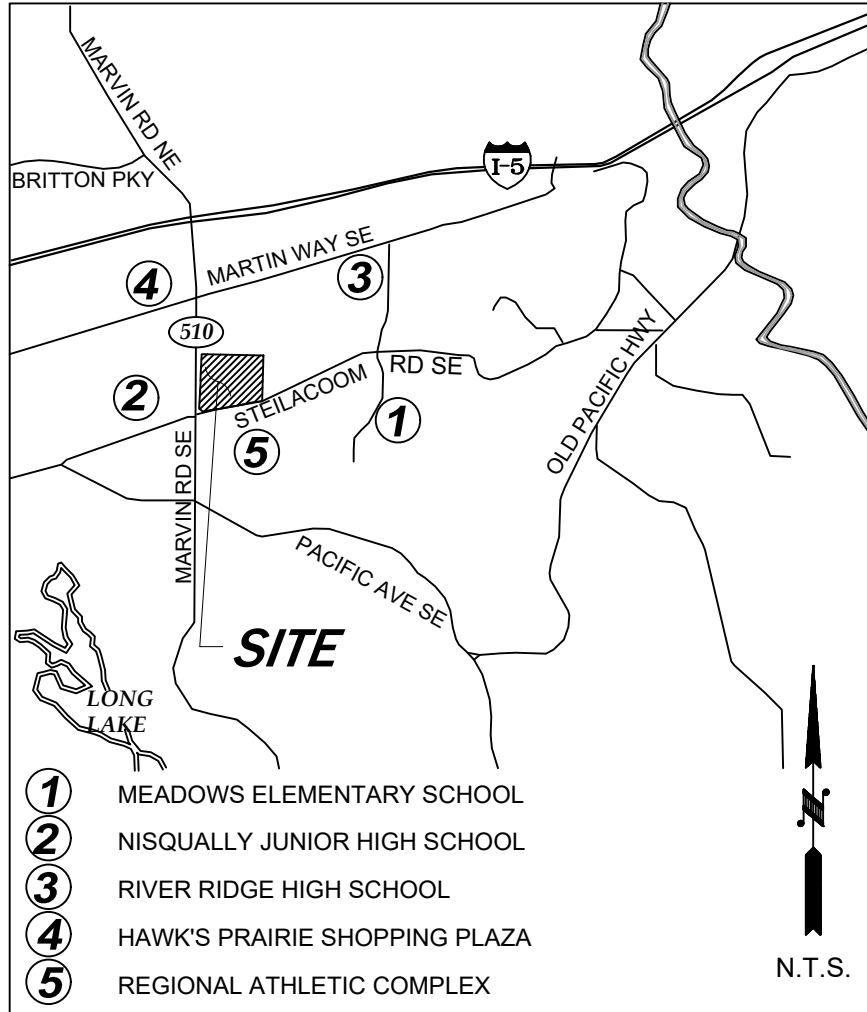
The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
33	Everett very gravelly sandy loam, 8 to 15 percent slopes	0.2	0.8%
109	Spana gravelly loam	6.9	21.6%
110	Spanaway gravelly sandy loam, 0 to 3 percent slopes	24.9	77.7%
Totals for Area of Interest		32.0	100.0%

MOREL MEADOWS

8322 STEILACOOM RD SE, LACEY, WA



VICINITY MAP

MOREL MEADOWS

8322 STEILACOOM RD SE, LACEY, WA



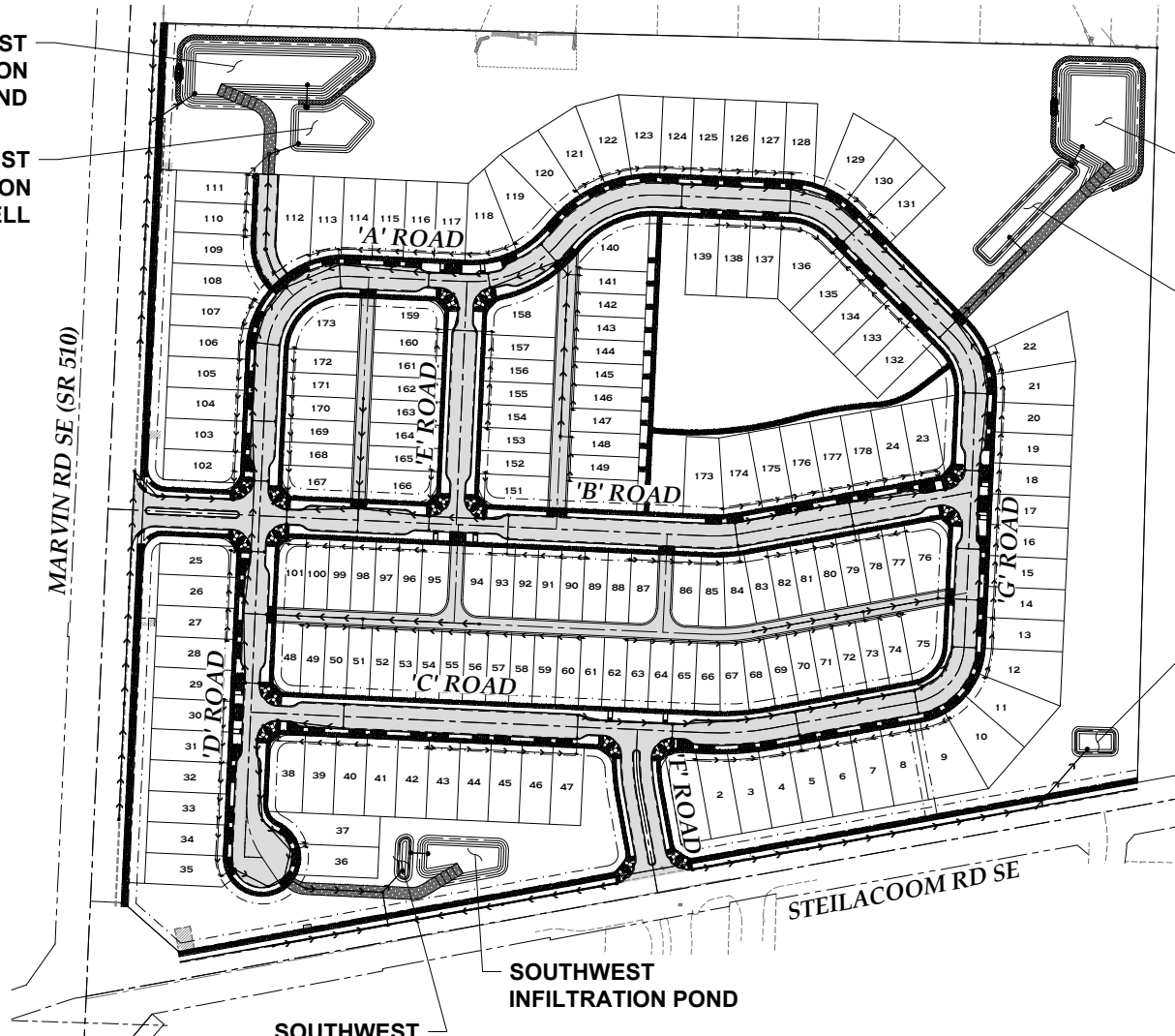
NORTHWEST
INFILTRATION
POND

NORTHWEST
BIORETENTION
CELL

NORTHEAST
INFILTRATION
POND

NORTHEAST
BIORETENTION
CELL

MARVIN RD SE (SR 510)



SOUTHEAST
BIORETENTION
CELL

SOUTHWEST
INFILTRATION
POND

SOUTHWEST
BIORETENTION
CELL

SITE PLAN

MOREL MEADOWS

8322 STEILACOOM RD SE, LACEY, WA



EXISTING
ONSITE
STRUCTURES

EXISTING
POND

MARVIN RD SE (SR 510)

EXISTING
ONSITE
STRUCTURES

EXISTING
ONSITE
STRUCTURES

STORMWATER
RUNOFF (TYP)

STEILACOOM RD SE

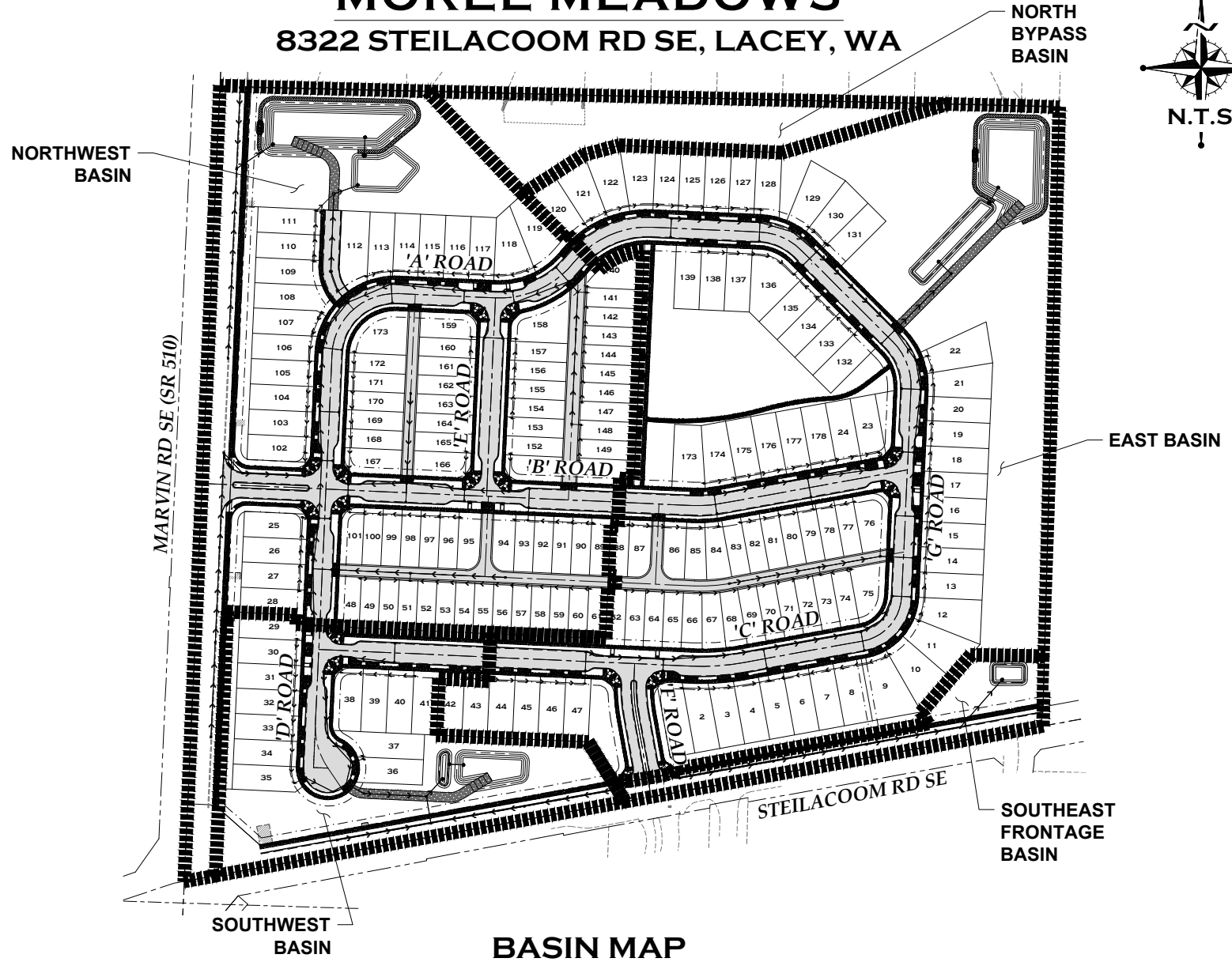
EXISTING
POND

EXISTING CONDITIONS



MOREL MEADOWS

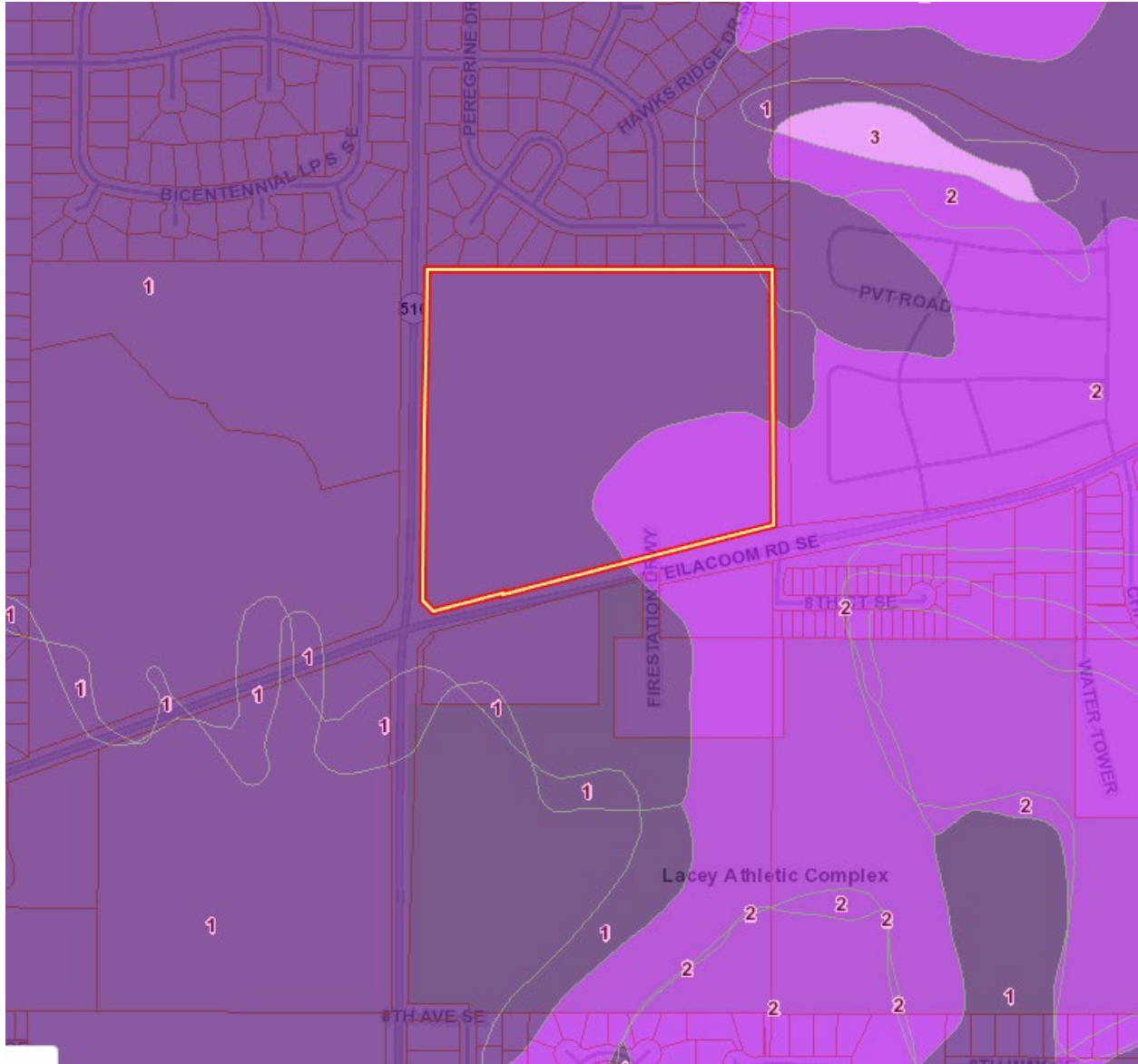
8322 STEILACOOM RD SE, LACEY, WA



BASIN MAP

Appendix 2: Supplemental Reports and Information

Critical Aquifer Recharge Area Map from Thurston County Geodata



CITY OF LACEY STORMWATER POLLUTION SOURCE CONTROL CHECKLIST

Project Name: Morel Meadows

Check all activities that will occur at a proposed site. Only activities common in the City of Lacey are included in this checklist. Other activities may apply to your site. Fill in the blank rows included under each activity grouping if needed based on the complete list of site-specific activities provided in Table 9A.1.

Source Control BMPs Applicable to All Sites		
BMP #	BMP Name	
S410	Correcting Illicit Discharges to Storm Drains	
S453	Formation of a Pollution Prevention Team	
S454	Preventive Maintenance/Good Housekeeping	
S455	Spill Prevention and Cleanup	
S456	Employee Training	
S457	Inspections	
S458	Record Keeping	
Source Control BMPs for Specific Activities		
BMP #	BMP Name	Activity Conducted on the Site?
Cleaning or Washing Source Control BMPs		
S431	Washing and Steam Cleaning Vehicles/Equipment/Building Structures	<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No
_____	_____	<input type="checkbox"/> Yes <input type="checkbox"/> No
Roads, Ditches, and Parking Lot Source Control BMPs		
S415	Maintenance of Public and Private Utility Corridors and Facilities	<input type="checkbox"/> Yes <input type="checkbox"/> No
S416	Maintenance of Roadside Ditches	<input type="checkbox"/> Yes <input type="checkbox"/> No
S417	Maintenance of Stormwater Drainage and Treatment Systems	<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No
S421	Parking and Storage of Vehicles and Equipment	<input type="checkbox"/> Yes <input type="checkbox"/> No
S430	Urban Streets	<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No
_____	_____	<input type="checkbox"/> Yes <input type="checkbox"/> No
Soil Erosion, Sediment Control, and Landscaping Source Control BMPs		
S407	Dust Control at Disturbed Land Areas and Unpaved Roadways and Parking Lots	<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No
S408	Dust Control at Manufacturing Areas	<input type="checkbox"/> Yes <input type="checkbox"/> No
S411	Landscaping and Lawn/Vegetation Management	<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No
S425	Soil Erosion and Sediment Control at Industrial Sites	<input type="checkbox"/> Yes <input type="checkbox"/> No
S435	Pesticides and an Integrated Pest Management Program	<input type="checkbox"/> Yes <input type="checkbox"/> No

BMP #	BMP Name	Activity Conducted on the Site?
Soil Erosion, Sediment Control, and Landscaping Source Control BMPs (continued)		
S444	Storage of Dry Pesticides and Fertilizers	<input type="checkbox"/> Yes <input type="checkbox"/> No
S449	Nurseries and Greenhouses	<input type="checkbox"/> Yes <input type="checkbox"/> No
S450	Irrigation	<input type="checkbox"/> Yes <input type="checkbox"/> No
_____	_____	<input type="checkbox"/> Yes <input type="checkbox"/> No
Storage and Stockpiling Source Control BMPs		
S427	Storage of Liquids, Food Waste, or Dangerous Waste Containers	<input type="checkbox"/> Yes <input type="checkbox"/> No
S428	Storage of Liquids in Permanent Aboveground Tanks	<input type="checkbox"/> Yes <input type="checkbox"/> No
S429	Storage or Transfer (Outside) of Solid Raw Materials, Byproducts or Finished Products	<input type="checkbox"/> Yes <input type="checkbox"/> No
_____	_____	<input type="checkbox"/> Yes <input type="checkbox"/> No
Transfer of Liquid or Solid Materials Source Control BMPs		
S409	Fueling at Dedicated Stations	<input type="checkbox"/> Yes <input type="checkbox"/> No
S412	Loading and Unloading Areas for Liquid or Solid Material	<input type="checkbox"/> Yes <input type="checkbox"/> No
S419	Mobile Fueling of Vehicles and Heavy Equipment	<input type="checkbox"/> Yes <input type="checkbox"/> No
S426	Spills of Oil and Hazardous Substances	<input type="checkbox"/> Yes <input type="checkbox"/> No
_____	_____	<input type="checkbox"/> Yes <input type="checkbox"/> No
Other Source Control BMPs		
S404	Commercial Printing Operations	<input type="checkbox"/> Yes <input type="checkbox"/> No
S414	Maintenance and Repair of Vehicles and Equipment	<input type="checkbox"/> Yes <input type="checkbox"/> No
S418	Manufacturing Activities – Outside	<input type="checkbox"/> Yes <input type="checkbox"/> No
S420	Painting/Finishing/Coating of Vehicles/Boats/Buildings/ Equipment	<input type="checkbox"/> Yes <input type="checkbox"/> No
S423	Recyclers and Scrap Yards	<input type="checkbox"/> Yes <input type="checkbox"/> No
S424	Roof/Building Drains at Manufacturing and Commercial Buildings	<input type="checkbox"/> Yes <input type="checkbox"/> No
S432	Wood Treatment Areas	<input type="checkbox"/> Yes <input type="checkbox"/> No
S433	Pools, Spas, Hot Tubs, and Fountains	<input type="checkbox"/> Yes <input type="checkbox"/> No
S438	Construction Demolition	<input type="checkbox"/> Yes <input type="checkbox"/> No
S443	Fertilizer Application	<input type="checkbox"/> Yes <input type="checkbox"/> No
S447	Roof Vents	<input type="checkbox"/> Yes <input type="checkbox"/> No
S451	Building, Repair, Remodeling, Painting, and Construction	<input type="checkbox"/> Yes <input type="checkbox"/> No
_____	_____	<input type="checkbox"/> Yes <input type="checkbox"/> No



GEOTECHNICAL ENGINEERING REPORT

PREPARED BY:

**THE RILEY GROUP, INC.
17522 BOTHELL WAY NORTHEAST
BOTHELL, WASHINGTON 98011**

PREPARED FOR:

**D.R. HORTON
11241 SLATER AVENUE NORTHEAST, SUITE 200
KIRKLAND, WASHINGTON 98034**

RGI PROJECT NO. 2022-007-2

**OSTROMS PLAT
8322 STEILACOOM ROAD SOUTHEAST
LACEY, WASHINGTON**

JUNE 28, 2022



June 28, 2022

Raelyn Hulquist
D.R. Horton
11241 Slater Avenue Northeast, Suite 200
Kirkland, Washington 98034

**Subject: Geotechnical Engineering Report
Ostroms Plat
8322 Steilacoom Road Southeast
Lacey, Washington
RGI Project No. 2022-007-2**

Dear Raelyn Hulquist:

As requested, The Riley Group, Inc. (RGI) has performed a Geotechnical Engineering Report (GER) for the Ostroms Plat project located at 8322 Steilacoom Road Southeast, Lacey, Washington. Our services were completed in accordance with our proposal dated January 11, 2022 and authorized by Clint Lucas with D.R. Horton on February 14, 2022. The information in this GER is based on our understanding of the proposed construction, and the soil and groundwater conditions encountered in the monitoring wells and test pits completed by RGI at the site on January 20, and February 3 and 4, 2022.

RGI recommends that you submit the project plans and specifications to RGI for a general review so that we may confirm that the recommendations in this GER are interpreted and implemented properly in the construction documents. RGI also recommends that a representative of our firm be present on site during portions of the project construction to confirm that the soil and groundwater conditions are consistent with those that form the basis for the engineering recommendations in this GER.

If you have any questions or require additional information, please contact us.

Respectfully submitted,

THE RILEY GROUP, INC.

A handwritten signature in blue ink, appearing to read 'Eric L. Woods'.

Eric L. Woods, LG
Project Geologist



Kristina M. Weller, PE
Principal Geotechnical Engineer

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Figure 3	Retaining Wall Drainage Detail
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Executive Summary

This Executive Summary should be used in conjunction with the entire Geotechnical Engineering Report (GER) for design and/or construction purposes. It should be recognized that specific details were not included or fully developed in this section, and the GER must be read in its entirety for a comprehensive understanding of the items contained herein. Section 7.0 should be read for an understanding of limitations.

RGI's geotechnical scope of work included the advancement of 2 monitoring wells and 11 test pits to approximate depths of 5 to 25 feet below existing site grades.

Based on the information obtained from our subsurface exploration, the site is suitable for development of the proposed project. The following geotechnical considerations were identified:

Soil Conditions: The soils encountered during field exploration include up to 12 feet of loose to medium dense fill comprised of gravel with some sand and trace silt and silty sandy gravel over medium dense native deposits of silty sandy gravel, sandy gravel with trace silt, and gravel with trace to some sand and silt. The gravel deposits were underlain by dense silty sand with gravel at a depth of 14 feet at TP-2.

Groundwater: Light groundwater seepage was encountered at two locations at depths of 13.5 to 23.5 feet during our subsurface exploration.

Foundations: Foundations for the proposed building may be supported on conventional spread footings bearing on medium dense native soil or structural fill.

Slab-on-grade: Slab-on-grade floors and slabs for the proposed building can be supported on medium dense native soil or structural fill.

Pavements: The following pavement sections are recommended in accordance with the City of Lacey standards:

- **For minor local access streets:** 4 inches of Hot Mix Asphalt (HMA) class ½ inch PG 64-22 over 2 inches of crushed surfacing base course (CSBC) over 13 inches of ballast or 9 inches of crushed surfacing base course (CSBC)
- **For concrete pavement areas:** 4 inches of concrete for sidewalks and 6 inches for driveways over 1 inch of CSTC or well graded sand

1.0 Introduction

This Geotechnical Engineering Report (GER) presents the results of the geotechnical engineering services provided for the Ostroms Plat project in Lacey, Washington. The purpose of this evaluation is to assess subsurface conditions and provide geotechnical recommendations for the construction of a residential development with associated roadways and infrastructure. Our scope of services included field explorations, laboratory testing, engineering analyses, and preparation of this GER.

The recommendations in the following sections of this GER are based upon our current understanding of the proposed site development as outlined below. If actual features vary or changes are made, RGI should review them in order to modify our recommendations as required. In addition, RGI requests to review the site grading plan, final design drawings and specifications when available to verify that our project understanding is correct and that our recommendations have been properly interpreted and incorporated into the project design and construction.

2.0 Project description

The project site is located at 8322 Steilacoom Road Southeast in Lacey, Washington. The approximate location of the site is shown on Figure 1.

The existing site is occupied by several concrete buildings with asphalt, concrete, and gravel roadways. RGI understands the existing structures are to be demolished and the site is to be developed with a 193-lot residential plat with associated paved streets and utilities, and stormwater facilities.

At the time of preparing this GER, building plans were not available for our review. Based on our experience with similar construction, RGI anticipates that the proposed residences will be supported on perimeter walls with bearing loads of two to six kips per linear foot, and a series of columns with a maximum load up to 30 kips. Slab-on-grade floor loading of 150 pounds per square foot (psf) are expected.

3.0 Field Exploration and Laboratory Testing

3.1 FIELD EXPLORATION

On January 20, RGI observed the drilling of 2 monitoring wells and on February 3 and 4, 2022 the excavation of 11 test pits. The approximate exploration locations are shown on Figure 2.

Field logs of each exploration were prepared by the geologist that continuously observed the excavation or drilling. These logs included visual classifications of the materials encountered during excavation and drilling as well as our interpretation of the subsurface

conditions between samples. The monitoring well and test pit logs included in Appendix A represent an interpretation of the field logs and include modifications based on laboratory observation and analysis of the samples.

3.2 LABORATORY TESTING

During the field exploration, a representative portion of each recovered sample was sealed in containers and transported to our laboratory for further visual and laboratory examination. Selected samples retrieved from the test pits were tested for moisture content and grain size analysis to aid in soil classification and provide input for the recommendations provided in this GER. The results and descriptions of the laboratory tests are enclosed in Appendix A.

4.0 Site Conditions

4.1 SURFACE

The subject site is an irregular-shaped parcel of land approximately 33.86 acres in size. The site is bound to the north and east by residential property, to the south by Steilacoom Road Southeast, and to the west by Marvin Road Southeast.

The existing site is occupied by several concrete buildings and asphalt, concrete, and gravel roadways and parking areas. The site slopes generally north with about 18 feet of elevation change across the site. The site is vegetated with grass, blackberries, decorative shrubs, and small- to large-diameter trees.

4.2 GEOLOGY

Review of the *Geologic Map of the Lacey 7.5-minute Quadrangle, Thurston County, Washington*, by Robert L. Logan, etc. (2003) indicates that the soil in the project vicinity is mapped as Vashon recessional outwash (Qgo), which is stratified sand and gravel deposited by meltwater streams issuing from the receding Vashon ice sheet. These descriptions are generally similar to the findings in our field explorations.

4.3 SOILS

The soils encountered during field exploration include up to 12 feet of loose to medium dense fill comprised of gravel with some sand and trace silt and silty sandy gravel over medium dense native deposits of silty sandy gravel, sandy gravel with trace silt, and gravel with trace to some sand and silt. The gravel deposits were underlain by dense silty sand with gravel at a depth of 14 feet at TP-2.

More detailed descriptions of the subsurface conditions encountered are presented in the test pits included in Appendix A. Sieve analysis was performed on five selected soil samples. Grain size distribution curves are included in Appendix A.

4.4 GROUNDWATER

Light groundwater seepage was encountered at two locations at depths of 13.5 to 23.5 feet during our subsurface exploration.

It should be recognized that fluctuations of the groundwater table will occur due to seasonal variations in the amount of rainfall, runoff, and other factors not evident at the time the explorations were performed. In addition, perched water can develop within seams and layers contained in fill soils or higher permeability soils overlying less permeable soils following periods of heavy or prolonged precipitation. Therefore, groundwater levels during construction or at other times in the future may be higher or lower than the levels indicated on the logs. Groundwater level fluctuations should be considered when developing the design and construction plans for the project.

4.5 SEISMIC CONSIDERATIONS

Based on the International Building Code (IBC), RGI recommends the follow seismic parameters for design.

Table 1 IBC

Parameter	2018 Value
Site Soil Class ¹	D ²
Site Latitude	47.0483
Site Longitude	-122.7609
Short Period Spectral Response Acceleration, S_s (g)	1.378
1-Second Period Spectral Response Acceleration, S_1 (g)	0.5
Adjusted Short Period Spectral Response Acceleration, S_{MS} (g)	1.378
Adjusted 1-Sec Period Spectral Response Acceleration, S_{M1} (g)	0.9 ³
Numeric seismic design value at 0.2 second; $S_{D5}(g)$	0.919
Numeric seismic design value at 1.0 second; $S_{D1}(g)$	0.6 ³

1. Note: In general accordance with Chapter 20 of ASCE 7-16, the Site Class is based on the average characteristics of the upper 100 feet of the subsurface profile.

2. Note: ASCE 7-16 require a site soil profile determination extending to a depth of 100 feet for seismic site classification. The current scope of our services does not include the required 100 foot soil profile determination. Test pits extended to a maximum depth of 25 feet, and this seismic site class definition considers that very dense soil continues below the maximum depth of the subsurface exploration. Additional exploration to deeper depths would be required to confirm the conditions below the current depth of exploration.

3. Note: In accordance with ASCE 11.4.8, a ground motion hazard analysis is not required for the following cases:

- Structures on Site Class E sites with S_s greater than or equal to 1.0, provided the site coefficient F_a is taken as equal to that of Site Class C.
- Structures on Site Class D sites with S_1 greater than or equal to 0.2, provided that the value of the seismic response coefficient C_s is determined by Eq. 12.8-2 for values of $T \leq 1.5T_s$ and taken as equal to 1.5 times the value computed in accordance with either Eq. 12.8-3 for $T_L \geq T > 1.5T_s$ or Eq. 12.8-4 for $T > T_L$.

- Structures on Site Class E sites with S_1 greater than or equal to 0.2, provided that T is less than or equal to T_s and the equivalent static force procedure is used for design.

The above exceptions do not apply to seismically isolated structures, structures with damping systems or structures designed using the response history procedures of Chapter 16.

Liquefaction is a phenomenon where there is a reduction or complete loss of soil strength due to an increase in water pressure induced by vibrations from a seismic event. Liquefaction mainly affects geologically recent deposits of fine-grained sands that are below the groundwater table. Soils of this nature derive their strength from intergranular friction. The generated water pressure or pore pressure essentially separates the soil grains and eliminates this intergranular friction, thus reducing or eliminating the soil's strength.

RGI reviewed the results of the field and laboratory testing and assessed the potential for liquefaction of the site's soil during an earthquake. Since the site is underlain by medium dense gravel and lacks an established shallow groundwater table, RGI considers that the possibility of liquefaction during an earthquake is low. Review of the *Liquefaction Susceptibility Map of Thurston County, Washington* by Stephen P. Palmer, etc. (2004) indicates the site is mapped as having a very low liquefaction susceptibility.

4.6 GEOLOGIC HAZARD AREAS

Regulated geologically hazardous areas include erosion, landslide, earthquake, or other geological hazards. Based on the definitions in the Thurston County Code, the site does not contain geologically hazardous areas.

5.0 Discussion and Recommendations

5.1 GEOTECHNICAL CONSIDERATIONS

Based on our study, the site is suitable for the proposed construction from a geotechnical standpoint. Foundations for the proposed building can be supported on conventional spread footings bearing on medium dense native soil or structural fill. Slab-on-grade floors and pavements can be similarly supported.

Detailed recommendations regarding the above issues and other geotechnical design considerations are provided in the following sections. These recommendations should be incorporated into the final design drawings and construction specifications.

5.2 EARTHWORK

Earthwork during plat work will include excavating the retention ponds, grading the lots, installing underground utilities, preparing roadway and sidewalk subgrades. The existing fill soils should be evaluated during construction and may need to be removed under structures and roadways. It may be possible to reuse some of the materials for structural fill provided the organic content is suitable and the debris is removed from the soils.

Earthwork for the home construction, should include excavating and backfilling building foundations and tying into the lot utilities.

5.2.1 EROSION AND SEDIMENT CONTROL

Potential sources or causes of erosion and sedimentation depend on construction methods, slope length and gradient, amount of soil exposed and/or disturbed, soil type, construction sequencing and weather. The impacts on erosion-prone areas can be reduced by implementing an erosion and sedimentation control plan. The plan should be designed in accordance with applicable city and/or county standards.

RGI recommends the following erosion control Best Management Practices (BMPs):

- Scheduling site preparation and grading for the drier summer and early fall months and undertaking activities that expose soil during periods of little or no rainfall
- Retaining existing vegetation whenever feasible
- Establishing a quarry spall construction entrance
- Installing siltation control fencing or anchored straw or coir wattles on the downhill side of work areas
- Covering soil stockpiles with anchored plastic sheeting
- Revegetating or mulching exposed soils with a minimum 3-inch thickness of straw if surfaces will be left undisturbed for more than one day during wet weather or one week in dry weather
- Directing runoff away from exposed soils and slopes
- Minimizing the length and steepness of slopes with exposed soils and cover excavation surfaces with anchored plastic sheeting (Graded and disturbed slopes should be tracked in place with the equipment running perpendicular to the slope contours so that the track marks provide a texture to help resist erosion and channeling. Some sloughing and raveling of slopes with exposed or disturbed soil should be expected.)
- Decreasing runoff velocities with check dams, straw bales or coir wattles
- Confining sediment to the project site
- Inspecting and maintaining erosion and sediment control measures frequently (The contractor should be aware that inspection and maintenance of erosion control BMPs is critical toward their satisfactory performance. Repair and/or replacement of dysfunctional erosion control elements should be anticipated.)

Permanent erosion protection should be provided by reestablishing vegetation using hydroseeding and/or landscape planting. Until the permanent erosion protection is established, site monitoring should be performed by qualified personnel to evaluate the effectiveness of the erosion control measures. Provisions for modifications to the erosion control system based on monitoring observations should be included in the erosion and sedimentation control plan.

5.2.2 STRIPPING

Stripping efforts should include removal of pavements, vegetation, organic materials, and deleterious debris from areas slated for building, pavement, and utility construction. The test pits encountered 6 to 30 inches of topsoil and rootmass. Deeper areas of stripping may be required in forested or heavily vegetated areas of the site.

5.2.3 EXCAVATIONS

All temporary cut slopes associated with the site and utility excavations should be adequately inclined to prevent sloughing and collapse. The site soils consist of gravel with varying amounts of sand and silt.

Accordingly, for excavations more than 4 feet but less than 20 feet in depth, the temporary side slopes should be laid back with a minimum slope inclination of 1.5H:1V (Horizontal:Vertical). If there is insufficient room to complete the excavations in this manner, or excavations greater than 20 feet in depth are planned, using temporary shoring to support the excavations should be considered. For open cuts at the site, RGI recommends:

- No traffic, construction equipment, stockpiles or building supplies are allowed at the top of cut slopes within a distance of at least five feet from the top of the cut
- Exposed soil along the slope is protected from surface erosion using waterproof tarps and/or plastic sheeting
- Construction activities are scheduled so that the length of time the temporary cut is left open is minimized
- Surface water is diverted away from the excavation
- The general condition of slopes should be observed periodically by a geotechnical engineer to confirm adequate stability and erosion control measures

In all cases, however, appropriate inclinations will depend on the actual soil and groundwater conditions encountered during earthwork. Ultimately, the site contractor must be responsible for maintaining safe excavation slopes that comply with applicable OSHA or WISHA guidelines.

5.2.4 SITE PREPARATION

RGI anticipates that some areas of loose or soft soil will be exposed upon completion of stripping and grubbing. Proofrolling and subgrade verification should be considered an essential step in site preparation. After stripping, grubbing, and prior to placement of structural fill, RGI recommends proofrolling building and pavement subgrades and areas to receive structural fill. These areas should moisture conditioned and compacted to a firm and unyielding condition in order to achieve a minimum compaction level of 95 percent of the modified proctor maximum dry density as determined by the American Society of

Testing and Materials D1557-09 Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (ASTM D1557).

Proofrolling and adequate subgrade compaction can only be achieved when the soils are within approximately ± 2 percent moisture content of the optimum moisture content. Soils which appear firm after stripping and grubbing may be proofrolled with a heavy compactor, loaded double-axle dump truck, or other heavy equipment under the observation of an RGI representative. This observer will assess the subgrade conditions prior to filling. The need for or advisability of proofrolling due to soil moisture conditions should be determined at the time of construction. In wet areas it may be necessary to hand probe the exposed subgrades in lieu of proofrolling with mechanical equipment.

If fill is placed in areas of the site where existing slopes are steeper than 5:1 (Horizontal:Vertical), the area should be benched to reduce the potential for slippage between existing slopes and fills. Benches should be wide enough to accommodate compaction and earth moving equipment, and to allow placement of horizontal lifts of fill.

Subgrade soils that become disturbed due to elevated moisture conditions should be overexcavated to reveal firm, non-yielding, non-organic soils and backfilled with compacted structural fill. In order to maximize utilization of site soils as structural fill, RGI recommends that the earthwork portion of this project be completed during extended periods of warm and dry weather if possible. If earthwork is completed during the wet season (typically November through May) it will be necessary to take extra precautionary measures to protect subgrade soils. Wet season earthwork will require additional mitigative measures beyond that which would be expected during the drier summer and fall months.

5.2.5 STRUCTURAL FILL

Once stripping, clearing and other preparing operations are complete, cuts and fills can be made to establish desired building grades. Prior to placing fill, RGI recommends proof-rolling as described above. The existing fill soils should be evaluated during construction and may need to be removed under structures and roadways. It may be possible to reuse some of the materials for structural fill provided the organic content is suitable and the debris is removed from the soils.

RGI recommends fill below the foundation and floor slab, behind retaining walls, and below pavement and hardscape surfaces be placed in accordance with the following recommendations for structural fill. The structural fill should be placed after completion of site preparation procedures as described above.

The suitability of excavated site soils and import soils for compacted structural fill use will depend on the gradation and moisture content of the soil when it is placed. As the amount of fines (that portion passing the U.S. No. 200 sieve) increases, soil becomes increasingly

sensitive to small changes in moisture content and adequate compaction becomes more difficult or impossible to achieve. Soils containing more than about 5 percent fines cannot be consistently compacted to a dense, non-yielding condition when the moisture content is more than 2 percent above or below optimum. Optimum moisture content is that moisture that results in the greatest compacted dry density with a specified compactive effort.

Non-organic site soils are only considered suitable for structural fill provided that their moisture content is within about two percent of the optimum moisture level as determined by ASTM D1557. Excavated site soils may not be suitable for re-use as structural fill depending on the moisture content and weather conditions at the time of construction. If soils are stockpiled for future reuse and wet weather is anticipated, the stockpile should be protected with plastic sheeting that is securely anchored.

Even during dry weather, moisture conditioning (such as, windrowing and drying) of site soils to be reused as structural fill may be required. Even during the summer, delays in grading can occur due to excessively high moisture conditions of the soils or due to precipitation. If wet weather occurs, the upper wetted portion of the site soils may need to be scarified and allowed to dry prior to further earthwork, or may need to be wasted from the site.

If on-site soils are or become unusable, it may become necessary to import clean, granular soils to complete site work that meet the grading requirements listed in Table 2 to be used as structural fill.

Table 2 Structural Fill Gradation

U.S. Sieve Size	Percent Passing
4 inches	100
No. 4 sieve	22 to 100
No. 200 sieve	0 to 5*

*Based on minus 3/4 inch fraction.

Prior to use, an RGI representative should observe and test all materials imported to the site for use as structural fill. Structural fill materials should be placed in uniform loose layers not exceeding 12 inches and compacted as specified in Table 3. The soil's maximum density and optimum moisture should be determined by ASTM D1557.

Table 3 Structural Fill Compaction ASTM D1557

Location	Material Type	Minimum Compaction Percentage	Moisture Content Range	
Foundations	On-site granular or approved imported fill soils:	95	+2	-2
Retaining Wall Backfill	On-site granular or approved imported fill soils:	92	+2	-2
Slab-on-grade	On-site granular or approved imported fill soils:	95	+2	-2
General Fill (non-structural areas)	On-site soils or approved imported fill soils:	90	+3	-2
Pavement – Subgrade and Base Course	On-site granular or approved imported fill soils:	95	+2	-2

Placement and compaction of structural fill should be observed by RGI. A representative number of in-place density tests should be performed as the fill is being placed to confirm that the recommended level of compaction is achieved.

5.2.6 CUT AND FILL SLOPES

All permanent cut and fill slopes (except interior slopes of infiltration pond) should be graded with a finished inclination no greater than 2H:1V. The interior slopes of the infiltration pond must be graded with a slope gradient no steeper than 3H:1V. Upon completion of construction, the slope face should be trackwalked, compacted and vegetated, or provided with other physical means to guard against erosion. All fill placed for slope construction should meet the structural fill requirements as described in Section 5.2.5.

Final grades at the top of the slopes must promote surface drainage away from the slope crest. Water must not be allowed to flow in an uncontrolled fashion over the slope face. If it is necessary to direct surface runoff towards the slope, it should be controlled at the top of the slope, piped in a closed conduit installed on the slope face, and taken to an appropriate point of discharge beyond the toe of the slope.

5.2.7 WET WEATHER CONSTRUCTION CONSIDERATIONS

RGI recommends that preparation for site grading and construction include procedures intended to drain ponded water, control surface water runoff, and to collect shallow subsurface seepage zones in excavations where encountered. It will not be possible to successfully compact the subgrade or utilize on-site soils as structural fill if accumulated water is not drained prior to grading or if drainage is not controlled during construction. Attempting to grade the site without adequate drainage control measures will reduce the



amount of on-site soil effectively available for use, increase the amount of select import fill materials required, and ultimately increase the cost of the earthwork phases of the project. Free water should not be allowed to pond on the subgrade soils. RGI anticipates that the use of berms and shallow drainage ditches, with sumps and pumps in utility trenches, will be required for surface water control during wet weather and/or wet site conditions.

5.3 FOUNDATIONS

Following site preparation and grading, the proposed building foundation can be supported on conventional spread footings bearing on medium dense native soil or structural fill. Loose, organic, or other unsuitable soils may be encountered in the proposed building footprint. If unsuitable soils are encountered, they should be overexcavated and backfilled with structural fill.

Perimeter foundations exposed to weather should be at a minimum depth of 18 inches below final exterior grades. Interior foundations can be constructed at any convenient depth below the floor slab. Finished grade is defined as the lowest adjacent grade within 5 feet of the foundation for perimeter (or exterior) footings and finished floor level for interior footings.

Table 4 Foundation Design

Design Parameter	Value
Allowable Bearing Capacity	2,000 psf ¹
Friction Coefficient	0.30
Passive pressure (equivalent fluid pressure)	250 pcf ²

- 1. psf = pounds per square foot
- 2. pcf = pounds per cubic foot

The allowable foundation bearing pressures apply to dead loads plus design live load conditions. For short-term loads, such as wind and seismic, a 1/3 increase in this allowable capacity may be used. At perimeter locations, RGI recommends not including the upper 12 inches of soil in the computation of passive pressures because they can be affected by weather or disturbed by future grading activity. The passive pressure value assumes the foundation will be constructed neat against competent soil or backfilled with structural fill as described in Section 5.2.5. The recommended base friction and passive resistance value includes a safety factor of about 1.5.

With spread footing foundations designed in accordance with the recommendations in this section, maximum total and differential post-construction settlements of 1 inch and 1/2 inch, respectively, should be expected.

5.4 RETAINING WALLS

If retaining walls are needed for the residences, retaining walls in ponds or for underground vaults, RGI recommends cast-in-place concrete walls be used. Modula block wall may be used for grade changes outside of structures.

The magnitude of earth pressure development on cast-in-place retaining walls will partly depend on the quality of the wall backfill. RGI recommends placing and compacting wall backfill as structural fill. Wall drainage will be needed behind the wall face. A typical retaining wall drainage detail is shown in Figure 3.

With wall backfill placed and compacted as recommended, level backfill and drainage properly installed, RGI recommends using the values in the following table for design.

Table 5 Retaining Wall Design

Design Parameter	Value
Allowable Bearing Capacity - Structural Fill Dense native soils	2,500 psf 4,000 psf
Active Earth Pressure (unrestrained walls)	35 pcf
At-rest Earth Pressure (restrained walls)	50 pcf

For seismic design, an additional uniform load of 7 times the wall height (H) for unrestrained walls and 14H in psf for restrained walls should be applied to the wall surface. Friction at the base of foundations and passive earth pressure will provide resistance to these lateral loads. Values for these parameters are provided in Section 5.3.

5.5 SLAB-ON-GRADE CONSTRUCTION

Once site preparation has been completed as described in Section 5.2, suitable support for slab-on-grade construction should be provided. RGI recommends that the concrete slab be placed on top of medium dense native soil or structural fill. Immediately below the floor slab, RGI recommends placing a four-inch thick capillary break layer of clean, free-draining sand or gravel that has less than five percent passing the U.S. No. 200 sieve. This material will reduce the potential for upward capillary movement of water through the underlying soil and subsequent wetting of the floor slab. Where moisture by vapor transmission is undesirable, an 8- to 10-millimeter thick plastic membrane should be placed on a 4-inch thick layer of clean gravel.

For the anticipated floor slab loading, we estimate post-construction floor settlements of 1/4- to 1/2-inch.

5.6 DRAINAGE

5.6.1 SURFACE

Final exterior grades should promote free and positive drainage away from the building area. Water must not be allowed to pond or collect adjacent to foundations or within the immediate building area. For non-pavement locations, RGI recommends providing a minimum drainage gradient of 3 percent for a minimum distance of 10 feet from the building perimeter. In paved locations, a minimum gradient of 1 percent should be provided unless provisions are included for collection and disposal of surface water adjacent to the structure.

5.6.2 SUBSURFACE

RGI recommends installing perimeter foundation drains. A typical footing drain detail is shown on Figure 4. The foundation drains and roof downspouts should be tightlined separately to an approved discharge facility. Subsurface drains must be laid with a gradient sufficient to promote positive flow to a controlled point of approved discharge.

5.6.3 INFILTRATION

RGI understands that an infiltration system is being considered for the on-site disposal of stormwater run-off at the Site. A small-scale Pilot Infiltration Test (PIT) was completed in test pit TP-1 in accordance with the City of Lacey 2022 Stormwater Design Manual (CLSDM). The test pit dimensions were 4 feet by 8 feet, with the test conducted at approximately 6 feet below grade. Soil conditions at the PIT testing horizon are gravel.

Table 6 Measured Infiltration Rates

Test Location	Test Depth	Measured Rate (inches/hour)	K _{sat} Design (inches/hour)
TP-1	6	140	21

A Total Correction Factor was applied to the field measured infiltration rate.

$$\text{Total Correction Factor } CF_T = CF_v \times CF_t \times CF_m$$

$$\text{Site variability } (CF_v) = 0.33$$

$$\text{Test Method } (CF_t) = 0.5 \text{ Small scale PIT}$$

$$\text{Influent Control } (CF_m) = 0.9$$

$$CF_T = 0.33 \times 0.5 \times 0.9 = 0.15$$

Application of the Total Correction Factor yields a K_{sat} Design rate of **21 inches/hour**.

5.7 UTILITIES

Utility pipes should be bedded and backfilled in accordance with American Public Works Association (APWA) specifications. For site utilities located within the right-of-ways, bedding and backfill should be completed in accordance with City of Lacey specifications. At a minimum, trench backfill should be placed and compacted as structural fill, as described in Section 5.2.5. Where utilities occur below unimproved areas, the degree of compaction can be reduced to a minimum of 90 percent of the soil's maximum density as determined by the referenced ASTM D1557. As noted, soils excavated on site should be suitable for use as backfill material. If on-site soils are or become unusable or do not meet the City of Lacey specifications, imported structural fill meeting the gradation provided in Table 2 should be used for trench backfill.

5.8 PAVEMENTS

Pavement subgrades should be prepared as described in Section 5.2 and as discussed below. Regardless of the relative compaction achieved, the subgrade must be firm and relatively unyielding before paving. The subgrade should be proof-rolled with heavy construction equipment to verify this condition.

5.8.1 FLEXIBLE PAVEMENTS

With the pavement subgrade prepared as described above, RGI recommends the following pavement sections for parking and drive areas paved with flexible asphalt concrete surfacing.

- **For minor local access streets:** 4 inches of Hot Mix Asphalt (HMA) class ½ inch PG 64-22 over 2 inches of crushed surfacing base course (CSBC) over 13 inches of ballast or 9 inches of crushed surfacing base course (CSBC)

5.8.2 CONCRETE PAVEMENTS

With the pavement subgrade prepared as described above, RGI recommends the following pavement sections for parking and drive areas paved with concrete surfacing.

- **For concrete pavement areas:** 4 inches of concrete for sidewalks and 6 inches for driveways over 1 inch of CSTC or well graded sand

The paving materials used should conform to the WSDOT specifications for HMA, concrete paving, and 9-03.9(3) Crushed Surfacing.

Long-term pavement performance will depend on surface drainage. A poorly-drained pavement section will be subject to premature failure as a result of surface water infiltrating into the subgrade soils and reducing their supporting capability.

For optimum pavement performance, surface drainage gradients of no less than 2 percent are recommended. Also, some degree of longitudinal and transverse cracking of the

pavement surface should be expected over time. Regular maintenance should be planned to seal cracks when they occur.

6.0 Additional Services

RGI is available to provide further geotechnical consultation throughout the design phase of the project. RGI should review the final design and specifications in order to verify that earthwork and foundation recommendations have been properly interpreted and incorporated into project design and construction.

RGI is also available to provide geotechnical engineering and construction monitoring services during construction. The integrity of the earthwork and construction depends on proper site preparation and procedures. In addition, engineering decisions may arise in the field in the event that variations in subsurface conditions become apparent. Construction monitoring services are not part of this scope of work. If these services are desired, please let us know and we will prepare a cost proposal.

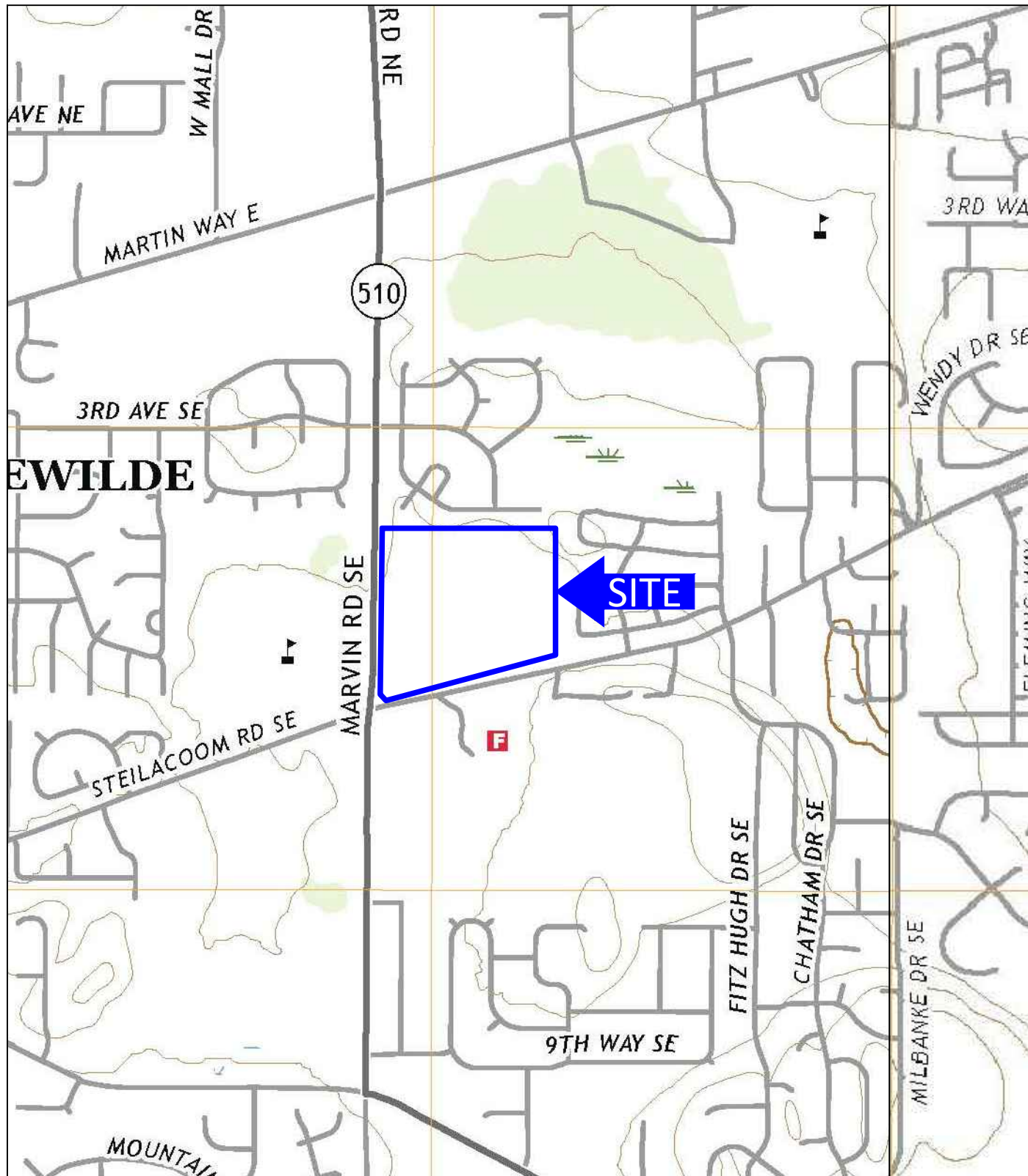
7.0 Limitations

This GER is the property of RGI, D.R. Horton, and its designated agents. Within the limits of the scope and budget, this GER was prepared in accordance with generally accepted geotechnical engineering practices in the area at the time this GER was issued. This GER is intended for specific application to the Ostroms Plat project in Lacey, Washington, and for the exclusive use of D.R. Horton and its authorized representatives. No other warranty, expressed or implied, is made. Site safety, excavation support, and dewatering requirements are the responsibility of others.

The scope of services for this project does not include either specifically or by implication any environmental or biological (for example, mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials or conditions. If the owner is concerned about the potential for such contamination or pollution, we can provide a proposal for these services.

The analyses and recommendations presented in this GER are based upon data obtained from the explorations performed on site. Variations in soil conditions can occur, the nature and extent of which may not become evident until construction. If variations appear evident, RGI should be requested to reevaluate the recommendations in this GER prior to proceeding with construction.

It is the client's responsibility to see that all parties to the project, including the designers, contractors, subcontractors, are made aware of this GER in its entirety. The use of information contained in this GER for bidding purposes should be done at the contractor's option and risk.



USGS, 2020, Nisqually, Washington
 USGS, 2020, Lacey, Washington
 7.5-Minute Quadrangle

Approximate Scale: 1"=1000'



Corporate Office
 17522 Bothell Way Northeast
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 Fax: 425.415.0311

Ostroms Plat

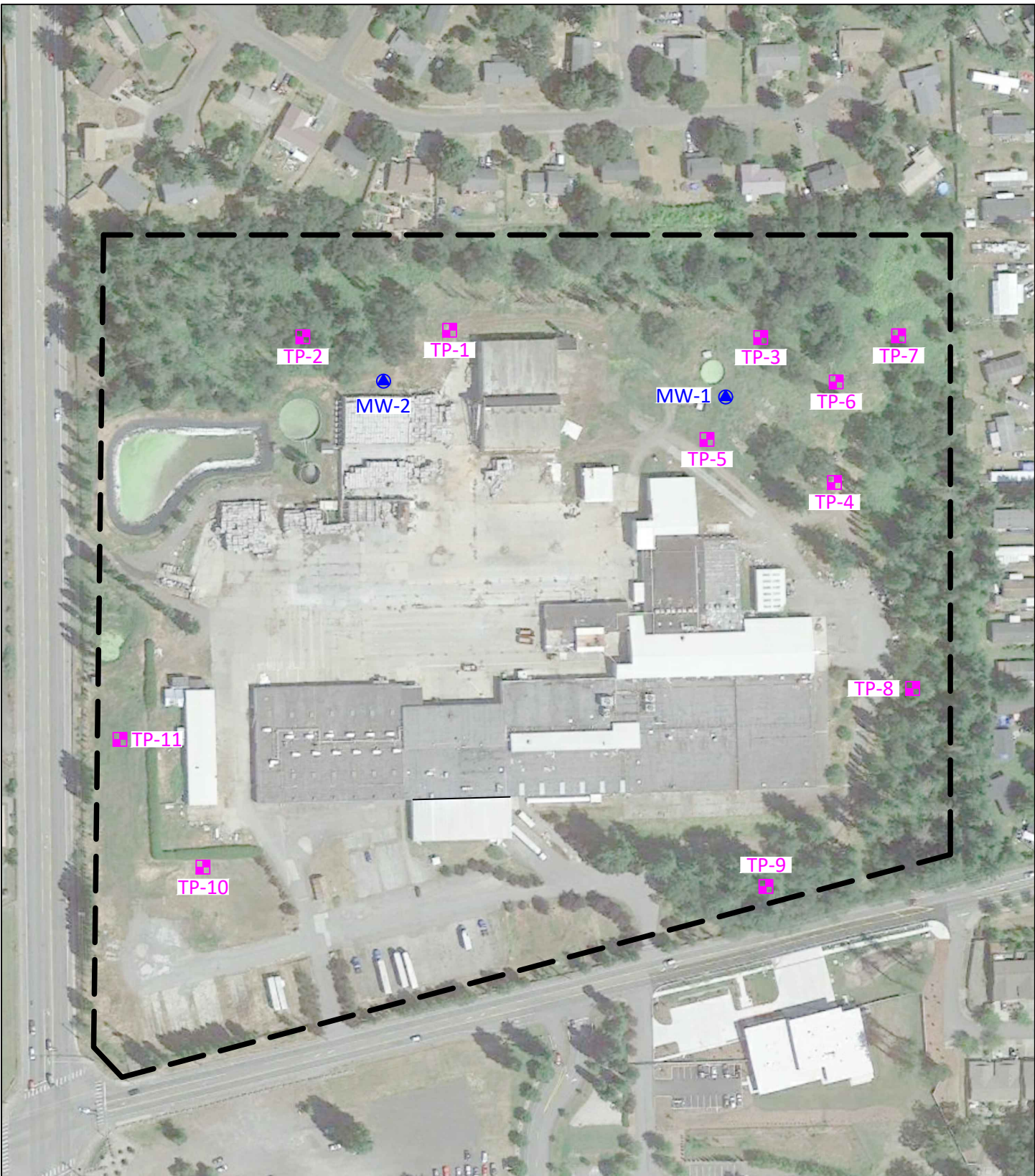
Figure 1




RGI Project Number:
 2022-007-2

Site Vicinity Map

Date Drawn:
 06/2022

Address: 8322 Steilacoom Road Southeast, Lacey, Washington 98513



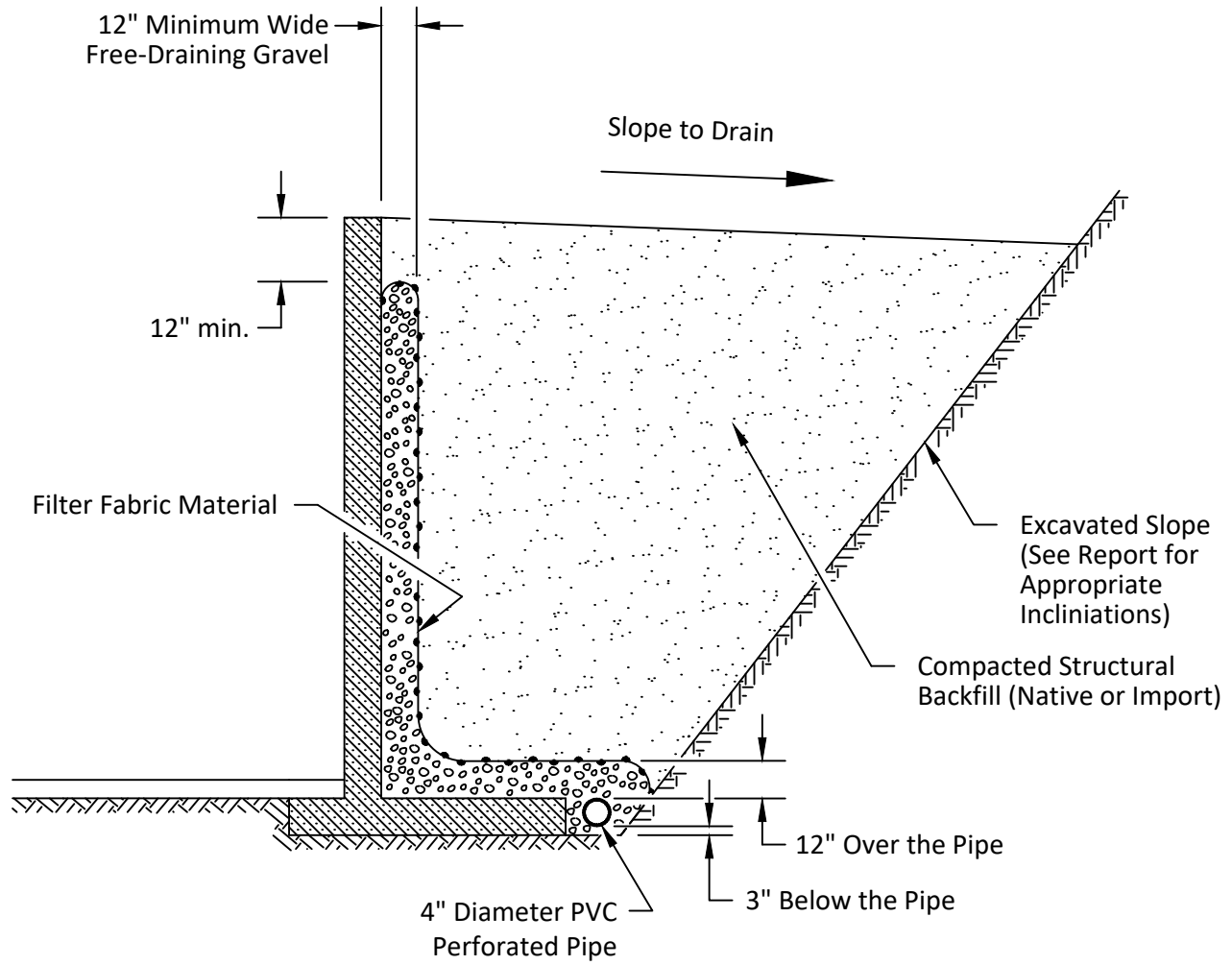
-  = Monitoring well by RGI, 01/20/22
-  = Test pit by RGI, 02/03-04/22
-  = Site boundary

Approximate Scale: 1"=200'



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Ostroms Plat		Figure 2
RGI Project Number: 2022-007-2	Geotechnical Exploration Plan	Date Drawn: 06/2022
Address: 8322 Steilacoom Road Southeast, Lacey, Washington 98513		

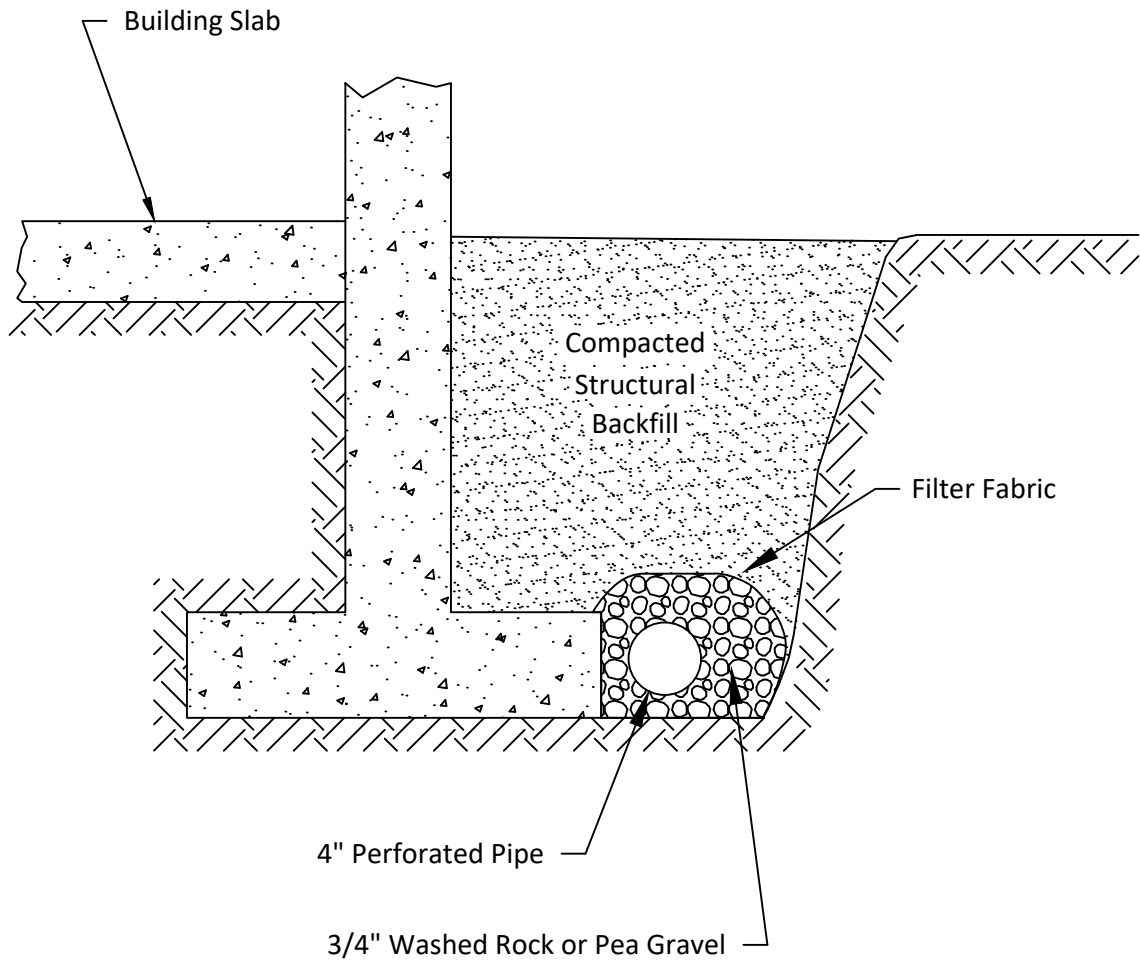


Not to Scale



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Ostroms Plat		Figure 3
RGI Project Number: 2022-007-2	Retaining Wall Drainage Detail	Date Drawn: 06/2022
Address: 8322 Steilacoom Road Southeast, Lacey, Washington 98513		



Not to Scale



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Ostroms Plat		Figure 4
RGI Project Number: 2022-007-2	Typical Footing Drain Detail	Date Drawn: 06/2022
Address: 8322 Steilacoom Road Southeast, Lacey, Washington 98513		

APPENDIX A

FIELD EXPLORATION AND LABORATORY TESTING

On January 20, and February 3 and 4, 2022, RGI performed field explorations using a drill rig and an excavator. We explored subsurface soil conditions at the site by observing the excavation/drilling of 2 monitoring wells and 11 test pits to a maximum depth of 25 feet below existing grade. The well and test pit locations are shown on Figure 2. The well and test pit locations were approximately determined by measurements from existing property lines and paved roads.

A geologist from our office conducted the field exploration and classified the soil conditions encountered, maintained a log of each test exploration, obtained representative soil samples, and observed pertinent site features. All soil samples were visually classified in accordance with the Unified Soil Classification System (USCS).

Representative soil samples obtained from the explorations were placed in closed containers and taken to our laboratory for further examination and testing. As a part of the laboratory testing program, the soil samples were classified in our in house laboratory based on visual observation, texture, plasticity, and the limited laboratory testing described below.

Moisture Content Determinations

Moisture content determinations were performed in accordance with ASTM D2216-10 Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass (ASTM D2216) on representative samples obtained from the exploration in order to aid in identification and correlation of soil types. The moisture content of typical sample was measured and is reported on the test pit logs.

Grain Size Analysis

A grain size analysis indicates the range in diameter of soil particles included in a particular sample. Grain size analyses was determined using D6913-04(2009) Standard Test Methods for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis (ASTM D6913) on five of the samples.

Project Name: **Ostroms Plat**

Project Number: **2022-007-1**

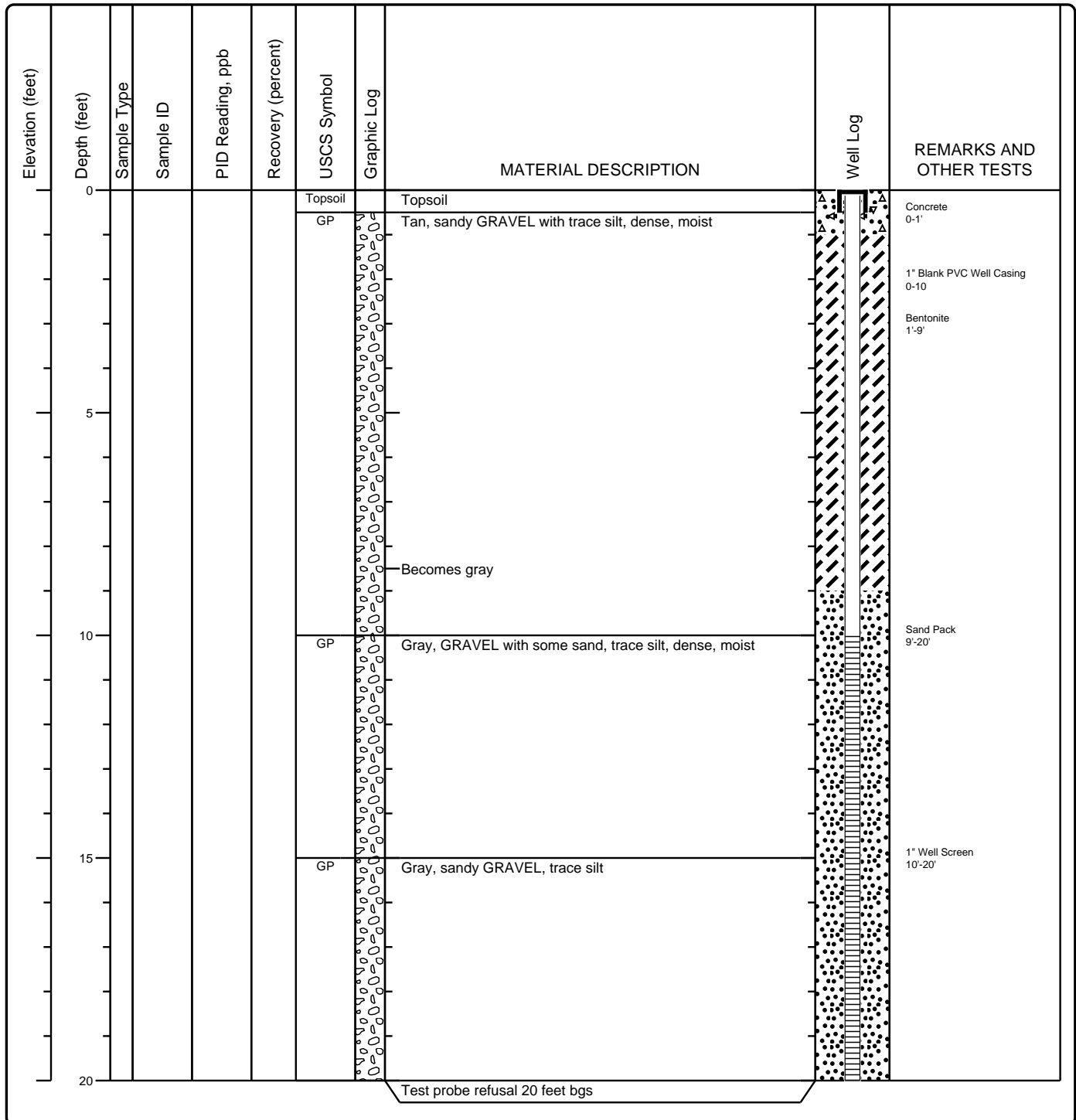
Client: **DR Horton**



Well No.: **MW-1**

Sheet 1 of 1

Date(s) Drilled: 01/20/22	Logged By: JH	Surface Conditions: Grass
Drilling Method(s): Geoprobe	Drill Bit Size/Type: 2.25"	Total Depth of Borehole: 20 feet bgs
Drill Rig Type: Geoprobe 7730 DT	Drilling Contractor: RGI	Approximate Surface Elevation (feet amsl): n/a
Groundwater Level and Date Measured: Not Encountered	Sampling Method(s): Continuous	Hammer Data : n/a
Borehole Backfill: Bentonite	Location: 8322 Steilacoom Road Southeast, Lacey, Washington 98513	



Project Name: **Ostroms Plat**

Project Number: **2022-007-1**

Client: **DR Horton**



Well No.: **MW-2**

Sheet 1 of 2

Date(s) Drilled: 01/20/22	Logged By: JH	Surface Conditions: Grass
Drilling Method(s): Geoprobe	Drill Bit Size/Type: 2.25"	Total Depth of Borehole: 25 feet bgs
Drill Rig Type: Geoprobe 7730 DT	Drilling Contractor: RGI	Approximate Surface Elevation (feet amsl): n/a
Groundwater Level and Date Measured: 23.5'	Sampling Method(s): Continuous	Hammer Data : n/a
Borehole Backfill: Native Soil	Location: 8322 Steilacoom Road Southeast, Lacey, Washington 98513	

Elevation (feet)	Depth (feet)	Sample Type	Sample ID	PID Reading, ppb	Recovery (percent)	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	Well Log	REMARKS AND OTHER TESTS
	0					Topsoil		Topsoil		Concrete 0-1'
						GP		Tan, sandy GRAVEL, trace silt, dense, moist		1" Blank PVC Well Casing 0-15' Bentonite 1'-14'
	5							Becomes gray		
	10					GP		Gray, GRAVEL with some sand, trace silt, dense, moist		
	15					GP		Gray, sandy GRAVEL, trace silt, dense, moist		Sand Pack 14'-25'
	20									1" Well Screen 15'-25'

Project Name: **Ostroms Plat**

Project Number: **2022-007-1**

Client: **DR Horton**



Well No.: **MW-2**

Sheet 2 of 2

Elevation (feet)	Depth (feet)	Sample Type	Sample ID	PID Reading, ppb	Recovery (percent)	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	Well Log	REMARKS AND OTHER TESTS
20						GP		Gray, sandy GRAVEL, trace silt, dense, moist		
25								Test probe refusal 25 feet bgs Groundwater encountered @ 23.5 feet		
30										
35										
40										
45										
50										

Project Name: **Ostroms Plat**

Project Number: **2022-007-1**

Client: **DR Horton**



Boring Log Key

Sheet 1 of 1

Elevation (feet)	Depth (feet)	Sample Type	Sample ID	PID Reading, ppb	Recovery (percent)	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	Well Log	REMARKS AND OTHER TESTS
1	2	3	4	5	6	7	8	9	10	11

COLUMN DESCRIPTIONS

- | | |
|--|---|
| <p>1 Elevation (feet): Elevation (MSL, feet).</p> <p>2 Depth (feet): Depth in feet below the ground surface.</p> <p>3 Sample Type: Type of soil sample collected at the depth interval shown.</p> <p>4 Sample ID: Sample identification number.</p> <p>5 PID Reading, ppb: The reading from a photo-ionization detector, in parts per million.</p> <p>6 Recovery (percent): Percent Recovery</p> | <p>7 USCS Symbol: USCS symbol of the subsurface material.</p> <p>8 Graphic Log: Graphic depiction of the subsurface material encountered.</p> <p>9 MATERIAL DESCRIPTION: Description of material encountered. May include consistency, moisture, color, and other descriptive text.</p> <p>10 Well Log: Graphical representation of well installed upon completion of drilling and sampling.</p> <p>11 REMARKS AND OTHER TESTS: Comments and observations regarding drilling or sampling made by driller or field personnel.</p> |
|--|---|









FIELD AND LABORATORY TEST ABBREVIATIONS

- | | |
|---|--|
| <p>CHEM: Chemical tests to assess corrosivity</p> <p>COMP: Compaction test</p> <p>CONS: One-dimensional consolidation test</p> <p>LL: Liquid Limit, percent</p> | <p>PI: Plasticity Index, percent</p> <p>SA: Sieve analysis (percent passing No. 200 Sieve)</p> <p>UC: Unconfined compressive strength test, Qu, in ksf</p> <p>WA: Wash sieve (percent passing No. 200 Sieve)</p> |
|---|--|








MATERIAL GRAPHIC SYMBOLS

- | | |
|--|---|
| <p> Bentonite chips</p> <p> Portland Cement Concrete</p> | <p> Poorly graded GRAVEL (GP)</p> <p> Poorly graded SAND (SP)</p> |
|--|---|

TYPICAL SAMPLER GRAPHIC SYMBOLS

- | | |
|---|---|
| <p> Auger sampler</p> <p> Bulk Sample</p> <p> 3-inch-OD California w/ brass rings</p> <p> CME Sampler</p> | <p> Continuous</p> <p> Grab Sample</p> <p> 2.5-inch-OD Modified California w/ brass liners</p> <p> Pitcher Sample</p> |
|---|---|

OTHER GRAPHIC SYMBOLS

- | | |
|---|---|
| <p> 2-inch-OD unlined split spoon (SPT)</p> <p> Shelby Tube (Thin-walled, fixed head)</p> | <p> Water level (at time of drilling, ATD)</p> <p> Water level (after waiting)</p> <p> Minor change in material properties within a stratum</p> <p> Inferred/gradational contact between strata</p> <p> Queried contact between strata</p> |
|---|---|

GENERAL NOTES

- Soil classifications are based on the Unified Soil Classification System. Descriptions and stratum lines are interpretive, and actual lithologic changes may be gradual. Field descriptions may have been modified to reflect results of lab tests.
- Descriptions on these logs apply only at the specific boring locations and at the time the borings were advanced. They are not warranted to be representative of subsurface conditions at other locations or times.

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-1**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 9 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s): Grab	Compaction Method: Bucket
Test Pit Backfill: Cuttings	Location: 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		Black gravelly topsoil with trace garbage debris, loose, moist (Fill)	
				GP		Brown GRAVEL with trace sand and silt, medium dense, moist	3% moisture, 1% fines
	5					Infiltration test at 6'	3% moisture, 2% fines
				GW		Brown GRAVEL with trace sand and silt, medium dense, moist	3% moisture, 1% fines
						Abundant cobbles and boulders	
	10					Test Pit terminated at 9'	
15							

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-2**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Blackberries
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 14.5 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Seepage at 13.5'	Sampling Method(s): Grab	Compaction Method: Bucket
Test Pit Backfill: Cuttings	Location: 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		2.5' topsoil	
				GP-GM		Brown GRAVEL with some sand and silt, medium dense, moist	
5				GP		Gray GRAVEL with trace sand and silt, medium dense, moist	
						Occasional cobble	
						Abundant cobbles	
10						Occasional boulder	
						Light groundwater seepage	
				SM		Gray silty SAND with some gravel, dense, moist	9% moisture
15						Test Pit terminated at 14.5'	

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-3**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 12 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s): Grab	Compaction Method: Bucket
Test Pit Backfill: Cuttings	Location: 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		12" topsoil	
				Fill		Gray GRAVEL with some sand and trace silt, medium dense, moist (Fill)	
				TPSL		Topsoil	
	5			GP-GM		Brown GRAVEL with some sand and silt, medium dense, moist	14% moisture
				GP		Gray GRAVEL with trace sand and silt, medium dense, moist	3% moisture
	10						
						Test Pit terminated at 12'	
	15						

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-4**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 6 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s): Grab	Compaction Method: Bucket
Test Pit Backfill: Cuttings	Location: 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		6" topsoil	
				Fill		Gray GRAVEL with some sand and trace silt, medium dense, moist (Fill)	
				GP		Brown GRAVEL with trace sand and silt, medium dense, moist	
						Occasional cobble	5% moisture
						Test Pit terminated at 6'	
5							
10							
15							

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

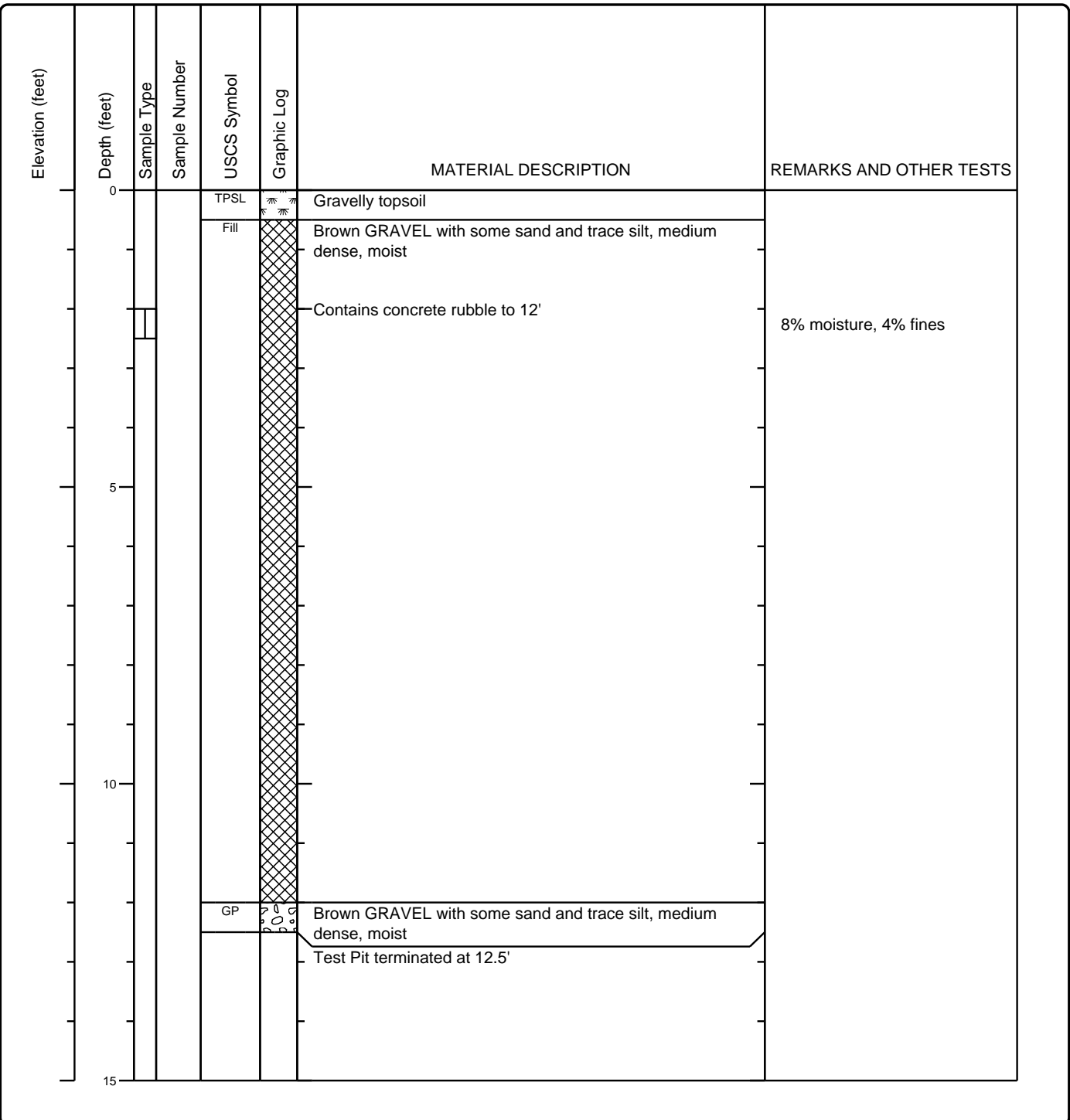
Client: **D.R. Horton**



Test Pit No.: **TP-5**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 12.5 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s): Grab	Compaction Method: Bucket
Test Pit Backfill: Cuttings	Location: 8322 Steilacoom Road Southeast, Lacey, Washington	



Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-6**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 9 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s)	Compaction Method Bucket
Test Pit Backfill: Cuttings	Location 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		6" topsoil	
				Fill		Dark brown GRAVEL with some sand and trace silt, medium dense, moist (Fill) Contains concrete and wood debris	
5				GP		Brown GRAVEL with some sand, medium dense, moist	
10						Test Pit terminated at 9'	
15							

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-7**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 13.5 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s)	Compaction Method Bucket
Test Pit Backfill: Cuttings	Location 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		6" topsoil	
				Fill		Brown GRAVEL with some sand and trace silt, loose to medium dense, moist (Fill)	
	5					Becomes black, moist to wet	
	10			GM		Brown silty sandy GRAVEL, medium dense, moist	
	13.5					Test Pit terminated at 13.5'	
15							

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-8**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass, Moss
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 6.5 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s)	Compaction Method Bucket
Test Pit Backfill: Cuttings	Location 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		12" topsoil	
				Fill		Brown silty sandy GRAVEL, medium dense, moist (Fill) Concrete debris	
5				GM		Reddish brown silty sandy GRAVEL, medium dense, moist	
						Test Pit terminated at 6.5'	
10							
15							

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-9**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass, Moss
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 6 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s): Grab	Compaction Method: Bucket
Test Pit Backfill: Cuttings	Location: 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		18" topsoil	
				GP-GM		Brown GRAVEL with some sand and silt, medium dense, moist	
	5			GP		Gray GRAVEL with some sand and trace silt, medium dense, moist Contains sand lenses	2% moisture
						Test Pit terminated at 6'	
10							
15							

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-10**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 5 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s): Grab	Compaction Method: Bucket
Test Pit Backfill: Cuttings	Location: 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		8" topsoil	
				Fill		Brown GRAVEL with some sand and trace silt, medium dense, moist (Fill)	
				GP		Gray GRAVEL with some sand and trace silt, medium dense, moist	
	5					Test Pit terminated at 5'	3% moisture
	10						
	15						

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Test Pit No.: **TP-11**

Sheet 1 of 1

Date(s) Excavated: 2/3/2022	Logged By ELW	Surface Conditions: Grass
Excavation Method: Test Pit	Bucket Size: N/A	Total Depth of Excavation: 6.5 feet bgs
Excavator Type: Tracked Excavator	Excavating Contractor: Client Provided	Approximate Surface Elevation: N/A
Groundwater Level: Not Encountered	Sampling Method(s): Grab	Compaction Method: Bucket
Test Pit Backfill: Cuttings	Location: 8322 Steilacoom Road Southeast, Lacey, Washington	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
0				TPSL		12" gravelly topsoil	
				GW		Reddish brown GRAVEL with trace sand and silt, medium dense, moist	5% moisture, 1% fines
				GP		Gray sandy GRAVEL with trace silt, medium dense, moist	4% moisture
						Test Pit terminated at 6.5'	

Project Name: **Ostroms Plat**

Project Number: **2022-007-2**

Client: **D.R. Horton**



Key to Logs
Sheet 1 of 1

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
1	2	3	4	5	6	7	8

COLUMN DESCRIPTIONS

- 1** Elevation (feet): Elevation (MSL, feet).
- 2** Depth (feet): Depth in feet below the ground surface.
- 3** Sample Type: Type of soil sample collected at the depth interval shown.
- 4** Sample Number: Sample identification number.
- 5** USCS Symbol: USCS symbol of the subsurface material.
- 6** Graphic Log: Graphic depiction of the subsurface material encountered.
- 7** MATERIAL DESCRIPTION: Description of material encountered. May include consistency, moisture, color, and other descriptive text.
- 8** REMARKS AND OTHER TESTS: Comments and observations regarding drilling or sampling made by driller or field personnel.

FIELD AND LABORATORY TEST ABBREVIATIONS

- CHEM: Chemical tests to assess corrosivity
- COMP: Compaction test
- CONS: One-dimensional consolidation test
- LL: Liquid Limit, percent
- PI: Plasticity Index, percent
- SA: Sieve analysis (percent passing No. 200 Sieve)
- UC: Unconfined compressive strength test, Qu, in ksf
- WA: Wash sieve (percent passing No. 200 Sieve)

MATERIAL GRAPHIC SYMBOLS

- AF
- Silty GRAVEL (GM)
- Poorly graded GRAVEL (GP)
- Poorly graded GRAVEL with Silt (GP-GM)
- Well graded GRAVEL (GW)
- Silty SAND (SM)
- Topsoil

TYPICAL SAMPLER GRAPHIC SYMBOLS

- Auger sampler
- Bulk Sample
- 3-inch-OD California w/ brass rings
- CME Sampler
- Grab Sample
- 2.5-inch-OD Modified California w/ brass liners

- Pitcher Sample
- 2-inch-OD unlined split spoon (SPT)
- Shelby Tube (Thin-walled, fixed head)

OTHER GRAPHIC SYMBOLS

- Water level (at time of drilling, ATD)
- Water level (after waiting)
- Minor change in material properties within a stratum
- Inferred/gradational contact between strata
- Queried contact between strata

GENERAL NOTES

- 1: Soil classifications are based on the Unified Soil Classification System. Descriptions and stratum lines are interpretive, and actual lithologic changes may be gradual. Field descriptions may have been modified to reflect results of lab tests.
- 2: Descriptions on these logs apply only at the specific boring locations and at the time the borings were advanced. They are not warranted to be representative of subsurface conditions at other locations or times.

GRAIN SIZE ANALYSIS
ASTM D421, D422, D1140, D2487, D6913

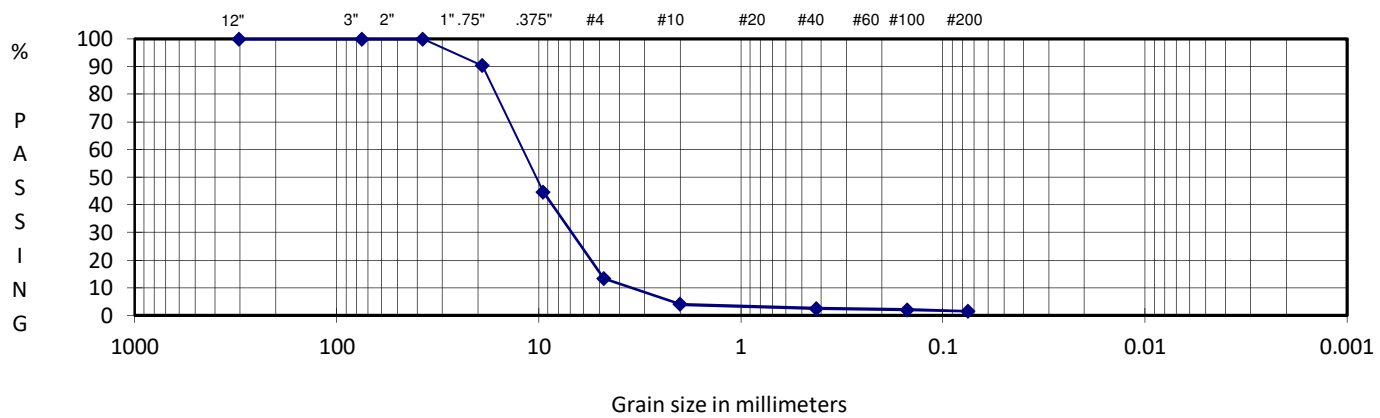
PROJECT TITLE	Ostroms Plat	SAMPLE ID/TYPE	TP-1
PROJECT NO.	2022-007-1	SAMPLE DEPTH	2.5'
TECH/TEST DATE	CM/EW 2/10/2022	DATE RECEIVED	2/4/2022

WATER CONTENT (Delivered Moisture)		Total Weight Of Sample Used For Sieve Corrected For Hygroscopic Moisture	
Wt Wet Soil & Tare (gm)	(w1) 848.8	Weight Of Sample (gm)	825.4
Wt Dry Soil & Tare (gm)	(w2) 825.4	Tare Weight (gm)	133.3
Weight of Tare (gm)	(w3) 133.3	(w6) Total Dry Weight (gm)	692.1

Weight of Water (gm)	(w4=w1-w2) 23.4	SIEVE ANALYSIS	
Weight of Dry Soil (gm)	(w5=w2-w3) 692.1	Cumulative	
Moisture Content (%)	(w4/w5)*100 3	Wt Ret	(Wt-Tare)
		+Tare	{(wt ret/w6)*100}
			% PASS
			(100-%ret)

% COBBLES	0.0
% C GRAVEL	9.6
% F GRAVEL	77.2
% C SAND	9.3
% M SAND	1.4
% F SAND	1.1
% FINES	1.4
% TOTAL	100.0
D10 (mm)	3.4
D30 (mm)	6.9
D60 (mm)	13
Cu	3.8
Cc	1.1

Sieve Size	Wt Ret +Tare	(Wt-Tare)	{(wt ret/w6)*100}	% PASS (100-%ret)	Material
12.0"	133.3	0.00	0.00	100.00	cobbles
3.0"	133.3	0.00	0.00	100.00	coarse gravel
2.5"					coarse gravel
2.0"					coarse gravel
1.5"	133.3	0.00	0.00	100.00	coarse gravel
1.0"					coarse gravel
0.75"	200.0	66.70	9.64	90.36	fine gravel
0.50"					fine gravel
0.375"	516.2	382.90	55.32	44.68	fine gravel
#4	734.0	600.70	86.79	13.21	coarse sand
#10	798.1	664.80	96.06	3.94	medium sand
#20					medium sand
#40	807.9	674.60	97.47	2.53	fine sand
#60					fine sand
#100	811.6	678.30	98.01	1.99	fine sand
#200	815.6	682.30	98.58	1.42	finer
PAN	825.4	692.10	100.00	0.00	silt/clay



DESCRIPTION: GRAVEL with trace sand and silt
 USCS: GP

Prepared For:
 D.R. Horton

Reviewed By:
 ELW

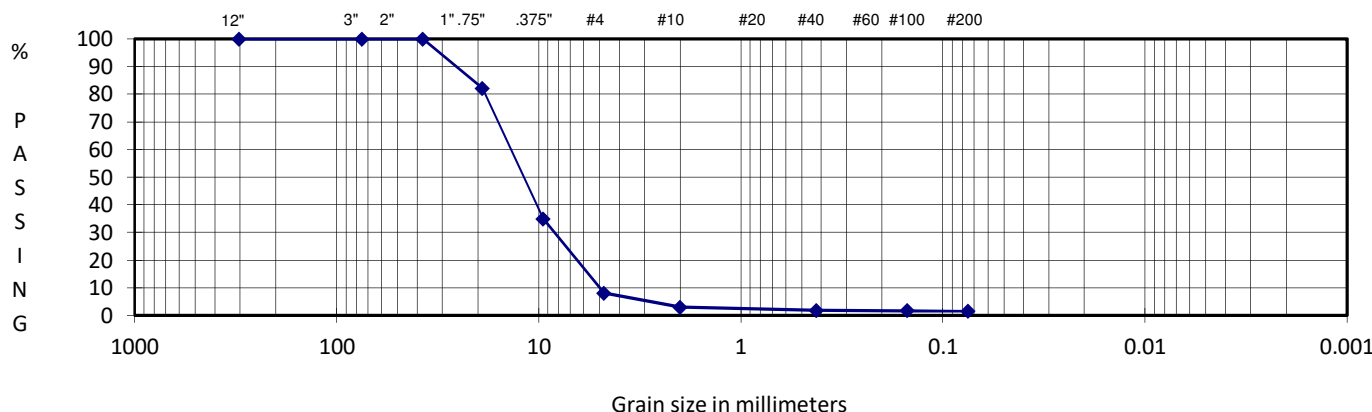


GRAIN SIZE ANALYSIS
ASTM D421, D422, D1140, D2487, D6913

PROJECT TITLE	Ostroms Plat	SAMPLE ID/TYPE	TP-1
PROJECT NO.	2022-007-1	SAMPLE DEPTH	6'
TECH/TEST DATE	CM/EW 2/10/2022	DATE RECEIVED	2/4/2022

WATER CONTENT (Delivered Moisture)		Total Weight Of Sample Used For Sieve Corrected For Hygroscopic Moisture	
Wt Wet Soil & Tare (gm)	(w1)	885.6	Weight Of Sample (gm)
Wt Dry Soil & Tare (gm)	(w2)	861.5	Tare Weight (gm)
Weight of Tare (gm)	(w3)	76.4	(w6) Total Dry Weight (gm)
Weight of Water (gm)	(w4=w1-w2)	24.1	
Weight of Dry Soil (gm)	(w5=w2-w3)	785.1	
Moisture Content (%)	(w4/w5)*100	3	

		Wt Ret +Tare	(Wt-Tare)	Cumulative (%Retained) (wt ret/w6)*100	% PASS (100-%ret)		
% COBBLES	0.0	12.0"	76.4	0.00	0.00	100.00	cobbles
% C GRAVEL	17.9	3.0"	76.4	0.00	0.00	100.00	coarse gravel
% F GRAVEL	74.1	2.5"					coarse gravel
% C SAND	5.1	2.0"					coarse gravel
% M SAND	1.1	1.5"	76.4	0.00	0.00	100.00	coarse gravel
% F SAND	0.3	1.0"					coarse gravel
% FINES	1.5	0.75"	217.2	140.80	17.93	82.07	fine gravel
% TOTAL	100.0	0.50"					fine gravel
D10 (mm)	4.9	0.375"	587.6	511.20	65.11	34.89	fine gravel
D30 (mm)	8.5	#4	798.8	722.40	92.01	7.99	coarse sand
D60 (mm)	15	#10	838.7	762.30	97.10	2.90	medium sand
Cu	3.1	#20					medium sand
Cc	1.0	#40	847.0	770.60	98.15	1.85	fine sand
		#60					fine sand
		#100	848.2	771.80	98.31	1.69	fine sand
		#200	849.5	773.10	98.47	1.53	finest
		PAN	861.5	785.10	100.00	0.00	silt/clay



DESCRIPTION: GRAVEL with trace sand and silt
 USCS: GP

Prepared For:
 D.R. Horton

Reviewed By:
 ELW



GRAIN SIZE ANALYSIS
ASTM D421, D422, D1140, D2487, D6913

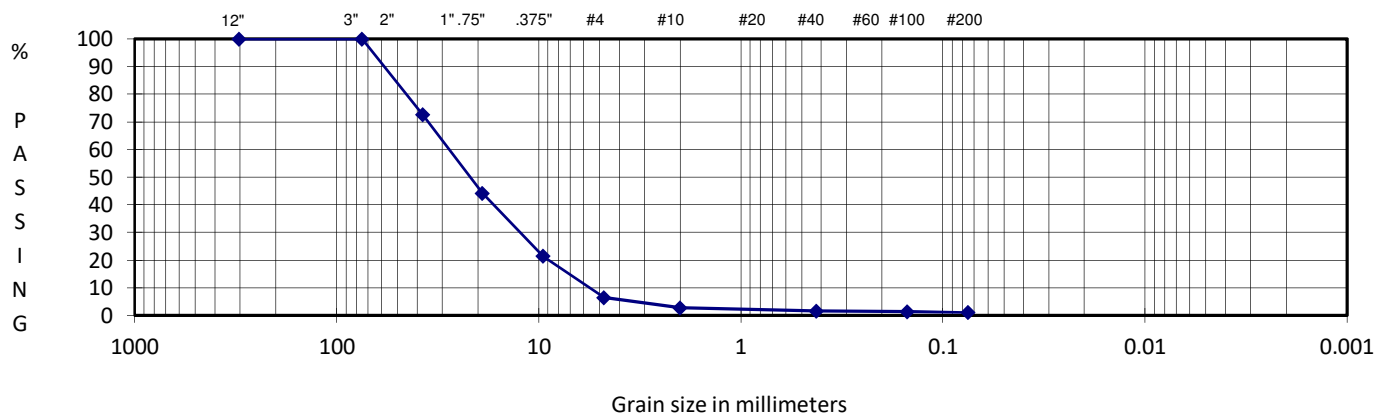
PROJECT TITLE	Ostroms Plat	SAMPLE ID/TYPE	TP-1
PROJECT NO.	2022-007-1	SAMPLE DEPTH	8.5'
TECH/TEST DATE	CM/EW 2/10/2022	DATE RECEIVED	2/4/2022

WATER CONTENT (Delivered Moisture)		Total Weight Of Sample Used For Sieve Corrected For Hygroscopic Moisture	
Wt Wet Soil & Tare (gm)	(w1) 780.6	Weight Of Sample (gm)	760.9
Wt Dry Soil & Tare (gm)	(w2) 760.9	Tare Weight (gm)	125.5
Weight of Tare (gm)	(w3) 125.5	(w6) Total Dry Weight (gm)	635.4

Weight of Water (gm)	(w4=w1-w2) 19.7	SIEVE ANALYSIS	
Weight of Dry Soil (gm)	(w5=w2-w3) 635.4	Cumulative	
Moisture Content (%)	(w4/w5)*100 3	Wt Ret +Tare	(Wt-Tare)
		{(wt ret/w6)*100}	% Retained
			% PASS (100-%ret)

% COBBLES	0.0
% C GRAVEL	55.9
% F GRAVEL	37.7
% C SAND	3.7
% M SAND	1.2
% F SAND	0.5
% FINES	1.1
% TOTAL	100.0
D10 (mm)	5.7
D30 (mm)	13
D60 (mm)	28
Cu	4.9
Cc	1.1

Sieve Size	Wt Ret +Tare	(Wt-Tare)	{(wt ret/w6)*100}	% PASS (100-%ret)	Material
12.0"	125.5	0.00	0.00	100.00	cobbles
3.0"	125.5	0.00	0.00	100.00	coarse gravel
2.5"					coarse gravel
2.0"					coarse gravel
1.5"	300.5	175.00	27.54	72.46	coarse gravel
1.0"					coarse gravel
0.75"	480.7	355.20	55.90	44.10	fine gravel
0.50"					fine gravel
0.375"	625.4	499.90	78.67	21.33	fine gravel
#4	720.2	594.70	93.59	6.41	coarse sand
#10	743.4	617.90	97.25	2.75	medium sand
#20					medium sand
#40	750.8	625.30	98.41	1.59	fine sand
#60					fine sand
#100	752.6	627.10	98.69	1.31	fine sand
#200	754.0	628.50	98.91	1.09	finest
PAN	760.9	635.40	100.00	0.00	silt/clay



DESCRIPTION: GRAVEL with trace sand and silt
 USCS: GW

Prepared For:
 D.R. Horton

Reviewed By:
 ELW



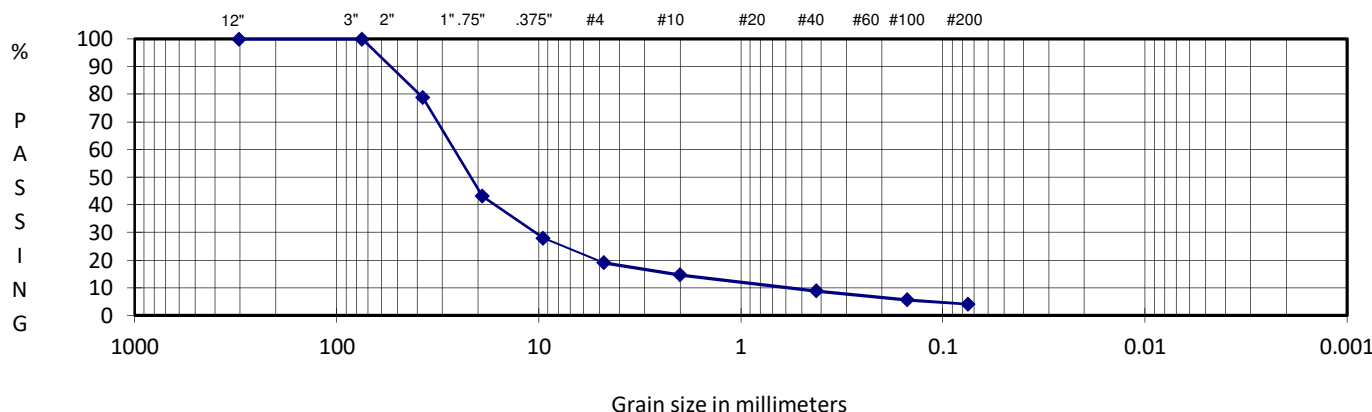
GRAIN SIZE ANALYSIS
ASTM D421, D422, D1140, D2487, D6913

PROJECT TITLE	Ostroms Plat	SAMPLE ID/TYPE	TP-5
PROJECT NO.	2022-007-1	SAMPLE DEPTH	2'
TECH/TEST DATE	CM/EW 2/10/2022	DATE RECEIVED	2/4/2022

WATER CONTENT (Delivered Moisture)		Total Weight Of Sample Used For Sieve Corrected For Hygroscopic Moisture	
Wt Wet Soil & Tare (gm)	(w1) 886.7	Weight Of Sample (gm)	819.1
Wt Dry Soil & Tare (gm)	(w2) 819.1	Tare Weight (gm)	16.0
Weight of Tare (gm)	(w3) 16.0	(w6) Total Dry Weight (gm)	803.1

Weight of Water (gm)	(w4=w1-w2) 67.6	SIEVE ANALYSIS	
Weight of Dry Soil (gm)	(w5=w2-w3) 803.1	Cumulative	
Moisture Content (%)	(w4/w5)*100 8	Wt Ret	(Wt-Tare)
		+Tare	{(wt ret/w6)*100}
			% PASS
			(100-%ret)

% COBBLES	0.0	12.0"	16.0	0.00	0.00	100.00	cobbles
% C GRAVEL	56.8	3.0"	16.0	0.00	0.00	100.00	coarse gravel
% F GRAVEL	24.1	2.5"					coarse gravel
% C SAND	4.4	2.0"					coarse gravel
% M SAND	5.8	1.5"	186.3	170.30	21.21	78.79	coarse gravel
% F SAND	4.9	1.0"					coarse gravel
% FINES	3.9	0.75"	472.4	456.40	56.83	43.17	fine gravel
% TOTAL	100.0	0.50"					fine gravel
D10 (mm)	0.6	0.375"	594.6	578.60	72.05	27.95	fine gravel
D30 (mm)	10	#4	666.0	650.00	80.94	19.06	coarse sand
D60 (mm)	27	#10	701.4	685.40	85.34	14.66	medium sand
Cu	45.0	#20					medium sand
Cc	6.2	#40	747.8	731.80	91.12	8.88	fine sand
		#60					fine sand
		#100	774.3	758.30	94.42	5.58	fine sand
		#200	787.4	771.40	96.05	3.95	finest
		PAN	819.1	803.10	100.00	0.00	silt/clay



DESCRIPTION: GRAVEL with some sand and trace silt
 USCS: GP

Prepared For:
 D.R. Horton

Reviewed By:
 ELW



GRAIN SIZE ANALYSIS
ASTM D421, D422, D1140, D2487, D6913

PROJECT TITLE	Ostroms Plat	SAMPLE ID/TYPE	TP-11
PROJECT NO.	2022-007-1	SAMPLE DEPTH	1.5'
TECH/TEST DATE	CM/EW 2/10/2022	DATE RECEIVED	2/4/2022

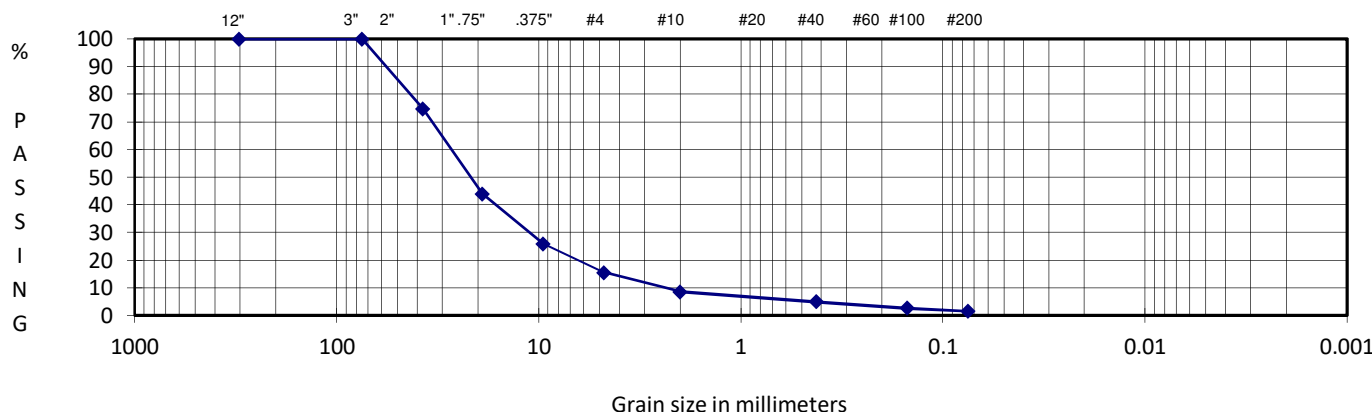
WATER CONTENT (Delivered Moisture)		Total Weight Of Sample Used For Sieve Corrected For Hygroscopic Moisture	
Wt Wet Soil & Tare (gm)	(w1) 785.0	Weight Of Sample (gm)	751.1
Wt Dry Soil & Tare (gm)	(w2) 751.1	Tare Weight (gm)	16.1
Weight of Tare (gm)	(w3) 16.1	(w6) Total Dry Weight (gm)	735.0

Weight of Water (gm)	(w4=w1-w2) 33.9	SIEVE ANALYSIS	
Weight of Dry Soil (gm)	(w5=w2-w3) 735.0	Cumulative	
Moisture Content (%)	(w4/w5)*100 5	Wt Ret +Tare	(Wt-Tare) (wt ret/w6)*100
		(%Retained)	% PASS (100-%ret)

% COBBLES	0.0
% C GRAVEL	56.0
% F GRAVEL	28.6
% C SAND	6.8
% M SAND	3.6
% F SAND	3.6
% FINES	1.4
% TOTAL	100.0

D10 (mm)	2.3
D30 (mm)	11
D60 (mm)	28
Cu	12.2
Cc	1.9

Sieve Size	Wt Ret +Tare	(Wt-Tare)	(%Retained)	% PASS (100-%ret)	Material
12.0"	16.1	0.00	0.00	100.00	cobbles
3.0"	16.1	0.00	0.00	100.00	coarse gravel
2.5"					coarse gravel
2.0"					coarse gravel
1.5"	202.1	186.00	25.31	74.69	coarse gravel
1.0"					coarse gravel
0.75"	427.9	411.80	56.03	43.97	fine gravel
0.50"					fine gravel
0.375"	560.7	544.60	74.10	25.90	fine gravel
#4	637.9	621.80	84.60	15.40	coarse sand
#10	688.2	672.10	91.44	8.56	medium sand
#20					medium sand
#40	714.6	698.50	95.03	4.97	fine sand
#60					fine sand
#100	731.8	715.70	97.37	2.63	fine sand
#200	740.8	724.70	98.60	1.40	finest
PAN	751.1	735.00	100.00	0.00	silt/clay



DESCRIPTION: GRAVEL with trace sand and silt
 USCS: GW

Prepared For:
 D.R. Horton

Reviewed By:
 ELW



Appendix 3: Design Calculations

Area Summary							
	Pre-Developed	Developed					Developed Total
	Pre-Developed Total	NW Basin	SW Basin	E Basin	SE Frontage	N Bypass	
Forest (A/B Flat)	32.08	0.40					0.40
Forest (A/B Moderate)				2.46		1.02	3.48
Forest (A/B Steep)						0.29	0.29
Lawn (A/B Flat)		3.59	1.82	6.22	0.26		11.89
Lawn (A/B Steep)		0.51			0.12	0.08	0.71
Roofs		2.76	0.62	3.41	0.00	0.00	6.79
Roads (Flat)		2.31	0.77	1.68	0.37		5.12
Alleys (Flat)		0.45		0.20			0.64
Access Road (Steep)		0.03	0.07	0.08	0.00		0.18
Sidewalk (Flat)		0.66	0.22	0.55	0.09		1.52
Driveway (Flat)		0.95	0.15	0.95			2.05
Bioretention		0.14	0.03	0.14	0.05		0.35
Pond		0.41	0.15	0.36			0.92
Total Area (ac)	32.08	12.22	3.82	16.04	0.88	1.39	34.36

Developed Area Summary					
Impervious					
NW Basin	SW Basin	E Basin	SE Frontage Basin	North Bypass Basin	Developed Total
7.72	2.00	7.36	0.51	0.00	17.59
63%	52%	46%	57%	0%	51%
Pervious					
4.50	1.82	8.68	0.38	1.39	16.77
37%	48%	54%	43%	100%	49%

Bioretention Stage and Storage

Bioretention Cell 1						
Interval (yrs)	Stage (ft)	L (ft)	W (ft)	Z (ft/ft)	V (cf)	V (ac-ft)
2	1.7408	60	58	3	7194.044	0.165153
5	1.7411	60	58	3	7195.49	0.165186
10	1.7413	60	58	3	7196.455	0.165208
25	1.7414	60	58	3	7196.937	0.165219
50	1.7415	60	58	3	7197.419	0.16523
100	1.7415	60	58	3	7197.419	0.16523

Bioretention Cell 2						
Interval (yrs)	Stage (ft)	L (ft)	W (ft)	Z (ft/ft)	V (cf)	V (ac-ft)
2	1.4645	42	5	3	647.6482	0.014868
5	1.4680	42	5	3	650.1013	0.014924
10	1.4695	42	5	3	651.1541	0.014948
25	1.4709	42	5	3	652.1374	0.014971
50	1.4717	42	5	3	652.6997	0.014984
100	1.4723	42	5	3	653.1216	0.014994

Bioretention Cell 3						
Interval (yrs)	Stage (ft)	L (ft)	W (ft)	Z (ft/ft)	V (cf)	V (ac-ft)
2	1.7357	50	50	3	5305.795	0.121804
5	1.7359	50	50	3	5306.525	0.121821
10	1.736	50	50	3	5306.89	0.121829
25	1.7361	50	50	3	5307.255	0.121838
50	1.7361	50	50	3	5307.255	0.121838
100	1.7361	50	50	3	5307.255	0.121838

Bioretention Cell 4						
Interval (yrs)	Stage (ft)	L (ft)	W (ft)	Z (ft/ft)	V (cf)	V (ac-ft)
2	1.5528	30	25	3	1607.375	0.0369
5	1.6068	30	25	3	1680.879	0.038588
10	1.6370	30	25	3	1722.553	0.039544
25	1.6707	30	25	3	1769.539	0.040623
50	1.6935	30	25	3	1801.618	0.041359
100	1.7145	30	25	3	1831.372	0.042043

Emergency Overflow Structure - Bioretention 1

$$Q_{100} = 9.739 \frac{D}{12} H^{\frac{3}{2}}$$

(equation from Volume II, Figure 3.28. Riser Inflow Curves of the Thurston County 2022 DDECM)

D = **72** Diameter of Discharge Riser (inches)
H = **0.33'** Flow Depth (ft)
Q = 11.08 cfs (100 Year Design Peak Inflow to NW Bioretention - 9.26 cfs)

Emergency Overflow Structure - Bioretention 2

$$Q_{100} = 9.739 \frac{D}{12} H^{\frac{3}{2}}$$

(equation from Volume II, Figure 3.28. Riser Inflow Curves of the Thurston County 2022 DDECM)

D = **18** Diameter of Discharge Riser (inches)
H = **0.33'** Flow Depth (ft)
Q = **2.77** cfs (100 Year Design Peak Inflow to SW Bioretention - 2.35 cfs)

Emergency Overflow Structure - Bioretention 3

$$Q_{100} = 9.739 \frac{D}{12} H^{\frac{3}{2}}$$

(equation from Volume II, Figure 3.28. Riser Inflow Curves of the Thurston County 2022 DDECM)

D = **60** Diameter of Discharge Riser (inches)
H = **0.33'** Flow Depth (ft)
Q = 9.23 cfs (100 Year Design Peak Inflow to E Bioretention - 9.13 cfs)

Emergency Overflow Spillway - NW Pond

Broad Crested Weir

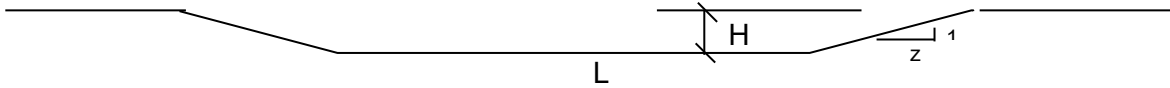


For this weir $Q_{100} = C\sqrt{2g}(2/3LH^{3/2} + 8/15Tan\theta H^{5/2})$

- C = **0.6** discharge coefficient
- L = **16.00'** (6' Minimum)
- z = **3** side slope
- H = **0.33'** flow depth
- Q = 10.22 cfs (100 Year Design Peak Inflow NW basin 9.86 cfs)
- V = 1.82 fps

Emergency Overflow Spillway - NE Pond

Broad Crested Weir



For this weir $Q_{100} = C\sqrt{2g}(2/3LH^{3/2} + 8/15Tan\theta H^{5/2})$

- C = **0.6** discharge coefficient
- L = **16.00'** (6' Minimum)
- z = **3** side slope
- H = **0.33'** flow depth
- Q = 10.22 cfs (100 Year Design Peak Inflow E Basin 9.91 cfs)
- V = 1.82 fps

WWHM2012

PROJECT REPORT

Site Analysis

General Model Information

Project Name: 21-100 Site Analysis
Site Name:
Site Address:
City:
Report Date: 6/13/2023
Gage: Fairgrounds (Kaiser)
Data Start: 1955/10/01
Data End: 2011/09/30
Timestep: 15 Minute
Precip Scale: 1.000
Version Date: 2019/09/13
Version: 4.2.17

POC Thresholds

Low Flow Threshold for POC1:	50 Percent of the 2 Year
High Flow Threshold for POC1:	50 Year

Landuse Basin Data

Predeveloped Land Use

Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use A B, Forest, Flat	acre 32.08
Pervious Total	32.08
Impervious Land Use	acre
Impervious Total	0
Basin Total	32.08

Element Flows To:		
Surface	Interflow	Groundwater

Mitigated Land Use

Northwest

Bypass: No

GroundWater: No

Pervious Land Use	acre
A B, Forest, Flat	0.4
A B, Pasture, Flat	3.59
A B, Pasture, Steep	0.15
A B IMP INF FLAT	0.14

PORTRION OF STEEP PASTURE IN NW BASIN GOING TO BIORETENTION CELL

Pervious Total 4.28

AREA ACCOUNTING FOR BIORETENTION CELL

Impervious Land Use	acre
ROADS FLAT	2.76
ROOF TOPS FLAT	2.76
DRIVEWAYS FLAT	0.95
SIDEWALKS FLAT	0.66

INCLUDES ROADS & ALLEYS

Impervious Total 7.13

Basin Total 11.41

NOTE: POND AREA, ACCESS ROAD AREA, AND TRIBUTARY LANDSCAPE AREAS ARE INCLUDED IN NW TRIB TO POND BASIN. SEE PAGE 11 OF THIS REPORT.

Element Flows To:

Surface	Interflow	Groundwater
Surface retention 1	Surface retention 1	

Southwest

Bypass: No

GroundWater: No

Pervious Land Use
A B, Pasture, Flat 1.53
A B IMP INF FLAT 0.03

**PORTION OF FLAT
PASTURE IN NW BASIN
GOING TO BIORETENTION
CELL**

**AREA ACCOUNTING FOR
BIORETENTION CELL**

Pervious Total 1.56

Impervious Land Use
ROADS FLAT 0.77
ROOF TOPS FLAT 0.62
DRIVEWAYS FLAT 0.15
SIDEWALKS FLAT 0.22

**INCLUDES ROADS &
ALLEYS**

Impervious Total 1.76

Basin Total 3.32

**NOTE: POND AREA, ACCESS ROAD
AREA, AND TRIBUTARY LANDSCAPE
AREAS ARE INCLUDED IN SW TRIB TO
POND BASIN. SEE PAGE 9 OF THIS
REPORT.**

Element Flows To:

Surface Interflow Groundwater
Surface retention 2 Surface retention 2

East

Bypass: No

GroundWater: No

Pervious Land Use acre
A B, Pasture, Flat 6.22
A B IMP INF FLAT 0.14

AREA ACCOUNTING FOR BIORETENTION CELL

Pervious Total 6.36

Impervious Land Use acre
ROADS FLAT 1.88
ROOF TOPS FLAT 3.41
DRIVEWAYS FLAT 0.95
SIDEWALKS FLAT 0.55

INCLUDES ROADS & ALLEYS

Impervious Total 6.79

NOTE: POND AREA, ACCESS ROAD AREA, AND TRIBUTARY FOREST AREAS ARE INCLUDED IN E TRIB TO POND BASIN. SEE PAGE 10 OF THIS REPORT.

Basin Total 13.15

Element Flows To:

Surface Interflow Groundwater
Surface retention 3 Surface retention 3

Southeast Frontage

Bypass: No

GroundWater: No

Pervious Land Use	acre
A B, Pasture, Flat	0.26
A B, Pasture, Steep	0.12
A B IMP INF FLAT	0.05

AREA ACCOUNTING FOR
BIORETENTION CELL



Pervious Total 0.43

Impervious Land Use	acre
ROADS FLAT	0.37
SIDEWALKS FLAT	0.09

Impervious Total 0.46

Basin Total 0.89

Element Flows To:

Surface	Interflow	Groundwater
Surface retention 4	Surface retention 4	

North Bypass

Bypass: Yes

GroundWater: No

Pervious Land Use acre

A B, Forest, Mod 1.02

A B, Forest, Steep 0.29

A B, Pasture, Steep 0.08

Pervious Total 1.39

Impervious Land Use acre

Impervious Total 0

Basin Total 1.39

AREA NOT GOING TO ANY
FLOW CONTROL FACILITY



Element Flows To:
Surface

Interflow

Groundwater

SW Trib to Pond

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
A B, Pasture, Flat	0.29
Pervious Total	0.29
Impervious Land Use	acre
ROADS STEEP	0.07
POND	0.15
Impervious Total	0.22
Basin Total	0.51

AREA IN SOUTHWEST
BASIN GOING DIRECTLY
TO INFILTRATION POND



Element Flows To:		
Surface	Interflow	Groundwater
Trapezoidal Pond 2	Trapezoidal Pond 2	

East trib to Pond

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
A B, Forest, Mod	2.46
Pervious Total	2.46
Impervious Land Use	acre
ROADS STEEP	0.08
POND	0.36
Impervious Total	0.44
Basin Total	2.9

 **AREA IN EAST BASIN
GOING DIRECTLY TO
INFILTRATION POND**

Element Flows To:

Surface	Interflow	Groundwater
Trapezoidal Pond 3	Trapezoidal Pond 3	

NW trib to Pond

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
A B, Pasture, Steep	0.36
Pervious Total	0.36
Impervious Land Use	acre
ROADS STEEP	0.03
POND	0.41
Impervious Total	0.44
Basin Total	0.8

 **AREA IN NORTHWEST
BASIN GOING DIRECTLY
TO INFILTRATION POND**

Element Flows To:		
Surface	Interflow	Groundwater
Trapezoidal Pond 1	Trapezoidal Pond 1	

Routing Elements
Predeveloped Routing

Mitigated Routing

Bioretention 1

Bottom Length:	60.00 ft.
Bottom Width:	58.00 ft.
Material thickness of first layer:	0.25
Material type for first layer:	SMMWW 12 in/hr
Material thickness of second layer:	1.5
Material type for second layer:	SMMWW 12 in/hr
Material thickness of third layer:	0
Material type for third layer:	GRAVEL
Infiltration On	
Infiltration rate:	10
Infiltration safety factor:	1
Wetted surface area On	
Total Volume Infiltrated (ac-ft.):	1409.15
Total Volume Through Riser (ac-ft.):	51.275
Total Volume Through Facility (ac-ft.):	1460.425
Percent Infiltrated:	96.49
Total Precip Applied to Facility:	22.452
Total Evap From Facility:	7.62
Underdrain not used	
Discharge Structure	
Riser Height:	1.5 ft.
Riser Diameter:	48 in.
Element Flows To:	
Outlet 1	Outlet 2
Trapezoidal Pond 1	

Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
212.00	0.1109	0.0000	0.0000	0.0000
212.04	0.1105	0.0015	0.0000	0.0000
212.08	0.1097	0.0030	0.0000	0.0000
212.12	0.1089	0.0046	0.0000	0.0000
212.16	0.1081	0.0061	0.0000	0.0000
212.21	0.1074	0.0077	0.0000	0.0000
212.25	0.1066	0.0093	0.0017	0.0017
212.29	0.1058	0.0109	0.0027	0.0027
212.33	0.1051	0.0125	0.0039	0.0039
212.37	0.1043	0.0141	0.0054	0.0054
212.41	0.1035	0.0157	0.0073	0.0073
212.45	0.1028	0.0173	0.0096	0.0096
212.49	0.1020	0.0190	0.0122	0.0122
212.54	0.1012	0.0207	0.0153	0.0153
212.58	0.1005	0.0223	0.0189	0.0189
212.62	0.0997	0.0240	0.0229	0.0229
212.66	0.0990	0.0257	0.0275	0.0275
212.70	0.0983	0.0275	0.0294	0.0294
212.74	0.0975	0.0292	0.0329	0.0329
212.78	0.0968	0.0309	0.0386	0.0386
212.82	0.0960	0.0327	0.0450	0.0450
212.87	0.0953	0.0345	0.0520	0.0520
212.91	0.0946	0.0363	0.0597	0.0597
212.95	0.0938	0.0381	0.0671	0.0671
212.99	0.0931	0.0399	0.0686	0.0686

213.03	0.0924	0.0417	0.0777	0.0777
213.07	0.0917	0.0436	0.0877	0.0877
213.11	0.0910	0.0454	0.0984	0.0984
213.15	0.0903	0.0473	0.1099	0.1099
213.20	0.0895	0.0492	0.1223	0.1223
213.24	0.0888	0.0511	0.1290	0.1290
213.28	0.0881	0.0530	0.1366	0.1366
213.32	0.0874	0.0549	0.1509	0.1509
213.36	0.0867	0.0569	0.1661	0.1661
213.40	0.0860	0.0588	0.1823	0.1823
213.44	0.0853	0.0608	0.1995	0.1995
213.48	0.0846	0.0628	0.2178	0.2178
213.52	0.0840	0.0648	0.2206	0.2206
213.57	0.0833	0.0668	0.2388	0.2388
213.61	0.0826	0.0688	0.2592	0.2592
213.65	0.0819	0.0709	0.2808	0.2808
213.69	0.0812	0.0729	0.3034	0.3034
213.73	0.0806	0.0750	0.3268	0.3268
213.75	0.0799	0.0760	0.7266	0.7266

Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	To Amended(cfs)	Infiltr(cfs)
1.7500	0.1109	0.0760	0.0000	0.2886	0.0080
1.7912	0.1117	0.0806	0.0000	0.2886	0.0160
1.8324	0.1124	0.0852	0.0000	0.2952	0.0240
1.8736	0.1132	0.0898	0.0000	0.3019	0.0320
1.9148	0.1140	0.0945	0.0000	0.3085	0.0401
1.9560	0.1148	0.0992	0.0000	0.3151	0.0482
1.9973	0.1156	0.1040	0.0000	0.3218	0.0564
2.0385	0.1165	0.1088	0.0000	0.3284	0.0646
2.0797	0.1173	0.1136	0.0000	0.3351	0.0727
2.1209	0.1181	0.1184	0.0000	0.3417	0.0810
2.1621	0.1189	0.1233	0.0000	0.3483	0.0892
2.2033	0.1197	0.1282	0.0000	0.3550	0.0975
2.2445	0.1205	0.1332	0.0000	0.3616	0.1058
2.2857	0.1214	0.1382	0.0000	0.3683	0.1142
2.3269	0.1222	0.1432	0.0000	0.3749	0.1225
2.3681	0.1230	0.1482	0.0000	0.3815	0.1309
2.4093	0.1238	0.1533	0.0000	0.3882	0.1393
2.4505	0.1247	0.1584	0.0000	0.3948	0.1478
2.4918	0.1255	0.1636	0.0000	0.4015	0.1563
2.5330	0.1264	0.1688	0.0000	0.4081	0.1648
2.5742	0.1272	0.1740	0.0000	0.4147	0.1733
2.6154	0.1281	0.1793	0.0000	0.4214	0.1819
2.6566	0.1289	0.1846	0.0000	0.4280	0.1905
2.6978	0.1298	0.1899	0.0000	0.4346	0.1991
2.7390	0.1306	0.1953	0.0000	0.4413	0.2077
2.7802	0.1315	0.2007	0.0000	0.4479	0.2164
2.8214	0.1323	0.2061	0.0000	0.4546	0.2251
2.8626	0.1332	0.2116	0.0000	0.4612	0.2339
2.9038	0.1341	0.2171	0.0000	0.4678	0.2426
2.9451	0.1349	0.2226	0.0000	0.4745	0.2514
2.9863	0.1358	0.2282	0.0000	0.4811	0.2602
3.0275	0.1367	0.2338	0.0000	0.4878	0.2691
3.0687	0.1375	0.2395	0.0000	0.4944	0.2779
3.1099	0.1384	0.2452	0.0000	0.5010	0.2868
3.1511	0.1393	0.2509	0.0000	0.5077	0.2958
3.1923	0.1402	0.2566	0.0000	0.5143	0.3047

3.2335	0.1411	0.2624	0.0000	0.5210	0.3137
3.2747	0.1420	0.2683	0.0000	0.5276	0.3227
3.3159	0.1429	0.2741	0.0000	0.5342	0.3318
3.3571	0.1438	0.2800	0.0000	0.5409	0.3409
3.3984	0.1447	0.2860	0.0000	0.5475	0.3500
3.4396	0.1456	0.2920	0.0000	0.5542	0.3591
3.4808	0.1465	0.2980	0.0000	0.5608	0.3683
3.5220	0.1474	0.3040	0.0000	0.5674	0.3774
3.5632	0.1483	0.3101	7.4148	0.5741	0.3867
3.6044	0.1492	0.3163	8.9151	0.5807	0.3959
3.6456	0.1501	0.3224	10.499	0.5873	0.4052
3.6868	0.1510	0.3286	12.160	0.5940	0.4145
3.7280	0.1520	0.3349	13.890	0.6006	0.4194
3.7500	0.1525	0.3382	15.682	0.6042	0.0000

Surface retention 1

Element Flows To:

Outlet 1

Outlet 2

Trapezoidal Pond 1

Bioretention 1

Trapezoidal Pond 1

Bottom Length: 100.00 ft.
 Bottom Width: 76.54 ft.
 Depth: 6 ft.
 Volume at riser head: 1.3650 acre-feet.
 Infiltration On
 Infiltration rate: 10
 Infiltration safety factor: 1
 Total Volume Infiltrated (ac-ft.): 140.694
 Total Volume Through Riser (ac-ft.): 0
 Total Volume Through Facility (ac-ft.): 140.694
Percent Infiltrated: **100**
 Total Precip Applied to Facility: 0
 Total Evap From Facility: 0
 Side slope 1: 7.3 To 1
 Side slope 2: 3 To 1
 Side slope 3: 3 To 1
 Side slope 4: 3 To 1
 Discharge Structure
 Riser Height: 5 ft.
 Riser Diameter: 12 in.
 Element Flows To:
 Outlet 1 Outlet 2

Pond Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
209.00	0.175	0.000	0.000	0.000
209.07	0.178	0.011	0.000	1.771
209.13	0.180	0.023	0.000	1.771
209.20	0.182	0.035	0.000	1.771
209.27	0.184	0.048	0.000	1.771
209.33	0.187	0.060	0.000	1.771
209.40	0.189	0.073	0.000	1.771
209.47	0.192	0.085	0.000	1.771
209.53	0.194	0.098	0.000	1.771
209.60	0.196	0.111	0.000	1.771
209.67	0.199	0.124	0.000	1.771
209.73	0.201	0.138	0.000	1.771
209.80	0.204	0.151	0.000	1.771
209.87	0.206	0.165	0.000	1.771
209.93	0.208	0.179	0.000	1.771
210.00	0.211	0.193	0.000	1.771
210.07	0.213	0.207	0.000	1.771
210.13	0.216	0.221	0.000	1.771
210.20	0.218	0.236	0.000	1.771
210.27	0.221	0.251	0.000	1.771
210.33	0.223	0.265	0.000	1.771
210.40	0.226	0.280	0.000	1.771
210.47	0.228	0.296	0.000	1.771
210.53	0.231	0.311	0.000	1.771
210.60	0.234	0.326	0.000	1.771
210.67	0.236	0.342	0.000	1.771
210.73	0.239	0.358	0.000	1.771
210.80	0.241	0.374	0.000	1.771
210.87	0.244	0.390	0.000	1.771

210.93	0.247	0.407	0.000	1.771
211.00	0.249	0.423	0.000	1.771
211.07	0.252	0.440	0.000	1.771
211.13	0.255	0.457	0.000	1.771
211.20	0.257	0.474	0.000	1.771
211.27	0.260	0.491	0.000	1.771
211.33	0.263	0.509	0.000	1.771
211.40	0.265	0.526	0.000	1.771
211.47	0.268	0.544	0.000	1.771
211.53	0.271	0.562	0.000	1.771
211.60	0.274	0.580	0.000	1.771
211.67	0.277	0.599	0.000	1.771
211.73	0.279	0.617	0.000	1.771
211.80	0.282	0.636	0.000	1.771
211.87	0.285	0.655	0.000	1.771
211.93	0.288	0.674	0.000	1.771
212.00	0.291	0.693	0.000	1.771
212.07	0.293	0.713	0.000	1.771
212.13	0.296	0.732	0.000	1.771
212.20	0.299	0.752	0.000	1.771
212.27	0.302	0.772	0.000	1.771
212.33	0.305	0.793	0.000	1.771
212.40	0.308	0.813	0.000	1.771
212.47	0.311	0.834	0.000	1.771
212.53	0.314	0.855	0.000	1.771
212.60	0.317	0.876	0.000	1.771
212.67	0.320	0.897	0.000	1.771
212.73	0.323	0.918	0.000	1.771
212.80	0.326	0.940	0.000	1.771
212.87	0.329	0.962	0.000	1.771
212.93	0.332	0.984	0.000	1.771
213.00	0.335	1.006	0.000	1.771
213.07	0.338	1.029	0.000	1.771
213.13	0.341	1.051	0.000	1.771
213.20	0.344	1.074	0.000	1.771
213.27	0.347	1.097	0.000	1.771
213.33	0.350	1.120	0.000	1.771
213.40	0.353	1.144	0.000	1.771
213.47	0.356	1.168	0.000	1.771
213.53	0.359	1.191	0.000	1.771
213.60	0.363	1.216	0.000	1.771
213.67	0.366	1.240	0.000	1.771
213.73	0.369	1.264	0.000	1.771
213.80	0.372	1.289	0.000	1.771
213.87	0.375	1.314	0.000	1.771
213.93	0.378	1.339	0.000	1.771
214.00	0.382	1.365	0.000	1.771
214.07	0.385	1.390	0.182	1.771
214.13	0.388	1.416	0.509	1.771
214.20	0.391	1.442	0.907	1.771
214.27	0.395	1.468	1.318	1.771
214.33	0.398	1.495	1.683	1.771
214.40	0.401	1.521	1.960	1.771
214.47	0.405	1.548	2.138	1.771
214.53	0.408	1.575	2.300	1.771
214.60	0.411	1.603	2.439	1.771
214.67	0.415	1.630	2.571	1.771
214.73	0.418	1.658	2.697	1.771

214.80	0.421	1.686	2.817	1.771
214.87	0.425	1.714	2.932	1.771
214.93	0.428	1.743	3.042	1.771
215.00	0.431	1.771	3.149	1.771
215.07	0.435	1.800	3.252	1.771

Bioretention 2

Bottom Length:	42.00 ft.
Bottom Width:	5.00 ft.
Material thickness of first layer:	0.25
Material type for first layer:	SMMWW 12 in/hr
Material thickness of second layer:	1.5
Material type for second layer:	SMMWW 12 in/hr
Material thickness of third layer:	0
Material type for third layer:	GRAVEL
Infiltration On	
Infiltration rate:	10
Infiltration safety factor:	1
Wetted surface area On	
Total Volume Infiltrated (ac-ft.):	342.733
Total Volume Through Riser (ac-ft.):	18.176
Total Volume Through Facility (ac-ft.):	360.909
Percent Infiltrated:	94.96
Total Precip Applied to Facility:	4.689
Total Evap From Facility:	0.902
Underdrain not used	
Discharge Structure	
Riser Height:	1.5 ft.
Riser Diameter:	24 in.
Element Flows To:	
Outlet 1	Outlet 2
Trapezoidal Pond	2

Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
221.50	0.0187	0.0000	0.0000	0.0000
221.54	0.0185	0.0001	0.0000	0.0000
221.58	0.0181	0.0002	0.0000	0.0000
221.62	0.0177	0.0003	0.0000	0.0000
221.66	0.0174	0.0004	0.0000	0.0000
221.71	0.0170	0.0005	0.0000	0.0000
221.75	0.0166	0.0006	0.0000	0.0001
221.79	0.0162	0.0008	0.0000	0.0002
221.83	0.0159	0.0009	0.0000	0.0003
221.87	0.0155	0.0010	0.0000	0.0005
221.91	0.0152	0.0012	0.0000	0.0006
221.95	0.0148	0.0013	0.0000	0.0009
221.99	0.0144	0.0015	0.0000	0.0011
222.04	0.0141	0.0016	0.0000	0.0015
222.08	0.0137	0.0018	0.0000	0.0019
222.12	0.0134	0.0020	0.0000	0.0023
222.16	0.0130	0.0021	0.0000	0.0029
222.20	0.0127	0.0023	0.0000	0.0031
222.24	0.0124	0.0025	0.0000	0.0036
222.28	0.0120	0.0027	0.0000	0.0043
222.32	0.0117	0.0029	0.0000	0.0051
222.37	0.0114	0.0031	0.0000	0.0061
222.41	0.0110	0.0033	0.0000	0.0071
222.45	0.0107	0.0035	0.0000	0.0082
222.49	0.0104	0.0038	0.0000	0.0085
222.53	0.0101	0.0040	0.0000	0.0099
222.57	0.0098	0.0042	0.0000	0.0113

222.61	0.0094	0.0045	0.0000	0.0130
222.65	0.0091	0.0047	0.0000	0.0148
222.70	0.0088	0.0050	0.0000	0.0167
222.74	0.0085	0.0052	0.0000	0.0180
222.78	0.0082	0.0055	0.0000	0.0193
222.82	0.0079	0.0058	0.0000	0.0217
222.86	0.0076	0.0061	0.0000	0.0243
222.90	0.0073	0.0063	0.0000	0.0271
222.94	0.0070	0.0066	0.0000	0.0302
222.98	0.0068	0.0069	0.0000	0.0334
223.02	0.0065	0.0072	0.0000	0.0344
223.07	0.0062	0.0076	0.0000	0.0378
223.11	0.0059	0.0079	0.0000	0.0416
223.15	0.0056	0.0082	0.0000	0.0457
223.19	0.0054	0.0086	0.0000	0.0501
223.23	0.0051	0.0089	0.0000	0.0547
223.25	0.0048	0.0091	0.0000	0.1224

Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	To Amended(cfs)	Infiltr(cfs)
1.7500	0.0187	0.0091	0.0000	0.0174	0.0039
1.7912	0.0191	0.0098	0.0000	0.0174	0.0078
1.8324	0.0195	0.0106	0.0000	0.0178	0.0118
1.8736	0.0199	0.0114	0.0000	0.0182	0.0158
1.9148	0.0202	0.0123	0.0000	0.0186	0.0198
1.9560	0.0206	0.0131	0.0000	0.0190	0.0239
1.9973	0.0210	0.0140	0.0000	0.0194	0.0279
2.0385	0.0215	0.0149	0.0000	0.0198	0.0320
2.0797	0.0219	0.0157	0.0000	0.0202	0.0362
2.1209	0.0223	0.0167	0.0000	0.0206	0.0403
2.1621	0.0227	0.0176	0.0000	0.0210	0.0445
2.2033	0.0231	0.0185	0.0000	0.0214	0.0487
2.2445	0.0235	0.0195	0.0000	0.0218	0.0530
2.2857	0.0239	0.0205	0.0000	0.0222	0.0573
2.3269	0.0244	0.0215	0.0000	0.0226	0.0616
2.3681	0.0248	0.0225	0.0000	0.0230	0.0659
2.4093	0.0252	0.0235	0.0000	0.0234	0.0703
2.4505	0.0256	0.0245	0.0000	0.0238	0.0746
2.4918	0.0261	0.0256	0.0000	0.0242	0.0791
2.5330	0.0265	0.0267	0.0000	0.0246	0.0835
2.5742	0.0270	0.0278	0.0000	0.0250	0.0880
2.6154	0.0274	0.0289	0.0000	0.0254	0.0925
2.6566	0.0279	0.0301	0.0000	0.0258	0.0970
2.6978	0.0283	0.0312	0.0000	0.0262	0.1016
2.7390	0.0288	0.0324	0.0000	0.0266	0.1061
2.7802	0.0292	0.0336	0.0000	0.0270	0.1108
2.8214	0.0297	0.0348	0.0000	0.0274	0.1154
2.8626	0.0301	0.0360	0.0000	0.0278	0.1201
2.9038	0.0306	0.0373	0.0000	0.0282	0.1248
2.9451	0.0311	0.0386	0.0000	0.0286	0.1295
2.9863	0.0315	0.0398	0.0000	0.0290	0.1342
3.0275	0.0320	0.0411	0.0000	0.0294	0.1390
3.0687	0.0325	0.0425	0.0000	0.0298	0.1438
3.1099	0.0329	0.0438	0.0000	0.0302	0.1487
3.1511	0.0334	0.0452	0.0000	0.0306	0.1536
3.1923	0.0339	0.0466	0.0000	0.0310	0.1585
3.2335	0.0344	0.0480	0.0000	0.0314	0.1634
3.2747	0.0349	0.0494	0.0825	0.0318	0.1683

3.3159	0.0354	0.0509	0.3591	0.0322	0.1733
3.3571	0.0359	0.0523	0.7432	0.0326	0.1783
3.3984	0.0364	0.0538	1.2091	0.0330	0.1834
3.4396	0.0369	0.0553	1.7423	0.0334	0.1884
3.4808	0.0374	0.0569	2.3316	0.0338	0.1935
3.5220	0.0379	0.0584	2.9673	0.0342	0.1986
3.5632	0.0384	0.0600	3.6398	0.0346	0.2038
3.6044	0.0389	0.0616	4.3398	0.0350	0.2090
3.6456	0.0394	0.0632	5.0574	0.0354	0.2142
3.6868	0.0399	0.0648	5.7828	0.0358	0.2194
3.7280	0.0404	0.0665	6.5061	0.0362	0.2222
3.7500	0.0407	0.0674	7.2173	0.0365	0.0000

Surface retention 2

Element Flows To:

Outlet 1

Outlet 2

Trapezoidal Pond 2

Bioretention 2

Trapezoidal Pond 2

Bottom Length: 76.00 ft.
 Bottom Width: 35.00 ft.
 Depth: 4 ft.
 Volume at riser head: 0.2777 acre-feet.
 Infiltration On
 Infiltration rate: 10
 Infiltration safety factor: 1
 Total Volume Infiltrated (ac-ft.): 63.197
 Total Volume Through Riser (ac-ft.): 0
 Total Volume Through Facility (ac-ft.): 63.197
Percent Infiltrated: **100**
 Total Precip Applied to Facility: 0
 Total Evap From Facility: 0
 Side slope 1: 4.7 To 1
 Side slope 2: 3 To 1
 Side slope 3: 3 To 1
 Side slope 4: 3 To 1
 Discharge Structure
 Riser Height: 3 ft.
 Riser Diameter: 12 in.
 Element Flows To:
 Outlet 1 Outlet 2

Pond Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
220.00	0.061	0.000	0.000	0.000
220.04	0.061	0.002	0.000	0.615
220.09	0.062	0.005	0.000	0.615
220.13	0.063	0.008	0.000	0.615
220.18	0.064	0.011	0.000	0.615
220.22	0.065	0.014	0.000	0.615
220.27	0.066	0.016	0.000	0.615
220.31	0.066	0.019	0.000	0.615
220.36	0.067	0.022	0.000	0.615
220.40	0.068	0.025	0.000	0.615
220.44	0.069	0.029	0.000	0.615
220.49	0.070	0.032	0.000	0.615
220.53	0.071	0.035	0.000	0.615
220.58	0.072	0.038	0.000	0.615
220.62	0.072	0.041	0.000	0.615
220.67	0.073	0.044	0.000	0.615
220.71	0.074	0.048	0.000	0.615
220.76	0.075	0.051	0.000	0.615
220.80	0.076	0.054	0.000	0.615
220.84	0.077	0.058	0.000	0.615
220.89	0.078	0.061	0.000	0.615
220.93	0.079	0.065	0.000	0.615
220.98	0.079	0.068	0.000	0.615
221.02	0.080	0.072	0.000	0.615
221.07	0.081	0.076	0.000	0.615
221.11	0.082	0.079	0.000	0.615
221.16	0.083	0.083	0.000	0.615
221.20	0.084	0.087	0.000	0.615
221.24	0.085	0.090	0.000	0.615

221.29	0.086	0.094	0.000	0.615
221.33	0.087	0.098	0.000	0.615
221.38	0.088	0.102	0.000	0.615
221.42	0.089	0.106	0.000	0.615
221.47	0.090	0.110	0.000	0.615
221.51	0.091	0.114	0.000	0.615
221.56	0.092	0.118	0.000	0.615
221.60	0.093	0.122	0.000	0.615
221.64	0.094	0.126	0.000	0.615
221.69	0.094	0.130	0.000	0.615
221.73	0.095	0.135	0.000	0.615
221.78	0.096	0.139	0.000	0.615
221.82	0.097	0.143	0.000	0.615
221.87	0.098	0.148	0.000	0.615
221.91	0.099	0.152	0.000	0.615
221.96	0.100	0.157	0.000	0.615
222.00	0.101	0.161	0.000	0.615
222.04	0.102	0.166	0.000	0.615
222.09	0.103	0.170	0.000	0.615
222.13	0.104	0.175	0.000	0.615
222.18	0.105	0.179	0.000	0.615
222.22	0.106	0.184	0.000	0.615
222.27	0.107	0.189	0.000	0.615
222.31	0.108	0.194	0.000	0.615
222.36	0.110	0.199	0.000	0.615
222.40	0.111	0.204	0.000	0.615
222.44	0.112	0.209	0.000	0.615
222.49	0.113	0.214	0.000	0.615
222.53	0.114	0.219	0.000	0.615
222.58	0.115	0.224	0.000	0.615
222.62	0.116	0.229	0.000	0.615
222.67	0.117	0.234	0.000	0.615
222.71	0.118	0.239	0.000	0.615
222.76	0.119	0.245	0.000	0.615
222.80	0.120	0.250	0.000	0.615
222.84	0.121	0.255	0.000	0.615
222.89	0.122	0.261	0.000	0.615
222.93	0.123	0.266	0.000	0.615
222.98	0.124	0.272	0.000	0.615
223.02	0.125	0.277	0.035	0.615
223.07	0.127	0.283	0.182	0.615
223.11	0.128	0.289	0.389	0.615
223.16	0.129	0.294	0.637	0.615
223.20	0.130	0.300	0.907	0.615
223.24	0.131	0.306	1.183	0.615
223.29	0.132	0.312	1.447	0.615
223.33	0.133	0.318	1.683	0.615
223.38	0.134	0.324	1.879	0.615
223.42	0.136	0.330	2.029	0.615
223.47	0.137	0.336	2.138	0.615
223.51	0.138	0.342	2.251	0.615
223.56	0.139	0.348	2.347	0.615
223.60	0.140	0.354	2.439	0.615
223.64	0.141	0.360	2.528	0.615
223.69	0.142	0.367	2.614	0.615
223.73	0.144	0.373	2.697	0.615
223.78	0.145	0.380	2.777	0.615
223.82	0.146	0.386	2.856	0.615

223.87	0.147	0.393	2.932	0.615
223.91	0.148	0.399	3.006	0.615
223.96	0.149	0.406	3.078	0.615
224.00	0.151	0.412	3.149	0.615
224.04	0.152	0.419	3.218	0.615

Bioretention 3

Bottom Length: 155.00 ft.
 Bottom Width: 16.80 ft.
 Material thickness of first layer: 0.25
 Material type for first layer: SMMWW 12 in/hr
 Material thickness of second layer: 1.5
 Material type for second layer: SMMWW 12 in/hr
 Material thickness of third layer: 0
 Material type for third layer: GRAVEL
 Infiltration On
 Infiltration rate: 10
 Infiltration safety factor: 1
 Wetted surface area On
 Total Volume Infiltrated (ac-ft.): 1345.72
 Total Volume Through Riser (ac-ft.): 47.119
 Total Volume Through Facility (ac-ft.): 1392.839
Percent Infiltrated: 96.62
 Total Precip Applied to Facility: 21.349
 Total Evap From Facility: 6.494
 Underdrain not used
 Discharge Structure
 Riser Height: 1.5 ft.
 Riser Diameter: 48 in.
 Element Flows To:
 Outlet 1 Outlet 2
 Trapezoidal Pond 3

Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
0.0000	0.1037	0.0000	0.0000	0.0000
0.0412	0.1032	0.0011	0.0000	0.0000
0.0824	0.1021	0.0023	0.0000	0.0000
0.1236	0.1010	0.0035	0.0000	0.0000
0.1648	0.0999	0.0047	0.0000	0.0000
0.2060	0.0989	0.0059	0.0000	0.0000
0.2473	0.0978	0.0071	0.0013	0.0013
0.2885	0.0967	0.0083	0.0021	0.0021
0.3297	0.0956	0.0096	0.0031	0.0031
0.3709	0.0946	0.0109	0.0043	0.0043
0.4121	0.0935	0.0122	0.0059	0.0059
0.4533	0.0924	0.0135	0.0077	0.0077
0.4945	0.0914	0.0149	0.0099	0.0099
0.5357	0.0903	0.0162	0.0125	0.0125
0.5769	0.0892	0.0176	0.0155	0.0155
0.6181	0.0882	0.0190	0.0190	0.0190
0.6593	0.0871	0.0204	0.0229	0.0229
0.7005	0.0861	0.0218	0.0246	0.0246
0.7418	0.0850	0.0233	0.0277	0.0277
0.7830	0.0840	0.0248	0.0327	0.0327
0.8242	0.0830	0.0263	0.0383	0.0383
0.8654	0.0819	0.0278	0.0445	0.0445
0.9066	0.0809	0.0293	0.0513	0.0513
0.9478	0.0798	0.0309	0.0579	0.0579
0.9890	0.0788	0.0325	0.0595	0.0595
1.0302	0.0778	0.0340	0.0678	0.0678
1.0714	0.0768	0.0357	0.0768	0.0768

1.1126	0.0757	0.0373	0.0866	0.0866
1.1538	0.0747	0.0389	0.0972	0.0972
1.1951	0.0737	0.0406	0.1086	0.1086
1.2363	0.0727	0.0423	0.1151	0.1151
1.2775	0.0717	0.0440	0.1224	0.1224
1.3187	0.0707	0.0457	0.1357	0.1357
1.3599	0.0697	0.0475	0.1500	0.1500
1.4011	0.0687	0.0493	0.1653	0.1653
1.4423	0.0677	0.0511	0.1816	0.1816
1.4835	0.0667	0.0529	0.1990	0.1990
1.5247	0.0657	0.0547	0.2023	0.2023
1.5659	0.0647	0.0566	0.2198	0.2198
1.6071	0.0637	0.0584	0.2396	0.2396
1.6484	0.0627	0.0603	0.2604	0.2604
1.6896	0.0617	0.0622	0.2824	0.2824
1.7308	0.0608	0.0642	0.3053	0.3053
1.7500	0.0598	0.0651	0.6798	0.6798

Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	To Amended(cfs)	Infiltr(cfs)
1.7500	0.1037	0.0651	0.0000	0.2159	0.0110
1.7912	0.1048	0.0694	0.0000	0.2159	0.0221
1.8324	0.1059	0.0737	0.0000	0.2209	0.0332
1.8736	0.1070	0.0781	0.0000	0.2259	0.0444
1.9148	0.1081	0.0825	0.0000	0.2308	0.0555
1.9560	0.1092	0.0870	0.0000	0.2358	0.0667
1.9973	0.1103	0.0915	0.0000	0.2408	0.0779
2.0385	0.1115	0.0961	0.0000	0.2457	0.0892
2.0797	0.1126	0.1007	0.0000	0.2507	0.1005
2.1209	0.1137	0.1054	0.0000	0.2557	0.1118
2.1621	0.1148	0.1101	0.0000	0.2607	0.1231
2.2033	0.1159	0.1149	0.0000	0.2656	0.1345
2.2445	0.1171	0.1197	0.0000	0.2706	0.1458
2.2857	0.1182	0.1245	0.0000	0.2756	0.1573
2.3269	0.1193	0.1294	0.0000	0.2805	0.1687
2.3681	0.1205	0.1343	0.0000	0.2855	0.1802
2.4093	0.1216	0.1393	0.0000	0.2905	0.1917
2.4505	0.1227	0.1444	0.0000	0.2954	0.2032
2.4918	0.1239	0.1494	0.0000	0.3004	0.2148
2.5330	0.1250	0.1546	0.0000	0.3054	0.2264
2.5742	0.1262	0.1597	0.0000	0.3103	0.2380
2.6154	0.1273	0.1650	0.0000	0.3153	0.2496
2.6566	0.1285	0.1702	0.0000	0.3203	0.2613
2.6978	0.1296	0.1756	0.0000	0.3252	0.2730
2.7390	0.1308	0.1809	0.0000	0.3302	0.2847
2.7802	0.1320	0.1863	0.0000	0.3352	0.2965
2.8214	0.1331	0.1918	0.0000	0.3401	0.3083
2.8626	0.1343	0.1973	0.0000	0.3451	0.3201
2.9038	0.1355	0.2029	0.0000	0.3501	0.3319
2.9451	0.1366	0.2085	0.0000	0.3550	0.3438
2.9863	0.1378	0.2141	0.0000	0.3600	0.3557
3.0275	0.1390	0.2198	0.0000	0.3650	0.3676
3.0687	0.1402	0.2256	0.0000	0.3699	0.3796
3.1099	0.1414	0.2314	0.0000	0.3749	0.3915
3.1511	0.1426	0.2372	0.0000	0.3799	0.4036
3.1923	0.1437	0.2431	0.0000	0.3849	0.4156
3.2335	0.1449	0.2491	0.0000	0.3898	0.4277
3.2747	0.1461	0.2551	0.0000	0.3948	0.4398

3.3159	0.1473	0.2611	0.0000	0.3998	0.4519
3.3571	0.1485	0.2672	0.0000	0.4047	0.4640
3.3984	0.1497	0.2734	0.0000	0.4097	0.4762
3.4396	0.1510	0.2796	0.0000	0.4147	0.4884
3.4808	0.1522	0.2858	0.0000	0.4196	0.5007
3.5220	0.1534	0.2921	0.0000	0.4246	0.5129
3.5632	0.1546	0.2985	7.4148	0.4296	0.5252
3.6044	0.1558	0.3048	8.9151	0.4345	0.5375
3.6456	0.1570	0.3113	10.499	0.4395	0.5499
3.6868	0.1583	0.3178	12.160	0.4445	0.5623
3.7280	0.1595	0.3243	13.890	0.4494	0.5689
3.7500	0.1601	0.3278	15.682	0.4521	0.0000

Surface retention 3

Element Flows To:

Outlet 1

Outlet 2

Trapezoidal Pond 3

Bioretention 3

Trapezoidal Pond 3

Bottom Length: 100.00 ft.
 Bottom Width: 75.00 ft.
 Depth: 6 ft.
 Volume at riser head: 1.2552 acre-feet.
 Infiltration On
 Infiltration rate: 10
 Infiltration safety factor: 1
 Total Volume Infiltrated (ac-ft.): 137.517
 Total Volume Through Riser (ac-ft.): 0
 Total Volume Through Facility (ac-ft.): 137.517
Percent Infiltrated: **100**
 Total Precip Applied to Facility: 0
 Total Evap From Facility: 0
 Side slope 1: 4.7 To 1
 Side slope 2: 3 To 1
 Side slope 3: 3 To 1
 Side slope 4: 3 To 1
 Discharge Structure
 Riser Height: 5 ft.
 Riser Diameter: 24 in.
 Element Flows To:
 Outlet 1 Outlet 2

Pond Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
215.00	0.172	0.000	0.000	0.000
215.07	0.174	0.011	0.000	1.736
215.13	0.175	0.023	0.000	1.736
215.20	0.177	0.035	0.000	1.736
215.27	0.179	0.046	0.000	1.736
215.33	0.181	0.059	0.000	1.736
215.40	0.183	0.071	0.000	1.736
215.47	0.185	0.083	0.000	1.736
215.53	0.187	0.095	0.000	1.736
215.60	0.189	0.108	0.000	1.736
215.67	0.191	0.121	0.000	1.736
215.73	0.193	0.133	0.000	1.736
215.80	0.195	0.146	0.000	1.736
215.87	0.197	0.160	0.000	1.736
215.93	0.199	0.173	0.000	1.736
216.00	0.201	0.186	0.000	1.736
216.07	0.203	0.200	0.000	1.736
216.13	0.205	0.213	0.000	1.736
216.20	0.207	0.227	0.000	1.736
216.27	0.209	0.241	0.000	1.736
216.33	0.211	0.255	0.000	1.736
216.40	0.213	0.269	0.000	1.736
216.47	0.215	0.283	0.000	1.736
216.53	0.217	0.298	0.000	1.736
216.60	0.219	0.312	0.000	1.736
216.67	0.221	0.327	0.000	1.736
216.73	0.223	0.342	0.000	1.736
216.80	0.226	0.357	0.000	1.736
216.87	0.228	0.372	0.000	1.736

216.93	0.230	0.387	0.000	1.736
217.00	0.232	0.403	0.000	1.736
217.07	0.234	0.418	0.000	1.736
217.13	0.236	0.434	0.000	1.736
217.20	0.238	0.450	0.000	1.736
217.27	0.241	0.466	0.000	1.736
217.33	0.243	0.482	0.000	1.736
217.40	0.245	0.498	0.000	1.736
217.47	0.247	0.515	0.000	1.736
217.53	0.249	0.531	0.000	1.736
217.60	0.252	0.548	0.000	1.736
217.67	0.254	0.565	0.000	1.736
217.73	0.256	0.582	0.000	1.736
217.80	0.258	0.599	0.000	1.736
217.87	0.261	0.617	0.000	1.736
217.93	0.263	0.634	0.000	1.736
218.00	0.265	0.652	0.000	1.736
218.07	0.268	0.669	0.000	1.736
218.13	0.270	0.687	0.000	1.736
218.20	0.272	0.705	0.000	1.736
218.27	0.275	0.724	0.000	1.736
218.33	0.277	0.742	0.000	1.736
218.40	0.279	0.761	0.000	1.736
218.47	0.282	0.779	0.000	1.736
218.53	0.284	0.798	0.000	1.736
218.60	0.286	0.817	0.000	1.736
218.67	0.289	0.837	0.000	1.736
218.73	0.291	0.856	0.000	1.736
218.80	0.293	0.875	0.000	1.736
218.87	0.296	0.895	0.000	1.736
218.93	0.298	0.915	0.000	1.736
219.00	0.301	0.935	0.000	1.736
219.07	0.303	0.955	0.000	1.736
219.13	0.306	0.975	0.000	1.736
219.20	0.308	0.996	0.000	1.736
219.27	0.311	1.017	0.000	1.736
219.33	0.313	1.037	0.000	1.736
219.40	0.315	1.058	0.000	1.736
219.47	0.318	1.080	0.000	1.736
219.53	0.320	1.101	0.000	1.736
219.60	0.323	1.122	0.000	1.736
219.67	0.326	1.144	0.000	1.736
219.73	0.328	1.166	0.000	1.736
219.80	0.331	1.188	0.000	1.736
219.87	0.333	1.210	0.000	1.736
219.93	0.336	1.232	0.000	1.736
220.00	0.338	1.255	0.000	1.736
220.07	0.341	1.277	0.365	1.736
220.13	0.343	1.300	1.030	1.736
220.20	0.346	1.323	1.886	1.736
220.27	0.349	1.346	2.883	1.736
220.33	0.351	1.370	3.979	1.736
220.40	0.354	1.393	5.134	1.736
220.47	0.357	1.417	6.307	1.736
220.53	0.359	1.441	7.456	1.736
220.60	0.362	1.465	8.540	1.736
220.67	0.364	1.489	9.523	1.736
220.73	0.367	1.514	10.37	1.736

220.80	0.370	1.538	11.08	1.736
220.87	0.373	1.563	11.65	1.736
220.93	0.375	1.588	12.09	1.736
221.00	0.378	1.613	12.46	1.736
221.07	0.381	1.638	13.01	1.736

Bioretention 4

Bottom Length:	41.00 ft.
Bottom Width:	19.80 ft.
Material thickness of first layer:	0.25
Material type for first layer:	SMMWW 12 in/hr
Material thickness of second layer:	1.5
Material type for second layer:	SMMWW 12 in/hr
Material thickness of third layer:	0
Material type for third layer:	GRAVEL
Infiltration On	
Infiltration rate:	10
Infiltration safety factor:	1
Wetted surface area On	
Total Volume Infiltrated (ac-ft.):	96.252
Total Volume Through Riser (ac-ft.):	0
Total Volume Through Facility (ac-ft.):	96.252
Percent Infiltrated:	100
Total Precip Applied to Facility:	4.792
Total Evap From Facility:	1.882
Underdrain not used	
Discharge Structure	
Riser Height:	1.5 ft.
Riser Diameter:	10 in.
Element Flows To:	
Outlet 1	Outlet 2

Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
215.00	0.0358	0.0000	0.0000	0.0000
215.04	0.0356	0.0004	0.0000	0.0000
215.08	0.0351	0.0007	0.0000	0.0000
215.12	0.0347	0.0011	0.0000	0.0000
215.16	0.0342	0.0015	0.0000	0.0000
215.21	0.0338	0.0018	0.0000	0.0000
215.25	0.0333	0.0022	0.0004	0.0004
215.29	0.0329	0.0026	0.0007	0.0007
215.33	0.0324	0.0030	0.0010	0.0010
215.37	0.0320	0.0034	0.0014	0.0014
215.41	0.0316	0.0038	0.0019	0.0019
215.45	0.0311	0.0043	0.0025	0.0025
215.49	0.0307	0.0047	0.0032	0.0032
215.54	0.0303	0.0051	0.0040	0.0040
215.58	0.0298	0.0056	0.0050	0.0050
215.62	0.0294	0.0060	0.0061	0.0061
215.66	0.0290	0.0065	0.0074	0.0074
215.70	0.0286	0.0070	0.0080	0.0080
215.74	0.0281	0.0074	0.0090	0.0090
215.78	0.0277	0.0079	0.0107	0.0107
215.82	0.0273	0.0084	0.0125	0.0125
215.87	0.0269	0.0089	0.0146	0.0146
215.91	0.0265	0.0094	0.0169	0.0169
215.95	0.0261	0.0099	0.0191	0.0191
215.99	0.0257	0.0104	0.0196	0.0196
216.03	0.0253	0.0110	0.0224	0.0224
216.07	0.0249	0.0115	0.0255	0.0255

216.11	0.0245	0.0120	0.0288	0.0288
216.15	0.0241	0.0126	0.0324	0.0324
216.20	0.0237	0.0131	0.0363	0.0363
216.24	0.0234	0.0137	0.0386	0.0386
216.28	0.0230	0.0143	0.0411	0.0411
216.32	0.0226	0.0149	0.0457	0.0457
216.36	0.0222	0.0154	0.0506	0.0506
216.40	0.0219	0.0160	0.0559	0.0559
216.44	0.0215	0.0167	0.0616	0.0616
216.48	0.0211	0.0173	0.0677	0.0677
216.52	0.0208	0.0179	0.0690	0.0690
216.57	0.0204	0.0185	0.0751	0.0751
216.61	0.0200	0.0192	0.0821	0.0821
216.65	0.0197	0.0198	0.0894	0.0894
216.69	0.0193	0.0205	0.0972	0.0972
216.73	0.0190	0.0211	0.1053	0.1053
216.75	0.0186	0.0215	0.2348	0.2348

Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	To Amended(cfs)	Infiltr(cfs)
1.7500	0.0358	0.0215	0.0000	0.0673	0.0047
1.7912	0.0363	0.0229	0.0000	0.0673	0.0094
1.8324	0.0368	0.0244	0.0000	0.0689	0.0142
1.8736	0.0372	0.0260	0.0000	0.0704	0.0190
1.9148	0.0377	0.0275	0.0000	0.0720	0.0238
1.9560	0.0382	0.0291	0.0000	0.0735	0.0286
1.9973	0.0387	0.0307	0.0000	0.0751	0.0335
2.0385	0.0391	0.0323	0.0000	0.0766	0.0384
2.0797	0.0396	0.0339	0.0000	0.0782	0.0433
2.1209	0.0401	0.0355	0.0000	0.0797	0.0482
2.1621	0.0406	0.0372	0.0000	0.0813	0.0532
2.2033	0.0411	0.0389	0.0000	0.0828	0.0582
2.2445	0.0416	0.0406	0.0000	0.0844	0.0633
2.2857	0.0421	0.0423	0.0000	0.0859	0.0683
2.3269	0.0426	0.0440	0.0000	0.0875	0.0734
2.3681	0.0431	0.0458	0.0000	0.0890	0.0785
2.4093	0.0436	0.0476	0.0000	0.0906	0.0837
2.4505	0.0441	0.0494	0.0000	0.0921	0.0889
2.4918	0.0446	0.0512	0.0000	0.0936	0.0941
2.5330	0.0452	0.0531	0.0000	0.0952	0.0993
2.5742	0.0457	0.0550	0.0000	0.0967	0.1046
2.6154	0.0462	0.0569	0.0000	0.0983	0.1098
2.6566	0.0467	0.0588	0.0000	0.0998	0.1152
2.6978	0.0472	0.0607	0.0000	0.1014	0.1205
2.7390	0.0478	0.0627	0.0000	0.1029	0.1259
2.7802	0.0483	0.0646	0.0000	0.1045	0.1313
2.8214	0.0488	0.0666	0.0000	0.1060	0.1367
2.8626	0.0494	0.0687	0.0000	0.1076	0.1422
2.9038	0.0499	0.0707	0.0000	0.1091	0.1477
2.9451	0.0505	0.0728	0.0000	0.1107	0.1532
2.9863	0.0510	0.0749	0.0000	0.1122	0.1587
3.0275	0.0516	0.0770	0.0000	0.1138	0.1643
3.0687	0.0521	0.0791	0.0000	0.1153	0.1699
3.1099	0.0527	0.0813	0.0000	0.1169	0.1755
3.1511	0.0532	0.0835	0.0000	0.1184	0.1812
3.1923	0.0538	0.0857	0.0000	0.1200	0.1869
3.2335	0.0544	0.0879	0.0000	0.1215	0.1926
3.2747	0.0549	0.0901	0.0000	0.1231	0.1983

3.3159	0.0555	0.0924	0.0000	0.1246	0.2041
3.3571	0.0561	0.0947	0.0000	0.1262	0.2099
3.3984	0.0566	0.0970	0.0000	0.1277	0.2157
3.4396	0.0572	0.0994	0.0000	0.1293	0.2216
3.4808	0.0578	0.1018	0.0000	0.1308	0.2275
3.5220	0.0584	0.1042	0.0000	0.1324	0.2334
3.5632	0.0590	0.1066	1.1865	0.1339	0.2393
3.6044	0.0596	0.1090	1.2920	0.1355	0.2453
3.6456	0.0602	0.1115	1.3660	0.1370	0.2513
3.6868	0.0607	0.1140	1.4456	0.1386	0.2573
3.7280	0.0613	0.1165	1.5122	0.1401	0.2606
3.7500	0.0617	0.1178	1.5761	0.1409	0.0000

Surface retention 4

Element Flows To:

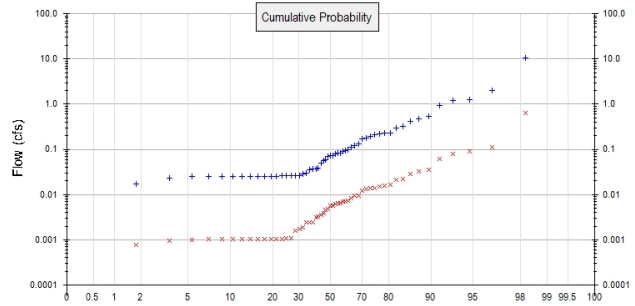
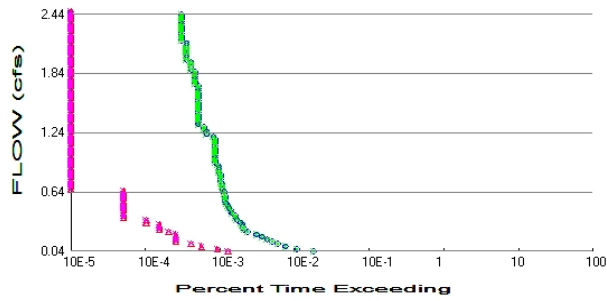
Outlet 1

Outlet 2

Bioretention 4

Analysis Results

POC 1



+ Predeveloped x Mitigated

Predeveloped Landuse Totals for POC #1

Total Pervious Area: 32.08
 Total Impervious Area: 0

Mitigated Landuse Totals for POC #1

Total Pervious Area: 17.13
 Total Impervious Area: 17.24

Flow Frequency Method: Log Pearson Type III 17B

Flow Frequency Return Periods for Predeveloped. POC #1

Return Period	Flow(cfs)
2 year	0.073583
5 year	0.258538
10 year	0.549081
25 year	1.32257
50 year	2.435458
100 year	4.342687

Flow Frequency Return Periods for Mitigated. POC #1

Return Period	Flow(cfs)
2 year	0.004648
5 year	0.018015
10 year	0.039396
25 year	0.096162
50 year	0.176787
100 year	0.312539

Annual Peaks

Annual Peaks for Predeveloped and Mitigated. POC #1

Year	Predeveloped	Mitigated
1956	0.222	0.015
1957	0.055	0.006
1958	0.038	0.004
1959	0.080	0.007
1960	0.074	0.006
1961	0.100	0.007
1962	0.026	0.001
1963	1.267	0.089
1964	0.320	0.022
1965	0.113	0.010

1966	0.026	0.001
1967	0.094	0.006
1968	0.035	0.002
1969	0.036	0.002
1970	0.072	0.005
1971	0.475	0.032
1972	0.418	0.029
1973	0.025	0.001
1974	0.181	0.012
1975	0.037	0.003
1976	0.209	0.014
1977	0.026	0.001
1978	0.172	0.014
1979	0.026	0.001
1980	0.026	0.002
1981	0.534	0.036
1982	0.917	0.061
1983	0.049	0.003
1984	0.069	0.005
1985	0.025	0.001
1986	0.091	0.007
1987	0.194	0.013
1988	0.025	0.001
1989	0.026	0.001
1990	0.030	0.002
1991	1.973	0.113
1992	10.608	0.643
1993	1.205	0.079
1994	0.025	0.001
1995	0.082	0.006
1996	0.229	0.016
1997	0.134	0.009
1998	0.025	0.001
1999	0.026	0.002
2000	0.017	0.001
2001	0.017	0.001
2002	0.028	0.002
2003	0.235	0.017
2004	0.295	0.021
2005	0.025	0.001
2006	0.025	0.001
2007	0.026	0.001
2008	0.023	0.001
2009	0.058	0.004
2010	0.084	0.006
2011	0.122	0.008

Ranked Annual Peaks

Ranked Annual Peaks for Predeveloped and Mitigated. POC #1

Rank	Predeveloped	Mitigated
1	10.6076	0.6429
2	1.9731	0.1126
3	1.2666	0.0891
4	1.2052	0.0790
5	0.9173	0.0610
6	0.5335	0.0360
7	0.4752	0.0320
8	0.4184	0.0287

9	0.3199	0.0217
10	0.2952	0.0215
11	0.2346	0.0166
12	0.2293	0.0157
13	0.2220	0.0150
14	0.2094	0.0141
15	0.1944	0.0138
16	0.1813	0.0134
17	0.1725	0.0122
18	0.1342	0.0096
19	0.1216	0.0093
20	0.1128	0.0082
21	0.1001	0.0072
22	0.0943	0.0071
23	0.0914	0.0070
24	0.0845	0.0064
25	0.0823	0.0064
26	0.0798	0.0062
27	0.0744	0.0058
28	0.0716	0.0056
29	0.0689	0.0048
30	0.0582	0.0046
31	0.0555	0.0039
32	0.0489	0.0035
33	0.0382	0.0033
34	0.0370	0.0031
35	0.0363	0.0025
36	0.0346	0.0025
37	0.0295	0.0024
38	0.0283	0.0019
39	0.0259	0.0017
40	0.0258	0.0016
41	0.0258	0.0011
42	0.0257	0.0011
43	0.0257	0.0011
44	0.0256	0.0011
45	0.0255	0.0010
46	0.0255	0.0010
47	0.0255	0.0010
48	0.0254	0.0010
49	0.0253	0.0010
50	0.0253	0.0010
51	0.0253	0.0010
52	0.0248	0.0010
53	0.0247	0.0010
54	0.0230	0.0009
55	0.0174	0.0008
56	0.0171	0.0007

Duration Flows

The Facility PASSED

Flow(cfs)	Predev	Mit	Percentage	Pass/Fail
0.0368	353	25	7	Pass
0.0610	210	18	8	Pass
0.0852	147	11	7	Pass
0.1095	115	8	6	Pass
0.1337	98	5	5	Pass
0.1579	84	5	5	Pass
0.1822	72	5	6	Pass
0.2064	60	5	8	Pass
0.2306	46	4	8	Pass
0.2549	41	3	7	Pass
0.2791	40	3	7	Pass
0.3033	39	3	7	Pass
0.3275	37	2	5	Pass
0.3518	33	2	6	Pass
0.3760	32	1	3	Pass
0.4002	31	1	3	Pass
0.4245	28	1	3	Pass
0.4487	27	1	3	Pass
0.4729	26	1	3	Pass
0.4971	25	1	4	Pass
0.5214	24	1	4	Pass
0.5456	23	1	4	Pass
0.5698	23	1	4	Pass
0.5941	23	1	4	Pass
0.6183	22	1	4	Pass
0.6425	22	1	4	Pass
0.6667	21	0	0	Pass
0.6910	21	0	0	Pass
0.7152	21	0	0	Pass
0.7394	20	0	0	Pass
0.7637	20	0	0	Pass
0.7879	20	0	0	Pass
0.8121	20	0	0	Pass
0.8363	19	0	0	Pass
0.8606	19	0	0	Pass
0.8848	19	0	0	Pass
0.9090	18	0	0	Pass
0.9333	17	0	0	Pass
0.9575	17	0	0	Pass
0.9817	17	0	0	Pass
1.0059	17	0	0	Pass
1.0302	17	0	0	Pass
1.0544	17	0	0	Pass
1.0786	17	0	0	Pass
1.1029	17	0	0	Pass
1.1271	17	0	0	Pass
1.1513	17	0	0	Pass
1.1756	17	0	0	Pass
1.1998	16	0	0	Pass
1.2240	13	0	0	Pass
1.2482	13	0	0	Pass
1.2725	12	0	0	Pass
1.2967	12	0	0	Pass

1.3209	10	0	0	Pass
1.3452	10	0	0	Pass
1.3694	10	0	0	Pass
1.3936	10	0	0	Pass
1.4178	10	0	0	Pass
1.4421	10	0	0	Pass
1.4663	10	0	0	Pass
1.4905	10	0	0	Pass
1.5148	10	0	0	Pass
1.5390	10	0	0	Pass
1.5632	10	0	0	Pass
1.5874	10	0	0	Pass
1.6117	10	0	0	Pass
1.6359	10	0	0	Pass
1.6601	10	0	0	Pass
1.6844	10	0	0	Pass
1.7086	10	0	0	Pass
1.7328	9	0	0	Pass
1.7570	9	0	0	Pass
1.7813	9	0	0	Pass
1.8055	9	0	0	Pass
1.8297	9	0	0	Pass
1.8540	9	0	0	Pass
1.8782	8	0	0	Pass
1.9024	8	0	0	Pass
1.9266	8	0	0	Pass
1.9509	8	0	0	Pass
1.9751	7	0	0	Pass
1.9993	7	0	0	Pass
2.0236	7	0	0	Pass
2.0478	7	0	0	Pass
2.0720	7	0	0	Pass
2.0963	7	0	0	Pass
2.1205	7	0	0	Pass
2.1447	7	0	0	Pass
2.1689	6	0	0	Pass
2.1932	6	0	0	Pass
2.2174	6	0	0	Pass
2.2416	6	0	0	Pass
2.2659	6	0	0	Pass
2.2901	6	0	0	Pass
2.3143	6	0	0	Pass
2.3385	6	0	0	Pass
2.3628	6	0	0	Pass
2.3870	6	0	0	Pass
2.4112	6	0	0	Pass
2.4355	6	0	0	Pass

Water Quality

Water Quality BMP Flow and Volume for POC #1

On-line facility volume: 0 acre-feet

On-line facility target flow: 0 cfs.

Adjusted for 15 min: 0 cfs.

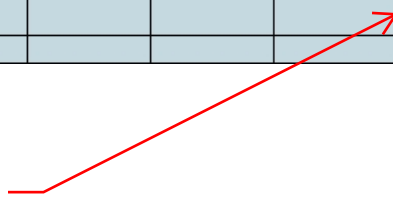
Off-line facility target flow: 0 cfs.

Adjusted for 15 min: 0 cfs.

LID Report

LID Technique	Used for Treatment ?	Total Volume Needs Treatment (ac-ft)	Volume Through Facility (ac-ft)	Infiltration Volume (ac-ft)	Cumulative Volume Infiltration Credit	Percent Volume Infiltrated	Water Quality	Percent Water Quality Treated	Comment
Trapezoidal Pond 1 POC retention 1	<input type="checkbox"/>	128.03			<input type="checkbox"/>	100.00			
Trapezoidal Pond 2 POC retention 2	<input type="checkbox"/>	57.51			<input type="checkbox"/>	100.00			
Trapezoidal Pond 3 POC retention 3	<input type="checkbox"/>	328.43			<input type="checkbox"/>	94.96			
Trapezoidal Pond 4 POC retention 4 POC	<input type="checkbox"/>	125.14			<input type="checkbox"/>	100.00			
	<input type="checkbox"/>	1267.48			<input type="checkbox"/>	96.62			
	<input type="checkbox"/>	87.59			<input type="checkbox"/>	100.00			
Total Volume Infiltrated		3323.17	0.00	0.00		96.81	0.00	0%	No Treat. Credit
Compliance with LID Standard 8% of 2-yr to 50% of 2-yr									Duration Analysis Result = Passed

INDICATES THAT THE SITE MEETS LID PERFORMANCE STANDARD



Model Default Modifications

Total of 0 changes have been made.

PERLND Changes

No PERLND changes have been made.

IMPLND Changes

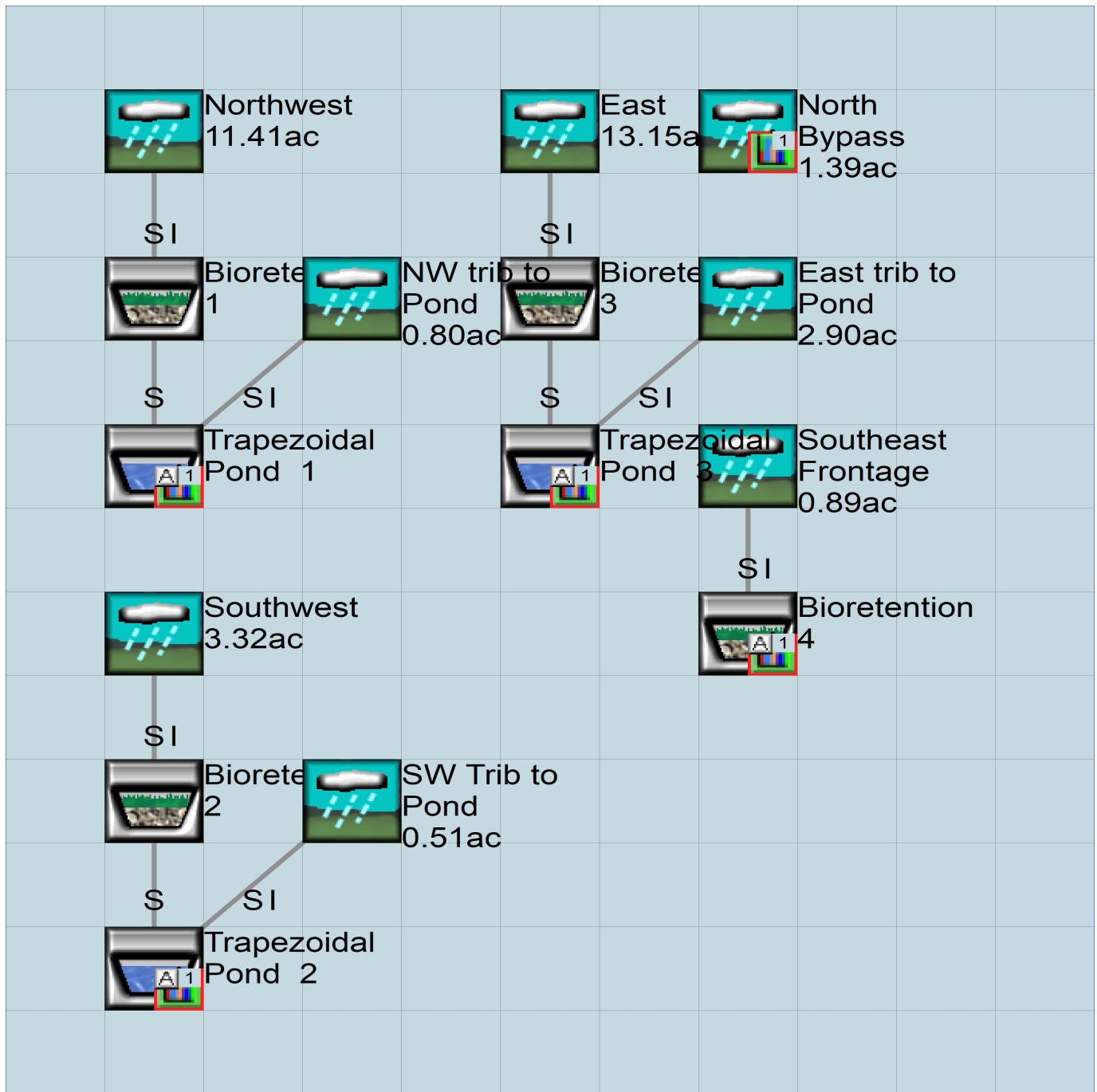
No IMPLND changes have been made.

Appendix
Predeveloped Schematic



Basin 1
32.08ac

Mitigated Schematic



Predeveloped UCI File

RUN

GLOBAL

```
WVHM4 model simulation
START      1955 10 01      END      2011 09 30
RUN INTERP OUTPUT LEVEL   3      0
RESUME     0 RUN          1
UNIT SYSTEM 1
```

END GLOBAL

FILES

```
<File> <Un#> <-----File Name----->***
<-ID->                                     ***
WDM      26      21-100 Site Analysis.wdm
MESSU    25      Pre21-100 Site Analysis.MES
          27      Pre21-100 Site Analysis.L61
          28      Pre21-100 Site Analysis.L62
          30      POC21-100 Site Analysis1.dat
```

END FILES

OPN SEQUENCE

```
INGRP          INDELT 00:15
  PERLND        1
  COPY          501
  DISPLY        1
```

END INGRP

END OPN SEQUENCE

DISPLY

DISPLY-INFO1

```
# - #<-----Title----->***TRAN PIVL DIG1 FIL1  PYR DIG2 FIL2 YRND
1      Basin 1          MAX          1      2      30      9
```

END DISPLY-INFO1

END DISPLY

COPY

TIMESERIES

```
# - # NPT NMN ***
1      1      1
501    1      1
```

END TIMESERIES

END COPY

GENER

OPCODE

```
#      # OPCODE ***
```

END OPCODE

PARAM

```
#      #          K ***
```

END PARAM

END GENER

PERLND

GEN-INFO

```
<PLS ><-----Name----->NBLKS  Unit-systems  Printer ***
# - #          User  t-series  Engl Metr ***
          in  out          ***
1      A/B, Forest, Flat  1      1      1      1      27      0
```

END GEN-INFO

*** Section PWATER***

ACTIVITY

```
<PLS > ***** Active Sections *****
# - # ATMP SNOW PWAT  SED  PST  PWG  PQAL MSTL PEST NITR PHOS TRAC ***
1      0      0      1      0      0      0      0      0      0      0      0      0
```

END ACTIVITY

PRINT-INFO

```
<PLS > ***** Print-flags ***** PIVL  PYR
# - # ATMP SNOW PWAT  SED  PST  PWG  PQAL MSTL PEST NITR PHOS TRAC *****
1      0      0      4      0      0      0      0      0      0      0      0      0      1      9
```

END PRINT-INFO

```

PWAT-PARM1
<PLS > PWATER variable monthly parameter value flags ***
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT ***
1 0 0 0 0 0 0 0 0 0 0 0
END PWAT-PARM1

PWAT-PARM2
<PLS > PWATER input info: Part 2 ***
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
1 0 5 2 400 0.05 0.3 0.996
END PWAT-PARM2

PWAT-PARM3
<PLS > PWATER input info: Part 3 ***
# - # ***PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP
1 0 0 2 2 0 0 0
END PWAT-PARM3

PWAT-PARM4
<PLS > PWATER input info: Part 4 ***
# - # CEPSC UZSN NSUR INTFW IRC LZETP ***
1 0.2 0.5 0.35 0 0.7 0.7
END PWAT-PARM4

PWAT-STATE1
<PLS > *** Initial conditions at start of simulation
ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***
# - # *** CEPS SURS UZS IFWS LZS AGWS GWVS
1 0 0 0 0 3 1 0
END PWAT-STATE1

END PERLND

IMPLND
GEN-INFO
<PLS ><-----Name-----> Unit-systems Printer ***
# - # User t-series Engr Metr ***
in out ***
END GEN-INFO
*** Section IWATER***

ACTIVITY
<PLS > ***** Active Sections *****
# - # ATMP SNOW IWAT SLD IWG IQAL ***
END ACTIVITY

PRINT-INFO
<ILS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW IWAT SLD IWG IQAL *****
END PRINT-INFO

IWAT-PARM1
<PLS > IWATER variable monthly parameter value flags ***
# - # CSNO RTOP VRS VNN RTLI ***
END IWAT-PARM1

IWAT-PARM2
<PLS > IWATER input info: Part 2 ***
# - # *** LSUR SLSUR NSUR RETSC
END IWAT-PARM2

IWAT-PARM3
<PLS > IWATER input info: Part 3 ***
# - # ***PETMAX PETMIN
END IWAT-PARM3

IWAT-STATE1
<PLS > *** Initial conditions at start of simulation
# - # *** RETS SURS
END IWAT-STATE1

```


END IMPLND

SCHEMATIC

<-Source->	<Name> #	<--Area-->	<-factor-->	<-Target->	<Name> #	MBLK	Tbl#	***
Basin	1							***
PERLND	1		32.08	COPY	501		12	
PERLND	1		32.08	COPY	501		13	

*****Routing*****
END SCHEMATIC

NETWORK

<-Volume->	<-Grp>	<-Member->	<--Mult-->	Tran	<-Target vols>	<-Grp>	<-Member->	***
<Name> #		<Name> #	#	<-factor-->strg	<Name> #	#	<Name> #	***
COPY	501	OUTPUT	MEAN	1 1	48.4	DISPLY	1	INPUT TIMSER 1

<-Volume->	<-Grp>	<-Member->	<--Mult-->	Tran	<-Target vols>	<-Grp>	<-Member->	***
<Name> #		<Name> #	#	<-factor-->strg	<Name> #	#	<Name> #	***

END NETWORK

RCHRES

GEN-INFO	RCHRES	Name	Nexits	Unit Systems	Printer	***
# - #	<----->	<---->	User	T-series	Engl Metr	LKFG
				in out		***

END GEN-INFO
*** Section RCHRES***

ACTIVITY

<PLS > ***** Active Sections *****

# - #	HYFG	ADFG	CNFG	HTFG	SDFG	GQFG	OXFG	NUFG	PKFG	PHFG	***

END ACTIVITY

PRINT-INFO

<PLS > ***** Print-flags ***** PIVL PYR

# - #	HYDR	ADCA	CONS	HEAT	SED	GQL	OXRX	NUTR	PLNK	PHCB	PIVL	PYR	*****

END PRINT-INFO

HYDR-PARM1

RCHRES	Flags for each HYDR Section	***	ODGTFG for each	FUNCT for each	***
# - #	VC A1 A2 A3	ODFVFG for each	*** possible exit	*** possible exit	possible exit
	FG FG FG FG	possible exit	*** possible exit	possible exit	***
	* * * *	* * * *	* * * *	* * * *	***

END HYDR-PARM1

HYDR-PARM2

# - #	FTABNO	LEN	DELTH	STCOR	KS	DB50	***
<----->	<----->	<----->	<----->	<----->	<----->	<----->	***

END HYDR-PARM2

HYDR-INIT

RCHRES	Initial conditions for each HYDR section	***	
# - #	*** VOL	Initial value of COLIND	Initial value of OUTDGT
	*** ac-ft	for each possible exit	for each possible exit
	<----->	<----->	<----->

END HYDR-INIT

END RCHRES

SPEC-ACTIONS

END SPEC-ACTIONS

FTABLES

END FTABLES

EXT SOURCES

<-Volume->	<Member>	SsysSgap	<--Mult-->	Tran	<-Target vols>	<-Grp>	<-Member->	***
<Name> #	<Name> #	tem	strg	<-factor-->strg	<Name> #	#	<Name> #	***
WDM	2	PREC	ENGL	1	PERLND	1 999	EXTNL	PREC
WDM	2	PREC	ENGL	1	IMPLND	1 999	EXTNL	PREC

```
WDM      1 EVAP      ENGL      0.76          PERLND   1 999 EXTNL  PETINP
WDM      1 EVAP      ENGL      0.76          IMPLND   1 999 EXTNL  PETINP
```

END EXT SOURCES

EXT TARGETS

```
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name>      #      <Name> # #<-factor->strg <Name>      # <Name>      tem strg strg***
COPY  501 OUTPUT MEAN  1 1      48.4      WDM  501 FLOW      ENGL      REPL
END EXT TARGETS
```

MASS-LINK

```
<Volume>   <-Grp> <-Member-><--Mult-->   <Target>           <-Grp> <-Member->***
<Name>     #      <Name> # #<-factor->   <Name>           <Name> # #***
MASS-LINK  12
PERLND     PWATER SURO           0.083333      COPY           INPUT  MEAN
END MASS-LINK  12
```

```
MASS-LINK  13
PERLND     PWATER IFWO           0.083333      COPY           INPUT  MEAN
END MASS-LINK  13
```

END MASS-LINK

END RUN

Mitigated UCI File

RUN

GLOBAL

WVHM4 model simulation
START 1955 10 01 END 2011 09 30
RUN INTERP OUTPUT LEVEL 3 0
RESUME 0 RUN 1 UNIT SYSTEM 1
END GLOBAL

FILES

<File>	<Un#>	<-----File Name----->	***
<-ID->			***
WDM	26	21-100 Site Analysis.wdm	
MESSU	25	Mit21-100 Site Analysis.MES	
	27	Mit21-100 Site Analysis.L61	
	28	Mit21-100 Site Analysis.L62	
	30	POC21-100 Site Analysis1.dat	

END FILES

OPN SEQUENCE

INGRP INDELT 00:15

PERLND	1
PERLND	4
PERLND	6
PERLND	31
IMPLND	1
IMPLND	4
IMPLND	5
IMPLND	8
PERLND	2
PERLND	3
IMPLND	3
IMPLND	14
RCHRES	1
RCHRES	2
RCHRES	3
RCHRES	4
RCHRES	5
RCHRES	6
RCHRES	7
RCHRES	8
RCHRES	9
RCHRES	10
RCHRES	11
COPY	1
COPY	501
COPY	601
DISPLY	1

END INGRP

END OPN SEQUENCE

DISPLY

DISPLY-INFO1

#	-	#	<-----Title----->	***	TRAN	PIVL	DIG1	FIL1	PYR	DIG2	FIL2	YRND
1			Surface retention 4		MAX				1	2	30	9

END DISPLY-INFO1

END DISPLY

COPY

TIMESERIES

#	-	#	NPT	NMN	***
1			1	1	
501			1	1	
601			1	1	

END TIMESERIES

END COPY

GENER

OPCODE

#	#	OPCD	***
---	---	------	-----

END OPCODE

```

PARM
# # K ***
END PARM
END GENER
PERLND

```

```

GEN-INFO
<PLS ><-----Name----->NBLKS Unit-systems Printer ***
# - # User t-series Engr Metr ***
in out ***
1 A/B, Forest, Flat 1 1 1 1 27 0
4 A/B, Pasture, Flat 1 1 1 1 27 0
6 A/B, Pasture, Steep 1 1 1 1 27 0
31 A/B/IMP INF/FLAT 1 1 1 1 27 0
2 A/B, Forest, Mod 1 1 1 1 27 0
3 A/B, Forest, Steep 1 1 1 1 27 0
END GEN-INFO
*** Section PWATER***

```

```

ACTIVITY
<PLS > ***** Active Sections *****
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ***
1 0 0 1 0 0 0 0 0 0 0 0 0 0
4 0 0 1 0 0 0 0 0 0 0 0 0 0
6 0 0 1 0 0 0 0 0 0 0 0 0 0
31 0 0 1 0 0 0 0 0 0 0 0 0 0
2 0 0 1 0 0 0 0 0 0 0 0 0 0
3 0 0 1 0 0 0 0 0 0 0 0 0 0
END ACTIVITY

```

```

PRINT-INFO
<PLS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
1 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9
4 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9
6 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9
31 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9
2 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9
3 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9
END PRINT-INFO

```

```

PWAT-PARM1
<PLS > PWATER variable monthly parameter value flags ***
# - # CSNO RTOP UZFG VCS VUZ VMN VIFW VIRC VLE INFC HWT ***
1 0 0 0 0 0 0 0 0 0 0 0 0
4 0 0 0 0 0 0 0 0 0 0 0 0
6 0 0 0 0 0 0 0 0 0 0 0 0
31 0 0 0 0 0 0 0 0 0 0 0 0
2 0 0 0 0 0 0 0 0 0 0 0 0
3 0 0 0 0 0 0 0 0 0 0 0 0
END PWAT-PARM1

```

```

PWAT-PARM2
<PLS > PWATER input info: Part 2 ***
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARV AGWRC
1 0 5 2 400 0.05 0.3 0.996
4 0 5 1.5 400 0.05 0.3 0.996
6 0 5 1.5 400 0.15 0.3 0.996
31 0 5 0.8 400 0.05 0.3 0.996
2 0 5 2 400 0.1 0.3 0.996
3 0 5 2 400 0.15 0.3 0.996
END PWAT-PARM2

```

```

PWAT-PARM3
<PLS > PWATER input info: Part 3 ***
# - # ***PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP
1 0 0 2 2 0 0 0
4 0 0 2 2 0 0 0
6 0 0 2 2 0 0 0
31 0 0 2 2 0 0 0
2 0 0 2 2 0 0 0

```

3 0 0 2 2 0 0 0

END PWAT-PARM3

PWAT-PARM4

```

<PLS > PWATER input info: Part 4 ***
# - # CEPSC UZSN NSUR INTFW IRC LZETP ***
1 0.2 0.5 0.35 0 0.7 0.7
4 0.15 0.5 0.3 0 0.7 0.4
6 0.15 0.5 0.3 0 0.7 0.4
31 0.1 0.5 0.25 0 0.7 0.25
2 0.2 0.5 0.35 0 0.7 0.7
3 0.2 0.5 0.35 0 0.7 0.7

```

END PWAT-PARM4

PWAT-STATE1

```

<PLS > *** Initial conditions at start of simulation
ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***
# - # *** CEPS SURS UZS IFWS LZS AGWS GWVS
1 0 0 0 0 3 1 0
4 0 0 0 0 3 1 0
6 0 0 0 0 3 1 0
31 0 0 0 0 3 1 0
2 0 0 0 0 3 1 0
3 0 0 0 0 3 1 0

```

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

```

<PLS ><-----Name-----> Unit-systems Printer ***
# - # User t-series Engl Metr ***
in out ***
1 ROADS/FLAT 1 1 1 27 0
4 ROOF TOPS/FLAT 1 1 1 27 0
5 DRIVEWAYS/FLAT 1 1 1 27 0
8 SIDEWALKS/FLAT 1 1 1 27 0
3 ROADS/STEEP 1 1 1 27 0
14 POND 1 1 1 27 0

```

END GEN-INFO

*** Section IWATER***

ACTIVITY

```

<PLS > ***** Active Sections *****
# - # ATMP SNOW IWAT SLD IWG IQAL ***
1 0 0 1 0 0 0
4 0 0 1 0 0 0
5 0 0 1 0 0 0
8 0 0 1 0 0 0
3 0 0 1 0 0 0
14 0 0 1 0 0 0

```

END ACTIVITY

PRINT-INFO

```

<ILS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW IWAT SLD IWG IQAL *****
1 0 0 4 0 0 0 1 9
4 0 0 4 0 0 0 1 9
5 0 0 4 0 0 0 1 9
8 0 0 4 0 0 0 1 9
3 0 0 4 0 0 0 1 9
14 0 0 4 0 0 0 1 9

```

END PRINT-INFO

IWAT-PARM1

```

<PLS > IWATER variable monthly parameter value flags ***
# - # CSNO RTOP VRS VNM RTLI ***
1 0 0 0 0 0
4 0 0 0 0 0
5 0 0 0 0 0
8 0 0 0 0 0

```

```

3      0  0  0  0  0
14     0  0  0  0  0
END IWAT-PARM1

```

```

IWAT-PARM2
<PLS >      IWATER input info: Part 2      ***
# - # ***  LSUR      SLSUR      NSUR      RETSC
1      400      0.01      0.1      0.1
4      400      0.01      0.1      0.1
5      400      0.01      0.1      0.1
8      400      0.01      0.1      0.1
3      400      0.1      0.1      0.05
14     400      0.01      0.1      0.1
END IWAT-PARM2

```

```

IWAT-PARM3
<PLS >      IWATER input info: Part 3      ***
# - # ***PETMAX      PETMIN
1      0      0
4      0      0
5      0      0
8      0      0
3      0      0
14     0      0
END IWAT-PARM3

```

```

IWAT-STATE1
<PLS > *** Initial conditions at start of simulation
# - # ***  RETS      SURS
1      0      0
4      0      0
5      0      0
8      0      0
3      0      0
14     0      0
END IWAT-STATE1

```

END IMPLND

```

SCHEMATIC
<-Source->      <--Area-->      <-Target-->      MBLK      ***
<Name> #      <-factor-->      <Name> #      Tbl#      ***
Northwest***
PERLND 1      0.4      RCHRES 1      2
PERLND 1      0.4      RCHRES 1      3
PERLND 4      3.59     RCHRES 1      2
PERLND 4      3.59     RCHRES 1      3
PERLND 6      0.15     RCHRES 1      2
PERLND 6      0.15     RCHRES 1      3
IMPLND 1      2.76     RCHRES 1      5
IMPLND 4      2.76     RCHRES 1      5
IMPLND 5      0.95     RCHRES 1      5
IMPLND 8      0.66     RCHRES 1      5
Southwest***
PERLND 4      1.53     RCHRES 3      2
PERLND 4      1.53     RCHRES 3      3
IMPLND 1      0.77     RCHRES 3      5
IMPLND 4      0.62     RCHRES 3      5
IMPLND 5      0.15     RCHRES 3      5
IMPLND 8      0.22     RCHRES 3      5
East***
PERLND 4      6.22     RCHRES 5      2
PERLND 4      6.22     RCHRES 5      3
IMPLND 1      1.88     RCHRES 5      5
IMPLND 4      3.41     RCHRES 5      5
IMPLND 5      0.95     RCHRES 5      5
IMPLND 8      0.55     RCHRES 5      5
Southeast Frontage***
PERLND 4      0.26     RCHRES 7      2
PERLND 4      0.26     RCHRES 7      3

```

PERLND	6	0.12	RCHRES	7	2
PERLND	6	0.12	RCHRES	7	3
IMPLND	1	0.37	RCHRES	7	5
IMPLND	8	0.09	RCHRES	7	5
SW Trib to Pond***					
PERLND	4	0.29	RCHRES	10	2
PERLND	4	0.29	RCHRES	10	3
IMPLND	3	0.07	RCHRES	10	5
IMPLND	14	0.15	RCHRES	10	5
East trib to Pond***					
PERLND	2	2.46	RCHRES	11	2
PERLND	2	2.46	RCHRES	11	3
IMPLND	3	0.08	RCHRES	11	5
IMPLND	14	0.36	RCHRES	11	5
NW trib to Pond***					
PERLND	6	0.36	RCHRES	9	2
PERLND	6	0.36	RCHRES	9	3
IMPLND	3	0.03	RCHRES	9	5
IMPLND	14	0.41	RCHRES	9	5
North Bypass***					
PERLND	2	1.02	COPY	501	12
PERLND	2	1.02	COPY	601	12
PERLND	2	1.02	COPY	501	13
PERLND	2	1.02	COPY	601	13
PERLND	3	0.29	COPY	501	12
PERLND	3	0.29	COPY	601	12
PERLND	3	0.29	COPY	501	13
PERLND	3	0.29	COPY	601	13
PERLND	6	0.08	COPY	501	12
PERLND	6	0.08	COPY	601	12
PERLND	6	0.08	COPY	501	13
PERLND	6	0.08	COPY	601	13
*****Routing*****					
PERLND	4	0.26	COPY	1	12
PERLND	6	0.12	COPY	1	12
PERLND	31	0.05	COPY	1	12
IMPLND	1	0.37	COPY	1	15
IMPLND	8	0.09	COPY	1	15
PERLND	4	0.26	COPY	1	13
PERLND	6	0.12	COPY	1	13
PERLND	31	0.05	COPY	1	13
RCHRES	2	1	RCHRES	9	7
RCHRES	2		COPY	1	17
RCHRES	1	1	RCHRES	9	7
RCHRES	1		COPY	1	17
RCHRES	1	1	RCHRES	2	8
RCHRES	4	1	RCHRES	10	7
RCHRES	4		COPY	1	17
RCHRES	3	1	RCHRES	10	7
RCHRES	3		COPY	1	17
RCHRES	3	1	RCHRES	4	8
PERLND	4	0.29	COPY	1	12
IMPLND	3	0.07	COPY	1	15
IMPLND	14	0.15	COPY	1	15
PERLND	4	0.29	COPY	1	13
RCHRES	6	1	RCHRES	11	7
RCHRES	6		COPY	1	17
RCHRES	5	1	RCHRES	11	7
RCHRES	5		COPY	1	17
RCHRES	5	1	RCHRES	6	8
PERLND	2	2.46	COPY	1	12
IMPLND	3	0.08	COPY	1	15
IMPLND	14	0.36	COPY	1	15
PERLND	2	2.46	COPY	1	13
RCHRES	7	1	RCHRES	8	8
PERLND	6	0.36	COPY	1	12
IMPLND	3	0.03	COPY	1	15
IMPLND	14	0.41	COPY	1	15
PERLND	6	0.36	COPY	1	13

```

RCHRES 9 1 COPY 501 17
RCHRES 10 1 COPY 501 17
RCHRES 11 1 COPY 501 17
RCHRES 8 1 COPY 501 17
RCHRES 7 1 COPY 501 17
END SCHEMATIC

```

NETWORK

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # #<-factor->strg <Name> # # <Name> # # ***
COPY 501 OUTPUT MEAN 1 1 48.4 DISPLY 1 INPUT TIMSER 1

```

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # #<-factor->strg <Name> # # <Name> # # ***
END NETWORK

```

RCHRES

GEN-INFO

RCHRES	Name	Nexits	Unit	Systems	Printer			
# - #	<----->	<---->	User	T-series	Engl	Metr	LKFG	
			in	out				
1	Surface retentio-010	3	1	1 1	28	0	1	
2	Bioretention 1	2	1	1 1	28	0	1	
3	Surface retentio-013	3	1	1 1	28	0	1	
4	Bioretention 2	2	1	1 1	28	0	1	
5	Surface retentio-017	3	1	1 1	28	0	1	
6	Bioretention 3	2	1	1 1	28	0	1	
7	Surface retentio-021	3	1	1 1	28	0	1	
8	Bioretention 4	2	1	1 1	28	0	1	
9	Trapezoidal Pond-011	2	1	1 1	28	0	1	
10	Trapezoidal Pond-014	2	1	1 1	28	0	1	
11	Trapezoidal Pond-019	2	1	1 1	28	0	1	

END GEN-INFO

*** Section RCHRES***

ACTIVITY

```

<PLS > ***** Active Sections *****
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUGF PKFG PHFG ***
1 1 0 0 0 0 0 0 0 0 0 0
2 1 0 0 0 0 0 0 0 0 0 0
3 1 0 0 0 0 0 0 0 0 0 0
4 1 0 0 0 0 0 0 0 0 0 0
5 1 0 0 0 0 0 0 0 0 0 0
6 1 0 0 0 0 0 0 0 0 0 0
7 1 0 0 0 0 0 0 0 0 0 0
8 1 0 0 0 0 0 0 0 0 0 0
9 1 0 0 0 0 0 0 0 0 0 0
10 1 0 0 0 0 0 0 0 0 0 0
11 1 0 0 0 0 0 0 0 0 0 0

```

END ACTIVITY

PRINT-INFO

```

<PLS > ***** Print-flags ***** PIVL PYR *****
# - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PHCB PIVL PYR *****
1 4 0 0 0 0 0 0 0 0 0 1 9
2 4 0 0 0 0 0 0 0 0 0 1 9
3 4 0 0 0 0 0 0 0 0 0 1 9
4 4 0 0 0 0 0 0 0 0 0 1 9
5 4 0 0 0 0 0 0 0 0 0 1 9
6 4 0 0 0 0 0 0 0 0 0 1 9
7 4 0 0 0 0 0 0 0 0 0 1 9
8 4 0 0 0 0 0 0 0 0 0 1 9
9 4 0 0 0 0 0 0 0 0 0 1 9
10 4 0 0 0 0 0 0 0 0 0 1 9
11 4 0 0 0 0 0 0 0 0 0 1 9

```

END PRINT-INFO

HYDR-PARM1


```

RCHRES   Flags for each HYDR Section                                     ***
# - #    VC A1 A2 A3  ODFVFG for each *** ODGTFG for each   FUNCT for each
          FG FG FG FG  possible exit  *** possible exit   possible exit
          * * * *   * * * *   * * * *   * * * *
1         0 1 0 0    4 5 6 0 0    0 0 0 0 0    2 2 2 2 2
2         0 1 0 0    4 5 0 0 0    0 0 0 0 0    2 2 2 2 2
3         0 1 0 0    4 5 6 0 0    0 0 0 0 0    2 2 2 2 2
4         0 1 0 0    4 5 0 0 0    0 0 0 0 0    2 2 2 2 2
5         0 1 0 0    4 5 6 0 0    0 0 0 0 0    2 2 2 2 2
6         0 1 0 0    4 5 0 0 0    0 0 0 0 0    2 2 2 2 2
7         0 1 0 0    4 5 6 0 0    0 0 0 0 0    2 2 2 2 2
8         0 1 0 0    4 5 0 0 0    0 0 0 0 0    2 2 2 2 2
9         0 1 0 0    4 5 0 0 0    0 0 0 0 0    2 2 2 2 2
10        0 1 0 0    4 5 0 0 0    0 0 0 0 0    2 2 2 2 2
11        0 1 0 0    4 5 0 0 0    0 0 0 0 0    2 2 2 2 2
END HYDR-PARM1

```

```

HYDR-PARM2
# - #    FTABNO      LEN      DELTH      STCOR      KS      DB50      ***
<-----><-----><-----><-----><-----><-----><----->
1         1         0.01      0.0      212.0      0.0      0.0
2         2         0.01      0.0      212.0      0.0      0.0
3         3         0.01      0.0      221.5      0.0      0.0
4         4         0.01      0.0      221.5      0.0      0.0
5         5         0.01      0.0      0.0        0.0      0.0
6         6         0.03      0.0      0.0        0.0      0.0
7         7         0.01      0.0      215.0      0.0      0.0
8         8         0.01      0.0      215.0      0.0      0.0
9         9         0.02      0.0      209.0      0.5      0.0
10        10        0.01      0.0      220.0      0.5      0.0
11        11        0.02      0.0      215.0      0.5      0.0
END HYDR-PARM2

```

```

HYDR-INIT
RCHRES   Initial conditions for each HYDR section                       ***
# - #    *** VOL      Initial value of COLIND      Initial value of OUTDGT
          *** ac-ft    for each possible exit    for each possible exit
<-----><-----><-----><-----><-----><-----><-----><----->
1         0         4.0 5.0 6.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
2         0         4.0 5.0 0.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
3         0         4.0 5.0 6.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
4         0         4.0 5.0 0.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
5         0         4.0 5.0 6.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
6         0         4.0 5.0 0.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
7         0         4.0 5.0 6.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
8         0         4.0 5.0 0.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
9         0         4.0 5.0 0.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
10        0         4.0 5.0 0.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
11        0         4.0 5.0 0.0 0.0 0.0    0.0 0.0 0.0 0.0 0.0
END HYDR-INIT
END RCHRES

```

```

SPEC-ACTIONS
END SPEC-ACTIONS
FTABLES

```

```

FTABLE      2
44      5
Depth      Area      Volume      Outflow1      Outflow2      Velocity      Travel Time***
(ft)      (acres)      (acre-ft)      (cfs)      (cfs)      (ft/sec)      (Minutes)***
0.000000  0.110864  0.000000  0.000000  0.000000
0.041209  0.110496  0.001512  0.000000  0.000000
0.082418  0.109710  0.003036  0.000000  0.000000
0.123626  0.108927  0.004574  0.000000  0.000000
0.164835  0.108146  0.006124  0.000000  0.000000
0.206044  0.107368  0.007686  0.000000  0.000000
0.247253  0.106593  0.009262  0.000000  0.001723
0.288462  0.105821  0.010851  0.000000  0.002663
0.329670  0.105051  0.012452  0.000000  0.003883
0.370879  0.104285  0.014067  0.000000  0.005418
0.412088  0.103521  0.015695  0.000000  0.007298
0.453297  0.102760  0.017336  0.000000  0.009553

```

0.494505	0.102002	0.018990	0.000000	0.012215
0.535714	0.101246	0.020658	0.000000	0.015312
0.576923	0.100494	0.022338	0.000000	0.018874
0.618132	0.099744	0.024032	0.000000	0.022929
0.659341	0.098997	0.025740	0.000000	0.027506
0.700549	0.098253	0.027461	0.000000	0.029418
0.741758	0.097512	0.029195	0.000000	0.032890
0.782967	0.096773	0.030943	0.000000	0.038640
0.824176	0.096037	0.032705	0.000000	0.045003
0.865385	0.095304	0.034480	0.000000	0.052007
0.906593	0.094574	0.036270	0.000000	0.059682
0.947802	0.093847	0.038073	0.000000	0.067089
0.989011	0.093122	0.039889	0.000000	0.068576
1.030220	0.092401	0.041720	0.000000	0.077745
1.071429	0.091682	0.043564	0.000000	0.087676
1.112637	0.090966	0.045423	0.000000	0.098399
1.153846	0.090252	0.047296	0.000000	0.109943
1.195055	0.089542	0.049182	0.000000	0.122337
1.236264	0.088834	0.051083	0.000000	0.129024
1.277473	0.088129	0.052998	0.000000	0.136622
1.318681	0.087427	0.054928	0.000000	0.150906
1.359890	0.086728	0.056871	0.000000	0.166134
1.401099	0.086032	0.058829	0.000000	0.182334
1.442308	0.085338	0.060802	0.000000	0.199533
1.483516	0.084647	0.062789	0.000000	0.217758
1.524725	0.083959	0.064790	0.000000	0.220568
1.565934	0.083274	0.066806	0.000000	0.238755
1.607143	0.082591	0.068837	0.000000	0.259235
1.648352	0.081912	0.070882	0.000000	0.280791
1.689560	0.081235	0.072942	0.000000	0.303390
1.730769	0.080561	0.075017	0.000000	0.326797
1.750000	0.079890	0.091003	0.000000	0.726623

END FTABLE 2
 FTABLE 1

50 6

Time***	Depth (ft) (Minutes)***	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Outflow3 (cfs)	Velocity (ft/sec)	Travel
0.000000	0.079890	0.000000	0.000000	0.000000	0.000000	0.007970		
0.041209	0.111655	0.004585	0.000000	0.288584	0.007970			
0.082418	0.112448	0.009202	0.000000	0.295223	0.015968			
0.123626	0.113244	0.013853	0.000000	0.301862	0.023994			
0.164835	0.114043	0.018536	0.000000	0.308501	0.032049			
0.206044	0.114844	0.023252	0.000000	0.315141	0.040132			
0.247253	0.115649	0.028001	0.000000	0.321780	0.048243			
0.288462	0.116456	0.032783	0.000000	0.328419	0.056383			
0.329670	0.117266	0.037599	0.000000	0.335058	0.064550			
0.370879	0.118079	0.042448	0.000000	0.341697	0.072747			
0.412088	0.118895	0.047331	0.000000	0.348337	0.080971			
0.453297	0.119713	0.052247	0.000000	0.354976	0.089224			
0.494505	0.120534	0.057197	0.000000	0.361615	0.097505			
0.535714	0.121358	0.062181	0.000000	0.368254	0.105814			
0.576923	0.122185	0.067200	0.000000	0.374893	0.114152			
0.618132	0.123015	0.072252	0.000000	0.381533	0.122518			
0.659341	0.123847	0.077338	0.000000	0.388172	0.130912			
0.700549	0.124683	0.082459	0.000000	0.394811	0.139335			
0.741758	0.125521	0.087614	0.000000	0.401450	0.147786			
0.782967	0.126362	0.092804	0.000000	0.408089	0.156265			
0.824176	0.127205	0.098029	0.000000	0.414729	0.164772			
0.865385	0.128052	0.103288	0.000000	0.421368	0.173308			
0.906593	0.128901	0.108583	0.000000	0.428007	0.181872			
0.947802	0.129753	0.113912	0.000000	0.434646	0.190465			
0.989011	0.130608	0.119277	0.000000	0.441285	0.199085			
1.030220	0.131466	0.124676	0.000000	0.447925	0.207734			
1.071429	0.132327	0.130112	0.000000	0.454564	0.216412			
1.112637	0.133190	0.135583	0.000000	0.461203	0.225117			
1.153846	0.134056	0.141089	0.000000	0.467842	0.233851			
1.195055	0.134925	0.146631	0.000000	0.474481	0.242613			
1.236264	0.135797	0.152209	0.000000	0.481121	0.251404			

1.277473	0.136672	0.157823	0.000000	0.487760	0.260223
1.318681	0.137549	0.163473	0.000000	0.494399	0.269070
1.359890	0.138429	0.169160	0.000000	0.501038	0.277945
1.401099	0.139312	0.174882	0.000000	0.507677	0.286849
1.442308	0.140198	0.180642	0.000000	0.514317	0.295781
1.483516	0.141087	0.186437	0.000000	0.520956	0.304741
1.524725	0.141978	0.192270	0.165124	0.527595	0.313730
1.565934	0.142872	0.198139	0.718678	0.534234	0.322747
1.607143	0.143769	0.204045	1.488081	0.540873	0.331792
1.648352	0.144669	0.209988	2.423511	0.547513	0.340866
1.689560	0.145572	0.215968	3.498944	0.554152	0.349967
1.730769	0.146477	0.221986	4.697313	0.560791	0.359098
1.771978	0.147386	0.228041	6.005982	0.567430	0.368256
1.813187	0.148297	0.234133	7.414796	0.574069	0.377443
1.854396	0.149211	0.240263	8.915080	0.580709	0.386658
1.895604	0.150127	0.246431	10.49906	0.587348	0.395901
1.936813	0.151047	0.252636	12.15951	0.593987	0.405173
1.978022	0.151969	0.258880	13.88954	0.600626	0.414473
2.000000	0.152462	0.262225	15.68240	0.604167	0.419444

END FTABLE 1

FTABLE 9

91 5

Depth (ft)	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Velocity (ft/sec)	Travel Time*** (Minutes)***
0.000000	0.175712	0.000000	0.000000	0.000000		
0.066667	0.177997	0.011790	0.000000	1.771759		
0.133333	0.180295	0.023733	0.000000	1.771759		
0.200000	0.182606	0.035830	0.000000	1.771759		
0.266667	0.184929	0.048081	0.000000	1.771759		
0.333333	0.187265	0.060488	0.000000	1.771759		
0.400000	0.189614	0.073050	0.000000	1.771759		
0.466667	0.191975	0.085770	0.000000	1.771759		
0.533333	0.194349	0.098648	0.000000	1.771759		
0.600000	0.196735	0.111684	0.000000	1.771759		
0.666667	0.199134	0.124879	0.000000	1.771759		
0.733333	0.201546	0.138235	0.000000	1.771759		
0.800000	0.203970	0.151753	0.000000	1.771759		
0.866667	0.206407	0.165432	0.000000	1.771759		
0.933333	0.208857	0.179274	0.000000	1.771759		
1.000000	0.211319	0.193280	0.000000	1.771759		
1.066667	0.213793	0.207450	0.000000	1.771759		
1.133333	0.216281	0.221786	0.000000	1.771759		
1.200000	0.218781	0.236288	0.000000	1.771759		
1.266667	0.221293	0.250957	0.000000	1.771759		
1.333333	0.223818	0.265794	0.000000	1.771759		
1.400000	0.226356	0.280800	0.000000	1.771759		
1.466667	0.228906	0.295975	0.000000	1.771759		
1.533333	0.231469	0.311321	0.000000	1.771759		
1.600000	0.234045	0.326838	0.000000	1.771759		
1.666667	0.236633	0.342528	0.000000	1.771759		
1.733333	0.239234	0.358390	0.000000	1.771759		
1.800000	0.241847	0.374426	0.000000	1.771759		
1.866667	0.244473	0.390637	0.000000	1.771759		
1.933333	0.247112	0.407023	0.000000	1.771759		
2.000000	0.249763	0.423585	0.000000	1.771759		
2.066667	0.252427	0.440325	0.000000	1.771759		
2.133333	0.255103	0.457243	0.000000	1.771759		
2.200000	0.257792	0.474339	0.000000	1.771759		
2.266667	0.260494	0.491615	0.000000	1.771759		
2.333333	0.263208	0.509072	0.000000	1.771759		
2.400000	0.265935	0.526710	0.000000	1.771759		
2.466667	0.268675	0.544531	0.000000	1.771759		
2.533333	0.271427	0.562534	0.000000	1.771759		
2.600000	0.274192	0.580721	0.000000	1.771759		
2.666667	0.276969	0.599093	0.000000	1.771759		
2.733333	0.279759	0.617651	0.000000	1.771759		
2.800000	0.282562	0.636395	0.000000	1.771759		
2.866667	0.285377	0.655326	0.000000	1.771759		
2.933333	0.288205	0.674446	0.000000	1.771759		
3.000000	0.291045	0.693754	0.000000	1.771759		

3.066667	0.293898	0.713252	0.000000	1.771759
3.133333	0.296764	0.732941	0.000000	1.771759
3.200000	0.299642	0.752821	0.000000	1.771759
3.266667	0.302533	0.772893	0.000000	1.771759
3.333333	0.305436	0.793159	0.000000	1.771759
3.400000	0.308352	0.813619	0.000000	1.771759
3.466667	0.311281	0.834273	0.000000	1.771759
3.533333	0.314222	0.855123	0.000000	1.771759
3.600000	0.317176	0.876170	0.000000	1.771759
3.666667	0.320143	0.897414	0.000000	1.771759
3.733333	0.323122	0.918856	0.000000	1.771759
3.800000	0.326113	0.940497	0.000000	1.771759
3.866667	0.329118	0.962338	0.000000	1.771759
3.933333	0.332135	0.984380	0.000000	1.771759
4.000000	0.335164	1.006623	0.000000	1.771759
4.066667	0.338207	1.029069	0.000000	1.771759
4.133333	0.341261	1.051718	0.000000	1.771759
4.200000	0.344329	1.074571	0.000000	1.771759
4.266667	0.347409	1.097629	0.000000	1.771759
4.333333	0.350501	1.120892	0.000000	1.771759
4.400000	0.353607	1.144363	0.000000	1.771759
4.466667	0.356724	1.168040	0.000000	1.771759
4.533333	0.359855	1.191926	0.000000	1.771759
4.600000	0.362998	1.216021	0.000000	1.771759
4.666667	0.366154	1.240326	0.000000	1.771759
4.733333	0.369322	1.264842	0.000000	1.771759
4.800000	0.372503	1.289570	0.000000	1.771759
4.866667	0.375696	1.314510	0.000000	1.771759
4.933333	0.378902	1.339663	0.000000	1.771759
5.000000	0.382121	1.365031	0.000000	1.771759
5.066667	0.385353	1.390613	0.182234	1.771759
5.133333	0.388597	1.416411	0.509662	1.771759
5.200000	0.391853	1.442426	0.907676	1.771759
5.266667	0.395122	1.468659	1.318080	1.771759
5.333333	0.398404	1.495110	1.683468	1.771759
5.400000	0.401698	1.521780	1.960035	1.771759
5.466667	0.405005	1.548670	2.138326	1.771759
5.533333	0.408325	1.575781	2.300165	1.771759
5.600000	0.411657	1.603114	2.439693	1.771759
5.666667	0.415002	1.630669	2.571662	1.771759
5.733333	0.418360	1.658448	2.697182	1.771759
5.800000	0.421730	1.686451	2.817115	1.771759
5.866667	0.425112	1.714679	2.932146	1.771759
5.933333	0.428508	1.743133	3.042832	1.771759
6.000000	0.431916	1.771813	3.149630	1.771759

END FTABLE 9

FTABLE 4

44 5

Depth (ft)	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Velocity (ft/sec)	Travel Time*** (Minutes)***
0.000000	0.018681	0.000000	0.000000	0.000000		
0.041209	0.018501	0.000093	0.000000	0.000000		
0.082418	0.018118	0.000192	0.000000	0.000000		
0.123626	0.017738	0.000295	0.000000	0.000000		
0.164835	0.017360	0.000404	0.000000	0.000000		
0.206044	0.016985	0.000518	0.000000	0.000000		
0.247253	0.016613	0.000638	0.000000	0.000133		
0.288462	0.016244	0.000762	0.000000	0.000213		
0.329670	0.015877	0.000892	0.000000	0.000321		
0.370879	0.015514	0.001028	0.000000	0.000462		
0.412088	0.015153	0.001169	0.000000	0.000642		
0.453297	0.014795	0.001315	0.000000	0.000866		
0.494505	0.014440	0.001467	0.000000	0.001140		
0.535714	0.014087	0.001625	0.000000	0.001470		
0.576923	0.013738	0.001789	0.000000	0.001861		
0.618132	0.013391	0.001958	0.000000	0.002322		
0.659341	0.013047	0.002133	0.000000	0.002857		
0.700549	0.012706	0.002314	0.000000	0.003132		
0.741758	0.012368	0.002501	0.000000	0.003587		
0.782967	0.012032	0.002694	0.000000	0.004314		

0.824176	0.011699	0.002893	0.000000	0.005140
0.865385	0.011369	0.003098	0.000000	0.006072
0.906593	0.011042	0.003309	0.000000	0.007120
0.947802	0.010718	0.003527	0.000000	0.008173
0.989011	0.010396	0.003750	0.000000	0.008526
1.030220	0.010078	0.003980	0.000000	0.009861
1.071429	0.009762	0.004216	0.000000	0.011338
1.112637	0.009449	0.004459	0.000000	0.012968
1.153846	0.009138	0.004708	0.000000	0.014760
1.195055	0.008831	0.004964	0.000000	0.016724
1.236264	0.008526	0.005226	0.000000	0.017952
1.277473	0.008224	0.005495	0.000000	0.019341
1.318681	0.007925	0.005770	0.000000	0.021727
1.359890	0.007629	0.006052	0.000000	0.024318
1.401099	0.007336	0.006341	0.000000	0.027125
1.442308	0.007045	0.006637	0.000000	0.030157
1.483516	0.006757	0.006940	0.000000	0.033427
1.524725	0.006472	0.007249	0.000000	0.034377
1.565934	0.006190	0.007566	0.000000	0.037770
1.607143	0.005911	0.007890	0.000000	0.041613
1.648352	0.005634	0.008220	0.000000	0.045724
1.689560	0.005360	0.008558	0.000000	0.050103
1.730769	0.005089	0.008903	0.000000	0.054718
1.750000	0.004821	0.011596	0.000000	0.122439

END FTABLE 4
 FTABLE 3

50	6							
Time***	Depth	Area	Volume	Outflow1	Outflow2	Outflow3	Velocity	Travel
(Minutes)***	(ft)	(acres)	(acre-ft)	(cfs)	(cfs)	(cfs)	(ft/sec)	
0.000000	0.004821	0.000000	0.000000	0.000000	0.000000	0.003906		
0.041209	0.019069	0.000778	0.000000	0.000000	0.017415	0.003906		
0.082418	0.019459	0.001572	0.000000	0.000000	0.017815	0.007840		
0.123626	0.019852	0.002382	0.000000	0.000000	0.018216	0.011803		
0.164835	0.020247	0.003208	0.000000	0.000000	0.018616	0.015794		
0.206044	0.020646	0.004050	0.000000	0.000000	0.019017	0.019813		
0.247253	0.021048	0.004909	0.000000	0.000000	0.019418	0.023861		
0.288462	0.021452	0.005785	0.000000	0.000000	0.019818	0.027937		
0.329670	0.021859	0.006678	0.000000	0.000000	0.020219	0.032041		
0.370879	0.022269	0.007587	0.000000	0.000000	0.020620	0.036174		
0.412088	0.022681	0.008513	0.000000	0.000000	0.021020	0.040335		
0.453297	0.023097	0.009456	0.000000	0.000000	0.021421	0.044524		
0.494505	0.023515	0.010417	0.000000	0.000000	0.021822	0.048741		
0.535714	0.023936	0.011394	0.000000	0.000000	0.022222	0.052987		
0.576923	0.024360	0.012389	0.000000	0.000000	0.022623	0.057261		
0.618132	0.024787	0.013402	0.000000	0.000000	0.023024	0.061563		
0.659341	0.025216	0.014432	0.000000	0.000000	0.023424	0.065894		
0.700549	0.025648	0.015480	0.000000	0.000000	0.023825	0.070253		
0.741758	0.026083	0.016546	0.000000	0.000000	0.024225	0.074640		
0.782967	0.026521	0.017630	0.000000	0.000000	0.024626	0.079056		
0.824176	0.026962	0.018732	0.000000	0.000000	0.025027	0.083499		
0.865385	0.027406	0.019852	0.000000	0.000000	0.025427	0.087972		
0.906593	0.027852	0.020991	0.000000	0.000000	0.025828	0.092472		
0.947802	0.028301	0.022148	0.000000	0.000000	0.026229	0.097001		
0.989011	0.028753	0.023323	0.000000	0.000000	0.026629	0.101558		
1.030220	0.029208	0.024518	0.000000	0.000000	0.027030	0.106143		
1.071429	0.029665	0.025731	0.000000	0.000000	0.027431	0.110757		
1.112637	0.030126	0.026963	0.000000	0.000000	0.027831	0.115399		
1.153846	0.030589	0.028214	0.000000	0.000000	0.028232	0.120069		
1.195055	0.031055	0.029484	0.000000	0.000000	0.028633	0.124768		
1.236264	0.031524	0.030773	0.000000	0.000000	0.029033	0.129494		
1.277473	0.031995	0.032082	0.000000	0.000000	0.029434	0.134250		
1.318681	0.032470	0.033410	0.000000	0.000000	0.029834	0.139033		
1.359890	0.032947	0.034758	0.000000	0.000000	0.030235	0.143845		
1.401099	0.033427	0.036126	0.000000	0.000000	0.030636	0.148685		
1.442308	0.033910	0.037513	0.000000	0.000000	0.031036	0.153553		
1.483516	0.034395	0.038920	0.000000	0.000000	0.031437	0.158450		
1.524725	0.034884	0.040348	0.082535	0.031838	0.163375			
1.565934	0.035375	0.041796	0.359097	0.032238	0.168328			

1.607143	0.035869	0.043263	0.743185	0.032639	0.173310
1.648352	0.036366	0.044752	1.209123	0.033040	0.178320
1.689560	0.036865	0.046261	1.742293	0.033440	0.183358
1.730769	0.037368	0.047790	2.331616	0.033841	0.188425
1.771978	0.037873	0.049340	2.967286	0.034241	0.193519
1.813187	0.038381	0.050912	3.639843	0.034642	0.198642
1.854396	0.038892	0.052504	4.339783	0.035043	0.203794
1.895604	0.039406	0.054117	5.057410	0.035443	0.208974
1.936813	0.039922	0.055752	5.782840	0.035844	0.214182
1.978022	0.040442	0.057407	6.506103	0.036245	0.219418
2.000000	0.040720	0.058299	7.217303	0.036458	0.222222

END FTABLE 3

FTABLE 10

91 5

Depth (ft)	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Velocity (ft/sec)	Travel Time*** (Minutes)***
0.000000	0.061065	0.000000	0.000000	0.000000		
0.044444	0.061879	0.002732	0.000000	0.615741		
0.088889	0.062696	0.005500	0.000000	0.615741		
0.133333	0.063518	0.008305	0.000000	0.615741		
0.177778	0.064344	0.011147	0.000000	0.615741		
0.222222	0.065174	0.014025	0.000000	0.615741		
0.266667	0.066009	0.016940	0.000000	0.615741		
0.311111	0.066847	0.019892	0.000000	0.615741		
0.355556	0.067690	0.022882	0.000000	0.615741		
0.400000	0.068537	0.025909	0.000000	0.615741		
0.444444	0.069388	0.028974	0.000000	0.615741		
0.488889	0.070243	0.032077	0.000000	0.615741		
0.533333	0.071103	0.035218	0.000000	0.615741		
0.577778	0.071967	0.038398	0.000000	0.615741		
0.622222	0.072835	0.041615	0.000000	0.615741		
0.666667	0.073707	0.044872	0.000000	0.615741		
0.711111	0.074583	0.048167	0.000000	0.615741		
0.755556	0.075464	0.051502	0.000000	0.615741		
0.800000	0.076348	0.054875	0.000000	0.615741		
0.844444	0.077237	0.058288	0.000000	0.615741		
0.888889	0.078130	0.061741	0.000000	0.615741		
0.933333	0.079027	0.065233	0.000000	0.615741		
0.977778	0.079929	0.068765	0.000000	0.615741		
1.022222	0.080834	0.072338	0.000000	0.615741		
1.066667	0.081744	0.075951	0.000000	0.615741		
1.111111	0.082658	0.079604	0.000000	0.615741		
1.155556	0.083576	0.083298	0.000000	0.615741		
1.200000	0.084499	0.087033	0.000000	0.615741		
1.244444	0.085425	0.090809	0.000000	0.615741		
1.288889	0.086356	0.094627	0.000000	0.615741		
1.333333	0.087291	0.098486	0.000000	0.615741		
1.377778	0.088230	0.102386	0.000000	0.615741		
1.422222	0.089174	0.106328	0.000000	0.615741		
1.466667	0.090121	0.110313	0.000000	0.615741		
1.511111	0.091073	0.114339	0.000000	0.615741		
1.555556	0.092029	0.118408	0.000000	0.615741		
1.600000	0.092989	0.122520	0.000000	0.615741		
1.644444	0.093953	0.126674	0.000000	0.615741		
1.688889	0.094922	0.130871	0.000000	0.615741		
1.733333	0.095894	0.135112	0.000000	0.615741		
1.777778	0.096871	0.139395	0.000000	0.615741		
1.822222	0.097852	0.143722	0.000000	0.615741		
1.866667	0.098837	0.148093	0.000000	0.615741		
1.911111	0.099827	0.152508	0.000000	0.615741		
1.955556	0.100820	0.156967	0.000000	0.615741		
2.000000	0.101818	0.161470	0.000000	0.615741		
2.044444	0.102820	0.166017	0.000000	0.615741		
2.088889	0.103826	0.170610	0.000000	0.615741		
2.133333	0.104837	0.175247	0.000000	0.615741		
2.177778	0.105851	0.179929	0.000000	0.615741		
2.222222	0.106870	0.184656	0.000000	0.615741		
2.266667	0.107893	0.189428	0.000000	0.615741		
2.311111	0.108920	0.194246	0.000000	0.615741		
2.355556	0.109951	0.199110	0.000000	0.615741		

2.400000	0.110987	0.204020	0.000000	0.615741
2.444444	0.112027	0.208976	0.000000	0.615741
2.488889	0.113071	0.213978	0.000000	0.615741
2.533333	0.114119	0.219026	0.000000	0.615741
2.577778	0.115171	0.224122	0.000000	0.615741
2.622222	0.116227	0.229264	0.000000	0.615741
2.666667	0.117288	0.234453	0.000000	0.615741
2.711111	0.118353	0.239690	0.000000	0.615741
2.755556	0.119422	0.244974	0.000000	0.615741
2.800000	0.120495	0.250305	0.000000	0.615741
2.844444	0.121573	0.255684	0.000000	0.615741
2.888889	0.122654	0.261112	0.000000	0.615741
2.933333	0.123740	0.266587	0.000000	0.615741
2.977778	0.124830	0.272111	0.000000	0.615741
3.022222	0.125924	0.277683	0.035147	0.615741
3.066667	0.127022	0.283304	0.182234	0.615741
3.111111	0.128125	0.288974	0.389839	0.615741
3.155556	0.129232	0.294693	0.637321	0.615741
3.200000	0.130343	0.300461	0.907676	0.615741
3.244444	0.131458	0.306279	1.183559	0.615741
3.288889	0.132577	0.312147	1.447495	0.615741
3.333333	0.133701	0.318064	1.683468	0.615741
3.377778	0.134828	0.324031	1.879270	0.615741
3.422222	0.135960	0.330049	2.029388	0.615741
3.466667	0.137096	0.336117	2.138326	0.615741
3.511111	0.138237	0.342235	2.251735	0.615741
3.555556	0.139381	0.348404	2.347596	0.615741
3.600000	0.140530	0.354625	2.439693	0.615741
3.644444	0.141682	0.360896	2.528438	0.615741
3.688889	0.142840	0.367219	2.614172	0.615741
3.733333	0.144001	0.373593	2.697182	0.615741
3.777778	0.145166	0.380019	2.777713	0.615741
3.822222	0.146336	0.386497	2.855973	0.615741
3.866667	0.147510	0.393027	2.932146	0.615741
3.911111	0.148687	0.399609	3.006389	0.615741
3.955556	0.149870	0.406243	3.078843	0.615741
4.000000	0.151056	0.412931	3.149630	0.615741

END FTABLE 10

FTABLE 6

44 5

Depth (ft)	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Velocity (ft/sec)	Travel Time*** (Minutes)***
0.000000	0.103722	0.000000	0.000000	0.000000		
0.041209	0.103212	0.001136	0.000000	0.000000		
0.082418	0.102120	0.002290	0.000000	0.000000		
0.123626	0.101032	0.003463	0.000000	0.000000		
0.164835	0.099946	0.004654	0.000000	0.000000		
0.206044	0.098862	0.005864	0.000000	0.000000		
0.247253	0.097782	0.007092	0.000000	0.001348		
0.288462	0.096704	0.008339	0.000000	0.002097		
0.329670	0.095629	0.009605	0.000000	0.003079		
0.370879	0.094557	0.010890	0.000000	0.004324		
0.412088	0.093488	0.012193	0.000000	0.005862		
0.453297	0.092422	0.013515	0.000000	0.007723		
0.494505	0.091358	0.014857	0.000000	0.009936		
0.535714	0.090298	0.016217	0.000000	0.012530		
0.576923	0.089240	0.017597	0.000000	0.015536		
0.618132	0.088184	0.018995	0.000000	0.018984		
0.659341	0.087132	0.020413	0.000000	0.022903		
0.700549	0.086083	0.021850	0.000000	0.024631		
0.741758	0.085036	0.023306	0.000000	0.027688		
0.782967	0.083992	0.024782	0.000000	0.032703		
0.824176	0.082951	0.026277	0.000000	0.038288		
0.865385	0.081912	0.027791	0.000000	0.044475		
0.906593	0.080877	0.029325	0.000000	0.051295		
0.947802	0.079844	0.030878	0.000000	0.057947		
0.989011	0.078814	0.032451	0.000000	0.059519		
1.030220	0.077787	0.034044	0.000000	0.067798		
1.071429	0.076763	0.035656	0.000000	0.076816		
1.112637	0.075741	0.037288	0.000000	0.086606		

1.153846	0.074723	0.038940	0.000000	0.097201
1.195055	0.073707	0.040612	0.000000	0.108636
1.236264	0.072694	0.042304	0.000000	0.115071
1.277473	0.071684	0.044015	0.000000	0.122366
1.318681	0.070676	0.045747	0.000000	0.135724
1.359890	0.069672	0.047499	0.000000	0.150033
1.401099	0.068670	0.049271	0.000000	0.165326
1.442308	0.067671	0.051063	0.000000	0.181637
1.483516	0.066675	0.052875	0.000000	0.198998
1.524725	0.065681	0.054707	0.000000	0.202335
1.565934	0.064691	0.056560	0.000000	0.219841
1.607143	0.063703	0.058433	0.000000	0.239578
1.648352	0.062718	0.060327	0.000000	0.260439
1.689560	0.061736	0.062241	0.000000	0.282401
1.730769	0.060756	0.064176	0.000000	0.305253
1.750000	0.059780	0.079132	0.000000	0.679814

END FTABLE 6

FTABLE 5

50 6

Time***	Depth (ft)	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Outflow3 (cfs)	Velocity (ft/sec)	Travel
(Minutes)***								
0.000000	0.059780	0.000000	0.000000	0.000000	0.000000	0.011049		
0.041209	0.104818	0.004297	0.000000	0.000000	0.215940	0.011049		
0.082418	0.105917	0.008639	0.000000	0.000000	0.220908	0.022126		
0.123626	0.107018	0.013026	0.000000	0.000000	0.225876	0.033232		
0.164835	0.108122	0.017459	0.000000	0.000000	0.230844	0.044366		
0.206044	0.109229	0.021938	0.000000	0.000000	0.235812	0.055528		
0.247253	0.110339	0.026462	0.000000	0.000000	0.240780	0.066718		
0.288462	0.111452	0.031032	0.000000	0.000000	0.245748	0.077937		
0.329670	0.112567	0.035647	0.000000	0.000000	0.250716	0.089184		
0.370879	0.113685	0.040309	0.000000	0.000000	0.255684	0.100459		
0.412088	0.114806	0.045017	0.000000	0.000000	0.260652	0.111763		
0.453297	0.115930	0.049771	0.000000	0.000000	0.265620	0.123095		
0.494505	0.117057	0.054572	0.000000	0.000000	0.270588	0.134455		
0.535714	0.118186	0.059419	0.000000	0.000000	0.275556	0.145844		
0.576923	0.119319	0.064312	0.000000	0.000000	0.280524	0.157261		
0.618132	0.120454	0.069253	0.000000	0.000000	0.285492	0.168706		
0.659341	0.121591	0.074240	0.000000	0.000000	0.290460	0.180180		
0.700549	0.122732	0.079274	0.000000	0.000000	0.295428	0.191681		
0.741758	0.123876	0.084355	0.000000	0.000000	0.300396	0.203211		
0.782967	0.125022	0.089484	0.000000	0.000000	0.305363	0.214770		
0.824176	0.126171	0.094659	0.000000	0.000000	0.310331	0.226357		
0.865385	0.127323	0.099882	0.000000	0.000000	0.315299	0.237972		
0.906593	0.128478	0.105153	0.000000	0.000000	0.320267	0.249615		
0.947802	0.129635	0.110471	0.000000	0.000000	0.325235	0.261286		
0.989011	0.130795	0.115837	0.000000	0.000000	0.330203	0.272986		
1.030220	0.131959	0.121251	0.000000	0.000000	0.335171	0.284715		
1.071429	0.133125	0.126713	0.000000	0.000000	0.340139	0.296471		
1.112637	0.134293	0.132223	0.000000	0.000000	0.345107	0.308256		
1.153846	0.135465	0.137781	0.000000	0.000000	0.350075	0.320069		
1.195055	0.136639	0.143388	0.000000	0.000000	0.355043	0.331910		
1.236264	0.137816	0.149043	0.000000	0.000000	0.360011	0.343780		
1.277473	0.138996	0.154746	0.000000	0.000000	0.364979	0.355678		
1.318681	0.140179	0.160499	0.000000	0.000000	0.369947	0.367605		
1.359890	0.141365	0.166300	0.000000	0.000000	0.374915	0.379559		
1.401099	0.142553	0.172150	0.000000	0.000000	0.379883	0.391542		
1.442308	0.143744	0.178049	0.000000	0.000000	0.384851	0.403553		
1.483516	0.144938	0.183997	0.000000	0.000000	0.389819	0.415593		
1.524725	0.146135	0.189994	0.165124	0.165124	0.394787	0.427661		
1.565934	0.147335	0.196041	0.718678	0.718678	0.399755	0.439757		
1.607143	0.148537	0.202137	1.488081	1.488081	0.404723	0.451881		
1.648352	0.149742	0.208283	2.423511	2.423511	0.409690	0.464034		
1.689560	0.150950	0.214479	3.498944	3.498944	0.414658	0.476215		
1.730769	0.152161	0.220724	4.697313	4.697313	0.419626	0.488425		
1.771978	0.153375	0.227019	6.005982	6.005982	0.424594	0.500662		
1.813187	0.154591	0.233365	7.414796	7.414796	0.429562	0.512928		
1.854396	0.155811	0.239761	8.915080	8.915080	0.434530	0.525222		
1.895604	0.157033	0.246207	10.49906	10.49906	0.439498	0.537545		

1.936813 0.158258 0.252703 12.15951 0.444466 0.549896
 1.978022 0.159485 0.259250 13.88954 0.449434 0.562275
 2.000000 0.160141 0.262762 15.68240 0.452084 0.568889

END FTABLE 5
 FTABLE 11

91 5

Depth (ft)	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Velocity (ft/sec)	Travel Time*** (Minutes)***
0.000000	0.172176	0.000000	0.000000	0.000000		
0.066667	0.174048	0.011541	0.000000	1.736111		
0.133333	0.175929	0.023207	0.000000	1.736111		
0.200000	0.177820	0.034998	0.000000	1.736111		
0.266667	0.179720	0.046916	0.000000	1.736111		
0.333333	0.181630	0.058961	0.000000	1.736111		
0.400000	0.183549	0.071134	0.000000	1.736111		
0.466667	0.185477	0.083435	0.000000	1.736111		
0.533333	0.187415	0.095865	0.000000	1.736111		
0.600000	0.189363	0.108424	0.000000	1.736111		
0.666667	0.191319	0.121113	0.000000	1.736111		
0.733333	0.193285	0.133933	0.000000	1.736111		
0.800000	0.195261	0.146885	0.000000	1.736111		
0.866667	0.197246	0.159969	0.000000	1.736111		
0.933333	0.199240	0.173185	0.000000	1.736111		
1.000000	0.201244	0.186534	0.000000	1.736111		
1.066667	0.203258	0.200018	0.000000	1.736111		
1.133333	0.205280	0.213636	0.000000	1.736111		
1.200000	0.207312	0.227389	0.000000	1.736111		
1.266667	0.209354	0.241278	0.000000	1.736111		
1.333333	0.211405	0.255303	0.000000	1.736111		
1.400000	0.213465	0.269465	0.000000	1.736111		
1.466667	0.215535	0.283765	0.000000	1.736111		
1.533333	0.217615	0.298204	0.000000	1.736111		
1.600000	0.219703	0.312781	0.000000	1.736111		
1.666667	0.221801	0.327498	0.000000	1.736111		
1.733333	0.223909	0.342355	0.000000	1.736111		
1.800000	0.226026	0.357352	0.000000	1.736111		
1.866667	0.228152	0.372492	0.000000	1.736111		
1.933333	0.230288	0.387773	0.000000	1.736111		
2.000000	0.232433	0.403197	0.000000	1.736111		
2.066667	0.234588	0.418765	0.000000	1.736111		
2.133333	0.236752	0.434476	0.000000	1.736111		
2.200000	0.238926	0.450332	0.000000	1.736111		
2.266667	0.241109	0.466333	0.000000	1.736111		
2.333333	0.243301	0.482480	0.000000	1.736111		
2.400000	0.245503	0.498773	0.000000	1.736111		
2.466667	0.247714	0.515214	0.000000	1.736111		
2.533333	0.249935	0.531802	0.000000	1.736111		
2.600000	0.252165	0.548539	0.000000	1.736111		
2.666667	0.254405	0.565425	0.000000	1.736111		
2.733333	0.256654	0.582460	0.000000	1.736111		
2.800000	0.258912	0.599645	0.000000	1.736111		
2.866667	0.261180	0.616982	0.000000	1.736111		
2.933333	0.263457	0.634470	0.000000	1.736111		
3.000000	0.265744	0.652110	0.000000	1.736111		
3.066667	0.268040	0.669903	0.000000	1.736111		
3.133333	0.270345	0.687849	0.000000	1.736111		
3.200000	0.272660	0.705949	0.000000	1.736111		
3.266667	0.274985	0.724204	0.000000	1.736111		
3.333333	0.277319	0.742614	0.000000	1.736111		
3.400000	0.279662	0.761180	0.000000	1.736111		
3.466667	0.282015	0.779902	0.000000	1.736111		
3.533333	0.284377	0.798782	0.000000	1.736111		
3.600000	0.286748	0.817820	0.000000	1.736111		
3.666667	0.289129	0.837016	0.000000	1.736111		
3.733333	0.291520	0.856371	0.000000	1.736111		
3.800000	0.293919	0.875885	0.000000	1.736111		
3.866667	0.296329	0.895560	0.000000	1.736111		
3.933333	0.298747	0.915396	0.000000	1.736111		
4.000000	0.301175	0.935393	0.000000	1.736111		
4.066667	0.303613	0.955553	0.000000	1.736111		

4.133333	0.306060	0.975875	0.000000	1.736111
4.200000	0.308516	0.996361	0.000000	1.736111
4.266667	0.310982	1.017011	0.000000	1.736111
4.333333	0.313457	1.037826	0.000000	1.736111
4.400000	0.315942	1.058806	0.000000	1.736111
4.466667	0.318436	1.079952	0.000000	1.736111
4.533333	0.320940	1.101264	0.000000	1.736111
4.600000	0.323453	1.122744	0.000000	1.736111
4.666667	0.325975	1.144392	0.000000	1.736111
4.733333	0.328507	1.166208	0.000000	1.736111
4.800000	0.331048	1.188193	0.000000	1.736111
4.866667	0.333599	1.210348	0.000000	1.736111
4.933333	0.336159	1.232673	0.000000	1.736111
5.000000	0.338728	1.255169	0.000000	1.736111
5.066667	0.341307	1.277837	0.365093	1.736111
5.133333	0.343896	1.300677	1.030872	1.736111
5.200000	0.346493	1.323690	1.886677	1.736111
5.266667	0.349101	1.346877	2.883085	1.736111
5.333333	0.351717	1.370237	3.979211	1.736111
5.400000	0.354343	1.393773	5.134592	1.736111
5.466667	0.356979	1.417483	6.307592	1.736111
5.533333	0.359624	1.441370	7.456186	1.736111
5.600000	0.362278	1.465433	8.539994	1.736111
5.666667	0.364942	1.489674	9.523132	1.736111
5.733333	0.367615	1.514093	10.37768	1.736111
5.800000	0.370298	1.538690	11.08763	1.736111
5.866667	0.372990	1.563466	11.65332	1.736111
5.933333	0.375691	1.588422	12.09620	1.736111
6.000000	0.378402	1.613558	12.46394	1.736111

END FTABLE 11

FTABLE 8

44 5

Depth (ft)	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Velocity (ft/sec)	Travel Time*** (Minutes)***
0.000000	0.035823	0.000000	0.000000	0.000000		
0.041209	0.035607	0.000354	0.000000	0.000000		
0.082418	0.035145	0.000715	0.000000	0.000000		
0.123626	0.034686	0.001083	0.000000	0.000000		
0.164835	0.034230	0.001457	0.000000	0.000000		
0.206044	0.033777	0.001838	0.000000	0.000000		
0.247253	0.033327	0.002226	0.000000	0.000426		
0.288462	0.032879	0.002621	0.000000	0.000664		
0.329670	0.032434	0.003022	0.000000	0.000978		
0.370879	0.031992	0.003431	0.000000	0.001376		
0.412088	0.031553	0.003846	0.000000	0.001870		
0.453297	0.031117	0.004268	0.000000	0.002470		
0.494505	0.030683	0.004698	0.000000	0.003185		
0.535714	0.030253	0.005135	0.000000	0.004027		
0.576923	0.029825	0.005578	0.000000	0.005005		
0.618132	0.029400	0.006029	0.000000	0.006130		
0.659341	0.028977	0.006488	0.000000	0.007414		
0.700549	0.028558	0.006954	0.000000	0.007993		
0.741758	0.028141	0.007427	0.000000	0.009007		
0.782967	0.027727	0.007907	0.000000	0.010664		
0.824176	0.027316	0.008395	0.000000	0.012516		
0.865385	0.026908	0.008891	0.000000	0.014574		
0.906593	0.026503	0.009394	0.000000	0.016850		
0.947802	0.026100	0.009905	0.000000	0.019082		
0.989011	0.025700	0.010424	0.000000	0.019648		
1.030220	0.025303	0.010950	0.000000	0.022437		
1.071429	0.024909	0.011484	0.000000	0.025484		
1.112637	0.024517	0.012027	0.000000	0.028802		
1.153846	0.024129	0.012577	0.000000	0.032406		
1.195055	0.023743	0.013135	0.000000	0.036307		
1.236264	0.023360	0.013701	0.000000	0.038553		
1.277473	0.022980	0.014275	0.000000	0.041098		
1.318681	0.022602	0.014857	0.000000	0.045696		
1.359890	0.022228	0.015448	0.000000	0.050638		
1.401099	0.021856	0.016046	0.000000	0.055936		
1.442308	0.021487	0.016653	0.000000	0.061605		

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1.483516 0.021121 0.017269 0.000000 0.067659
1.524725 0.020758 0.017893 0.000000 0.068961
1.565934 0.020397 0.018525 0.000000 0.075110
1.607143 0.020039 0.019166 0.000000 0.082053
1.648352 0.019684 0.019815 0.000000 0.089414
1.689560 0.019332 0.020473 0.000000 0.097189
1.730769 0.018983 0.021140 0.000000 0.105308
1.750000 0.018636 0.026305 0.000000 0.234790

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END FTABLE 8
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FTABLE 7
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Time***	Depth (ft) (Minutes)***	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Outflow2 (cfs)	Outflow3 (cfs)	Velocity (ft/sec)	Travel
0.000000	0.018636	0.000000	0.000000	0.000000	0.000000	0.004696		
0.041209	0.036289	0.001486	0.000000	0.000000	0.067320	0.004696		
0.082418	0.036757	0.002991	0.000000	0.000000	0.068868	0.009420		
0.123626	0.037229	0.004515	0.000000	0.000000	0.070417	0.014173		
0.164835	0.037703	0.006059	0.000000	0.000000	0.071966	0.018954		
0.206044	0.038180	0.007623	0.000000	0.000000	0.073515	0.023763		
0.247253	0.038659	0.009206	0.000000	0.000000	0.075063	0.028600		
0.288462	0.039142	0.010809	0.000000	0.000000	0.076612	0.033466		
0.329670	0.039627	0.012432	0.000000	0.000000	0.078161	0.038360		
0.370879	0.040115	0.014075	0.000000	0.000000	0.079710	0.043282		
0.412088	0.040606	0.015738	0.000000	0.000000	0.081259	0.048233		
0.453297	0.041100	0.017422	0.000000	0.000000	0.082807	0.053212		
0.494505	0.041597	0.019126	0.000000	0.000000	0.084356	0.058219		
0.535714	0.042096	0.020850	0.000000	0.000000	0.085905	0.063255		
0.576923	0.042598	0.022595	0.000000	0.000000	0.087454	0.068319		
0.618132	0.043103	0.024361	0.000000	0.000000	0.089002	0.073411		
0.659341	0.043611	0.026148	0.000000	0.000000	0.090551	0.078531		
0.700549	0.044122	0.027955	0.000000	0.000000	0.092100	0.083680		
0.741758	0.044635	0.029784	0.000000	0.000000	0.093649	0.088857		
0.782967	0.045152	0.031634	0.000000	0.000000	0.095197	0.094062		
0.824176	0.045671	0.033506	0.000000	0.000000	0.096746	0.099296		
0.865385	0.046192	0.035398	0.000000	0.000000	0.098295	0.104558		
0.906593	0.046717	0.037313	0.000000	0.000000	0.099844	0.109848		
0.947802	0.047245	0.039249	0.000000	0.000000	0.101392	0.115167		
0.989011	0.047775	0.041207	0.000000	0.000000	0.102941	0.120514		
1.030220	0.048308	0.043186	0.000000	0.000000	0.104490	0.125889		
1.071429	0.048844	0.045188	0.000000	0.000000	0.106039	0.131293		
1.112637	0.049382	0.047212	0.000000	0.000000	0.107588	0.136724		
1.153846	0.049924	0.049258	0.000000	0.000000	0.109136	0.142184		
1.195055	0.050468	0.051327	0.000000	0.000000	0.110685	0.147673		
1.236264	0.051015	0.053418	0.000000	0.000000	0.112234	0.153190		
1.277473	0.051565	0.055531	0.000000	0.000000	0.113783	0.158735		
1.318681	0.052118	0.057668	0.000000	0.000000	0.115331	0.164308		
1.359890	0.052674	0.059827	0.000000	0.000000	0.116880	0.169909		
1.401099	0.053232	0.062009	0.000000	0.000000	0.118429	0.175539		
1.442308	0.053793	0.064214	0.000000	0.000000	0.119978	0.181198		
1.483516	0.054357	0.066442	0.000000	0.000000	0.121526	0.186884		
1.524725	0.054924	0.068694	0.034364	0.123075	0.123075	0.192599		
1.565934	0.055493	0.070969	0.149204	0.124624	0.124624	0.198342		
1.607143	0.056066	0.073268	0.306354	0.126173	0.126173	0.204114		
1.648352	0.056641	0.075590	0.489526	0.127721	0.127721	0.209913		
1.689560	0.057219	0.077936	0.683816	0.129270	0.129270	0.215741		
1.730769	0.057800	0.080306	0.873926	0.130819	0.130819	0.221598		
1.771978	0.058383	0.082700	1.045314	0.132368	0.132368	0.227482		
1.813187	0.058970	0.085118	1.186487	0.133917	0.133917	0.233395		
1.854396	0.059559	0.087560	1.292007	0.135465	0.135465	0.239336		
1.895604	0.060151	0.090026	1.366028	0.137014	0.137014	0.245306		
1.936813	0.060746	0.092517	1.445589	0.138563	0.138563	0.251304		
1.978022	0.061343	0.095033	1.512241	0.140112	0.140112	0.257330		
2.000000	0.061663	0.096385	1.576076	0.140938	0.140938	0.260556		

```
END FTABLE 7
```

```
END FTABLES
```

```
EXT SOURCES
```

```
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
```

<Name>	#	<Name>	#	tem	strg	<-factor-->	strg	<Name>	#	#	<Name>	#	#	***
WDM	2	PREC		ENGL	1			PERLND	1	999	EXTNL	PREC		
WDM	2	PREC		ENGL	1			IMPLND	1	999	EXTNL	PREC		
WDM	1	EVAP		ENGL	0.76			PERLND	1	999	EXTNL	PETINP		
WDM	1	EVAP		ENGL	0.76			IMPLND	1	999	EXTNL	PETINP		
WDM	2	PREC		ENGL	1			RCHRES	1		EXTNL	PREC		
WDM	2	PREC		ENGL	1			RCHRES	3		EXTNL	PREC		
WDM	2	PREC		ENGL	1			RCHRES	5		EXTNL	PREC		
WDM	2	PREC		ENGL	1			RCHRES	7		EXTNL	PREC		
WDM	1	EVAP		ENGL	0.5			RCHRES	1		EXTNL	POTEV		
WDM	1	EVAP		ENGL	0.76			RCHRES	2		EXTNL	POTEV		
WDM	1	EVAP		ENGL	0.5			RCHRES	3		EXTNL	POTEV		
WDM	1	EVAP		ENGL	0.76			RCHRES	4		EXTNL	POTEV		
WDM	1	EVAP		ENGL	0.5			RCHRES	5		EXTNL	POTEV		
WDM	1	EVAP		ENGL	0.76			RCHRES	6		EXTNL	POTEV		
WDM	1	EVAP		ENGL	0.5			RCHRES	7		EXTNL	POTEV		
WDM	1	EVAP		ENGL	0.76			RCHRES	8		EXTNL	POTEV		

END EXT SOURCES

EXT TARGETS

<-Volume-->	<-Grp>	<-Member-->	<--Mult-->	Tran	<-Volume-->	<Member>	Tsys	Tgap	Amd	***			
<Name>	#	<Name>	#	#	<-factor-->	strg	<Name>	#	<Name>	tem	strg	strg	***
COPY	1	OUTPUT	MEAN	1	1	48.4	WDM	701	FLOW	ENGL	REPL		
COPY	501	OUTPUT	MEAN	1	1	48.4	WDM	801	FLOW	ENGL	REPL		
COPY	601	OUTPUT	MEAN	1	1	48.4	WDM	901	FLOW	ENGL	REPL		
RCHRES	9	HYDR	RO	1	1	1	WDM	1000	FLOW	ENGL	REPL		
RCHRES	9	HYDR	O	1	1	1	WDM	1001	FLOW	ENGL	REPL		
RCHRES	9	HYDR	O	2	1	1	WDM	1002	FLOW	ENGL	REPL		
RCHRES	9	HYDR	STAGE	1	1	1	WDM	1003	STAG	ENGL	REPL		
RCHRES	10	HYDR	RO	1	1	1	WDM	1004	FLOW	ENGL	REPL		
RCHRES	10	HYDR	O	1	1	1	WDM	1005	FLOW	ENGL	REPL		
RCHRES	10	HYDR	O	2	1	1	WDM	1006	FLOW	ENGL	REPL		
RCHRES	10	HYDR	STAGE	1	1	1	WDM	1007	STAG	ENGL	REPL		
RCHRES	11	HYDR	RO	1	1	1	WDM	1008	FLOW	ENGL	REPL		
RCHRES	11	HYDR	O	1	1	1	WDM	1009	FLOW	ENGL	REPL		
RCHRES	11	HYDR	O	2	1	1	WDM	1010	FLOW	ENGL	REPL		
RCHRES	11	HYDR	STAGE	1	1	1	WDM	1011	STAG	ENGL	REPL		
RCHRES	8	HYDR	RO	1	1	1	WDM	1012	FLOW	ENGL	REPL		
RCHRES	8	HYDR	O	1	1	1	WDM	1013	FLOW	ENGL	REPL		
RCHRES	8	HYDR	O	2	1	1	WDM	1014	FLOW	ENGL	REPL		
RCHRES	8	HYDR	STAGE	1	1	1	WDM	1015	STAG	ENGL	REPL		
RCHRES	7	HYDR	STAGE	1	1	1	WDM	1016	STAG	ENGL	REPL		
RCHRES	7	HYDR	O	1	1	1	WDM	1017	FLOW	ENGL	REPL		

MASS-LINK

<Volume>	<-Grp>	<-Member-->	<--Mult-->	<Target>	<-Grp>	<-Member-->	***
<Name>	#	<Name>	#	<-factor-->	<Name>	#	***
MASS-LINK		2					
PERLND	PWATER	SURO		0.083333	RCHRES	INFLOW	IVOL
END MASS-LINK		2					
MASS-LINK		3					
PERLND	PWATER	IFWO		0.083333	RCHRES	INFLOW	IVOL
END MASS-LINK		3					
MASS-LINK		5					
IMPLND	IWATER	SURO		0.083333	RCHRES	INFLOW	IVOL
END MASS-LINK		5					
MASS-LINK		7					
RCHRES	OFLOW	OVOL	1		RCHRES	INFLOW	IVOL
END MASS-LINK		7					
MASS-LINK		8					
RCHRES	OFLOW	OVOL	2		RCHRES	INFLOW	IVOL
END MASS-LINK		8					
MASS-LINK		12					

PERLND	PWATER	SURO	0.083333	COPY	INPUT	MEAN
END MASS-LINK		12				
MASS-LINK		13				
PERLND	PWATER	IFWO	0.083333	COPY	INPUT	MEAN
END MASS-LINK		13				
MASS-LINK		15				
IMPLND	IWATER	SURO	0.083333	COPY	INPUT	MEAN
END MASS-LINK		15				
MASS-LINK		17				
RCHRES	OFLOW	OVOL	1	COPY	INPUT	MEAN
END MASS-LINK		17				

END MASS-LINK

END RUN

Predeveloped HSPF Message File

Mitigated HSPF Message File

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Conveyances from WWHM

Inflow to Bioretention Cell 1 (Northwest):

Flow Frequency	
Flow(cfs)	0701 15m
2 Year	= 3.2659
5 Year	= 4.5032
10 Year	= 5.4595
25 Year	= 6.8352
50 Year	= 7.9899
100 Year	= 9.2631

Inflow to Bioretention Cell 2 (Southwest):

Flow Frequency	
Flow(cfs)	0702 15m
2 Year	= 0.8092
5 Year	= 1.1243
10 Year	= 1.3693
25 Year	= 1.7235
50 Year	= 2.0221
100 Year	= 2.3526

Inflow to Bioretention Cell 3 (East):

Flow Frequency	
Flow(cfs)	0703 15m
2 Year	= 3.1244
5 Year	= 4.3473
10 Year	= 5.2992
25 Year	= 6.6768
50 Year	= 7.8392
100 Year	= 9.1263

Inflow to Bioretention Cell 4 (Southeast Frontage):

Flow Frequency	
Flow(cfs)	0704 15m
2 Year	= 0.2132
5 Year	= 0.2986
10 Year	= 0.3654
25 Year	= 0.4625
50 Year	= 0.5448
100 Year	= 0.6361

Bioretention Stage from WWHM

Bioretention Cell 1 (Northwest):

Stage Frequency	
(feet)	1021 15m
2 Year	= 1.7408
5 Year	= 1.7411
10 Year	= 1.7413
25 Year	= 1.7414
50 Year	= 1.7415
100 Year	= 1.7415

Bioretention Cell 2 (Southwest):

Stage Frequency	
(feet)	1027 15m
2 Year	= 1.4645
5 Year	= 1.4680
10 Year	= 1.4695
25 Year	= 1.4709
50 Year	= 1.4717
100 Year	= 1.4723

Bioretention Cell 3 (East):

Stage Frequency	
(feet)	1033 15m
2 Year	= 1.7357
5 Year	= 1.7359
10 Year	= 1.7360
25 Year	= 1.7361
50 Year	= 1.7361
100 Year	= 1.7361

Bioretention Cell 4 (Southeast Frontage):

Stage Frequency	
(feet)	1015 15m
2 Year	= 1.5528
5 Year	= 1.6068
10 Year	= 1.6370
25 Year	= 1.6707
50 Year	= 1.6935
100 Year	= 1.7145

Appendix 4: Soil Management Plan

To be completed with final design.

Drainage Control Plan Attachments

Attachment 1: Construction SWPPP Report

To be completed with final design.

Attachment 2: Maintenance and Source Control Manual

To be completed with final design.